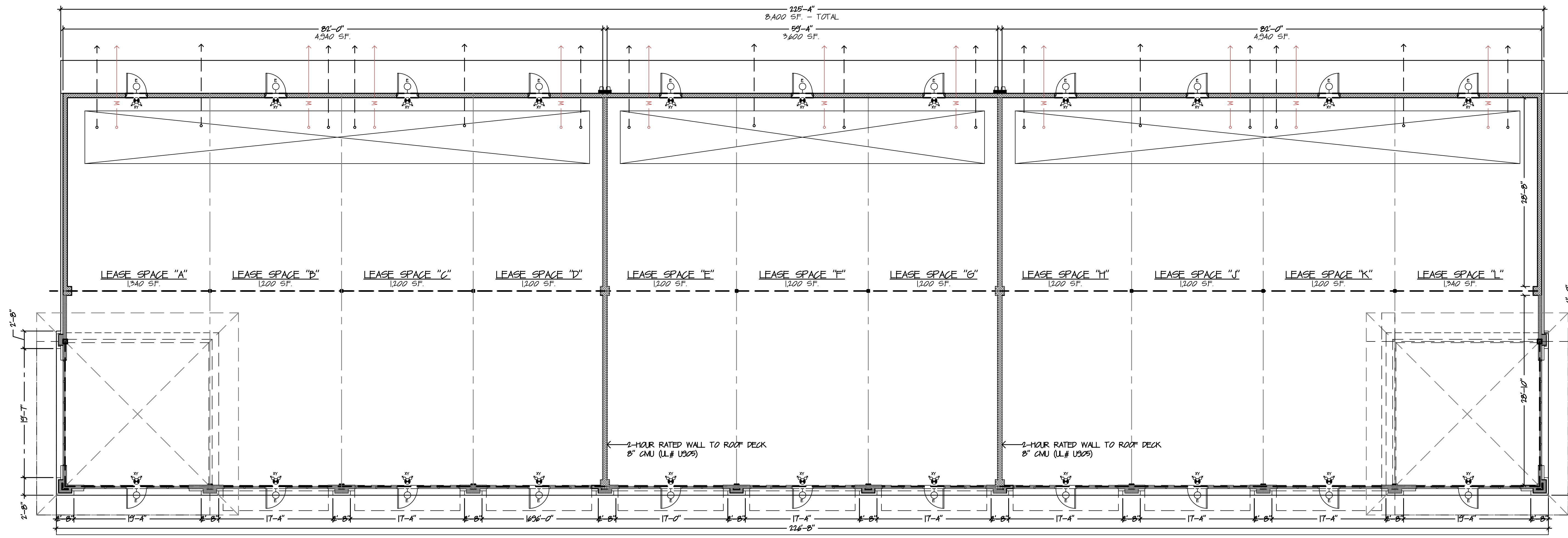


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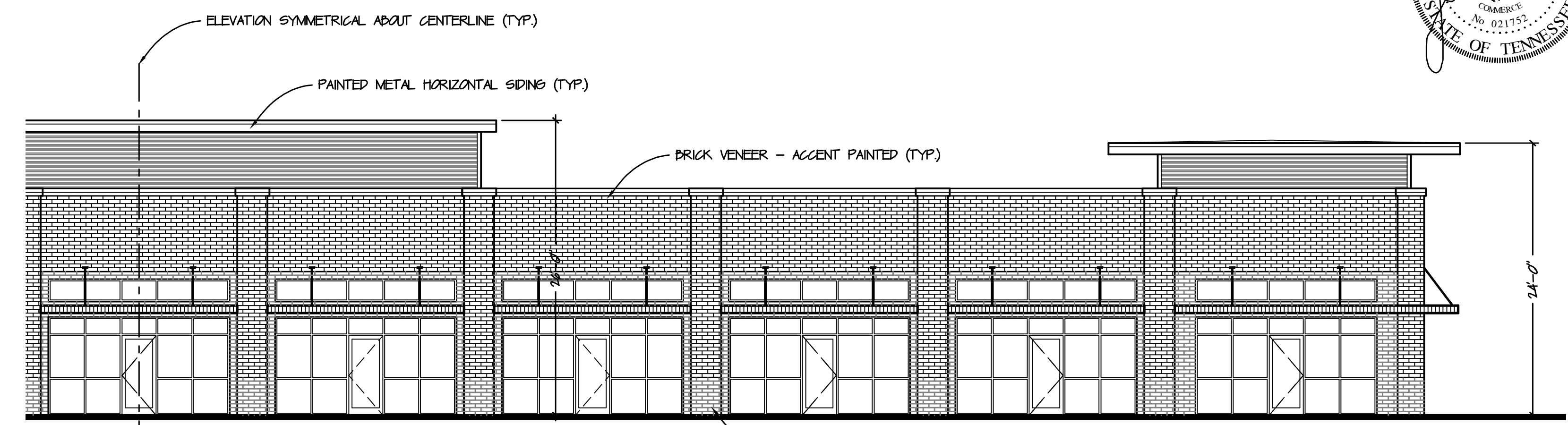
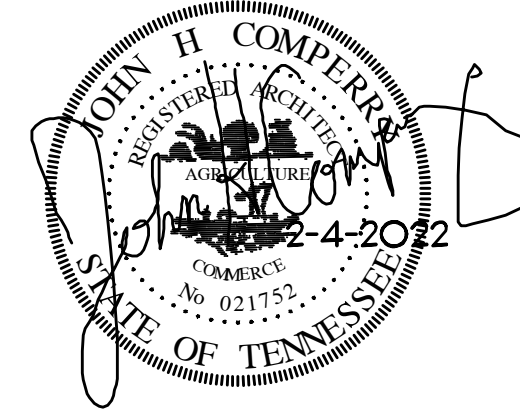
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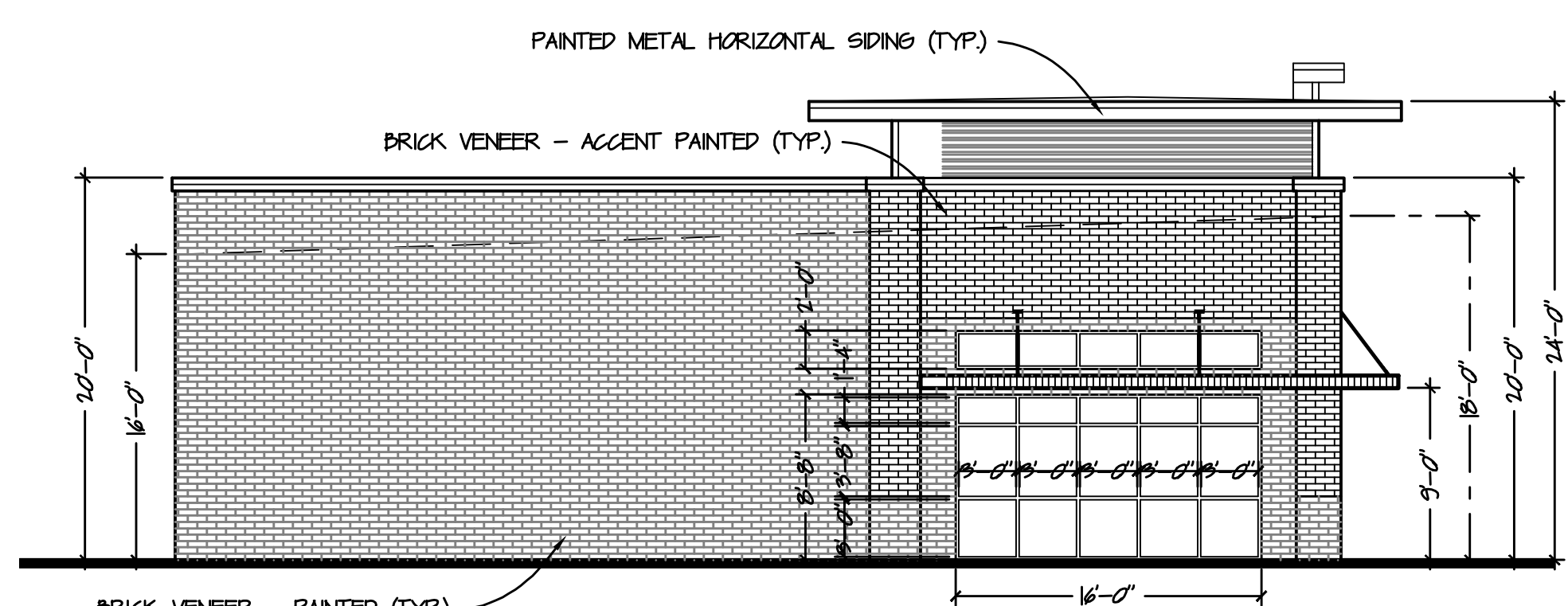
A FLOOR PLAN
 SCALE: 1/8" = 1'-0"

CODE SUMMARY DATA

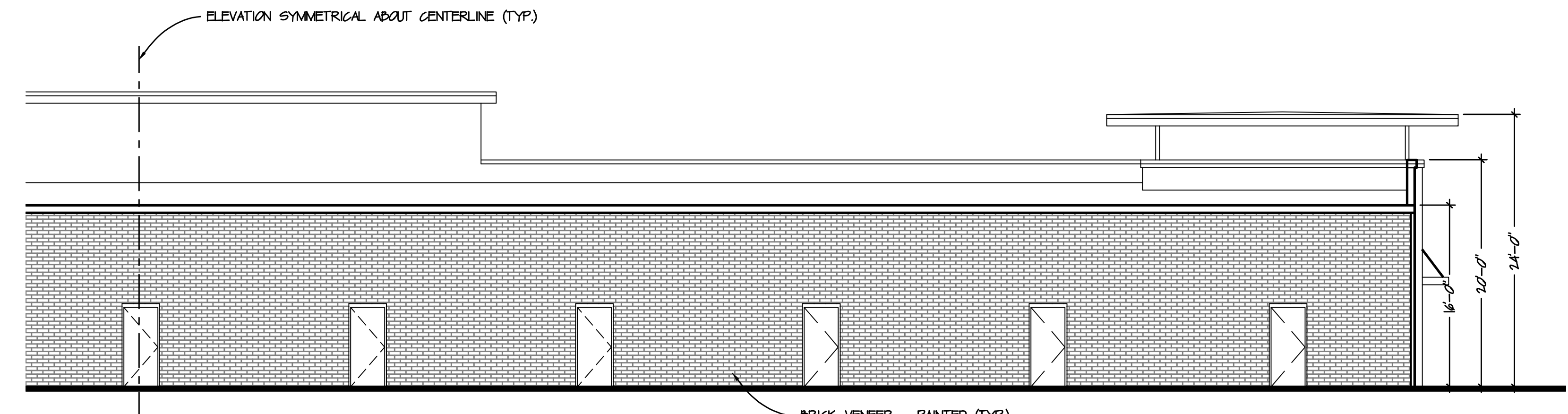
- BUILDING CODE(S):** 2018 ICC - IBC, IFGC, IMC, IPC, 2009 IECC, & 2017 NEC
- ACCESSIBILITY CODE:** 2009 ICC/ANSI A117.1 - ACCESSIBLE & USEABLE BUILDINGS
- USE GROUP:** B "BUSINESS" (SECTION 304)
M "MERCANTILE" (SECTION 309)
- CONSTRUCTION TYPE:** V-B (TABLE 601)
- SPRINKLER SYSTEM:** n/a
- TWO HOUR RATED FIRE BARRIER:** UL# U905
- ALLOWABLE AREA:** 15,750 SF - 2 STORY (W/ 75% FRONTAGE INCREASE)
- TOTAL GROSS AREA:** 13,775 SF - 1 STORY
- MAX. ALLOWABLE HEIGHT:** 40'-0"
- ACTUAL BUILDING HEIGHT:** 27'-8"
- EXIT ACCESS TRAVEL DISTANCE:** 200 FEET, (TABLE 1016.1)
- ACTUAL MAX. TRAVEL DISTANCE:** <35 FEET



B FRONT ELEVATION
 SCALE: 1/8" = 1'-0"



C TYP. SIDE ELEVATION
 SCALE: 1/8" = 1'-0"



D REAR ELEVATION
 SCALE: 1/8" = 1'-0"

Accent
 Storefront Metal
 Brick

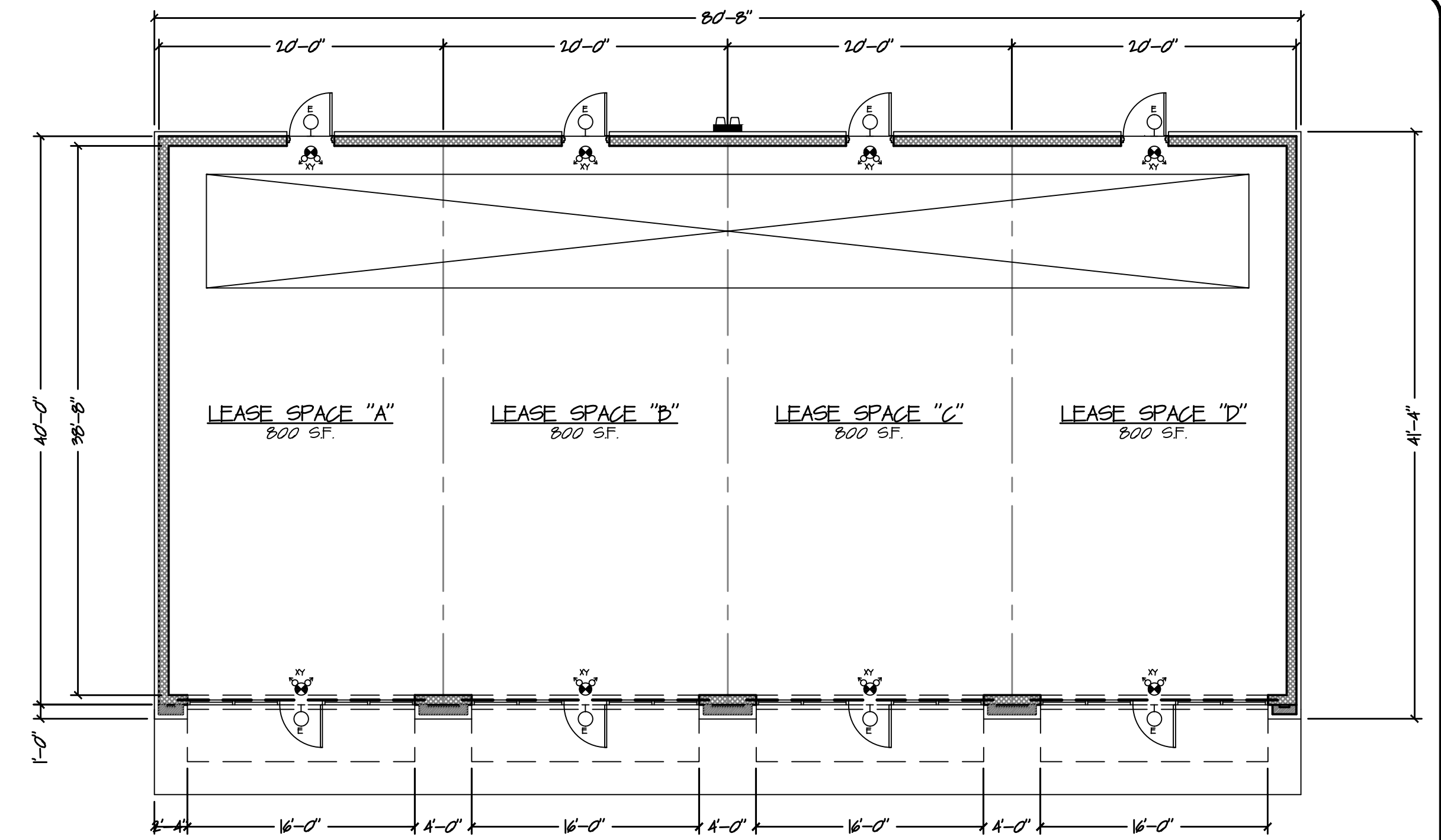
SW 6376
 Critch Linen

SW 7069
 Iron Ore

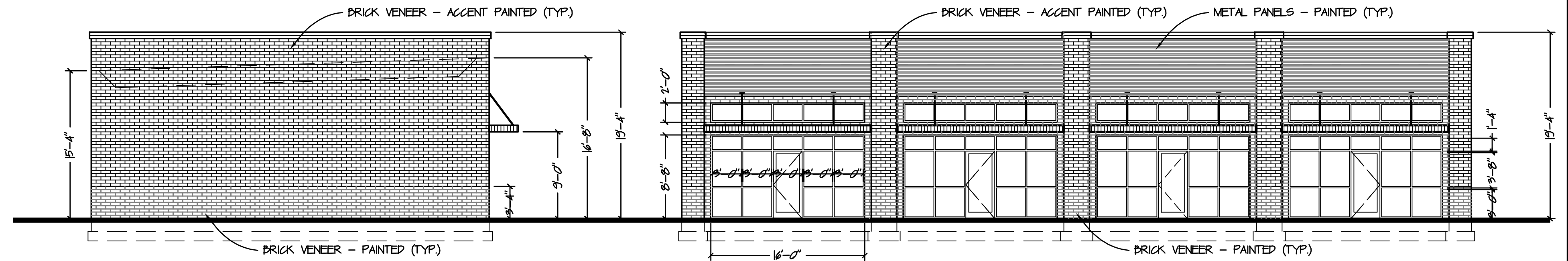
SW 7025
 Blacktop

CODE SUMMARY DATA

BUILDING CODE(S):	2018 ICC - IBC, IFGC, IMC, IPC, 2009 IECC, & 2017 NEC
ACCESSIBILITY CODE:	2009 ICC/ANSI A117.1 - ACCESSIBLE & USEABLE BUILDINGS
USE GROUP:	B "BUSINESS" (SECTION 304) M "MERCANTILE" (SECTION 309)
CONSTRUCTION TYPE:	V-B (TABLE 601)
SPRINKLER SYSTEM:	n/a
TWO HOUR RATED FIRE BARRIER:	n/a
ALLOWABLE AREA:	9,000 SF - 2 STORY
TOTAL GROSS AREA:	3,200 SF - 1 STORY
MAX. ALLOWABLE HEIGHT:	40'-0"
ACTUAL BUILDING HEIGHT:	19'-4"
EXIT ACCESS TRAVEL DISTANCE:	200 FEET, (TABLE 1016.1)
ACTUAL MAX. TRAVEL DISTANCE:	<35 FEET

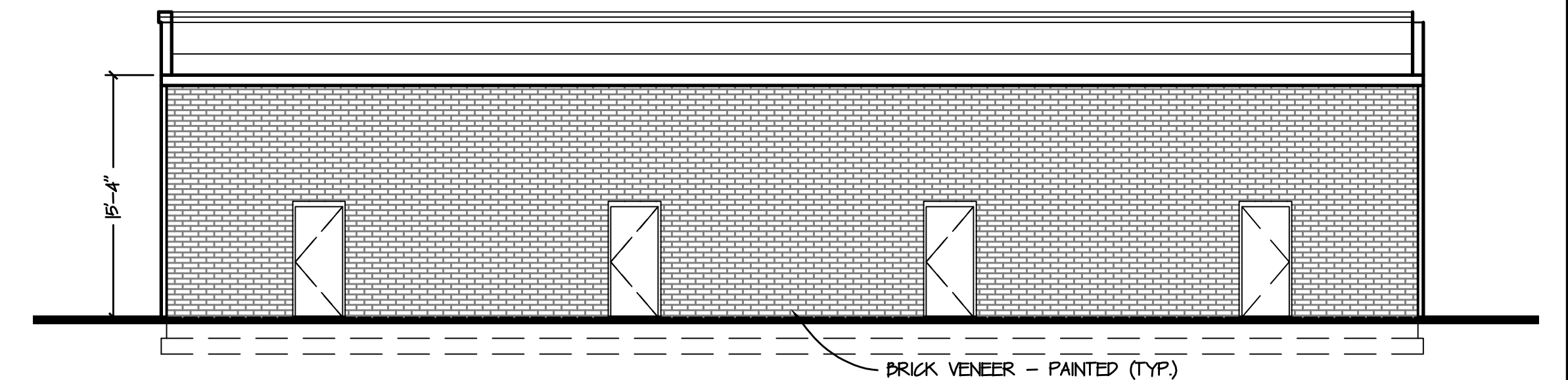


A FLOOR PLAN
A-101.1 SCALE: 1/8" = 1'-0"



C TYP. SIDE ELEVATION
A-101.1 SCALE: 1/8" = 1'-0"

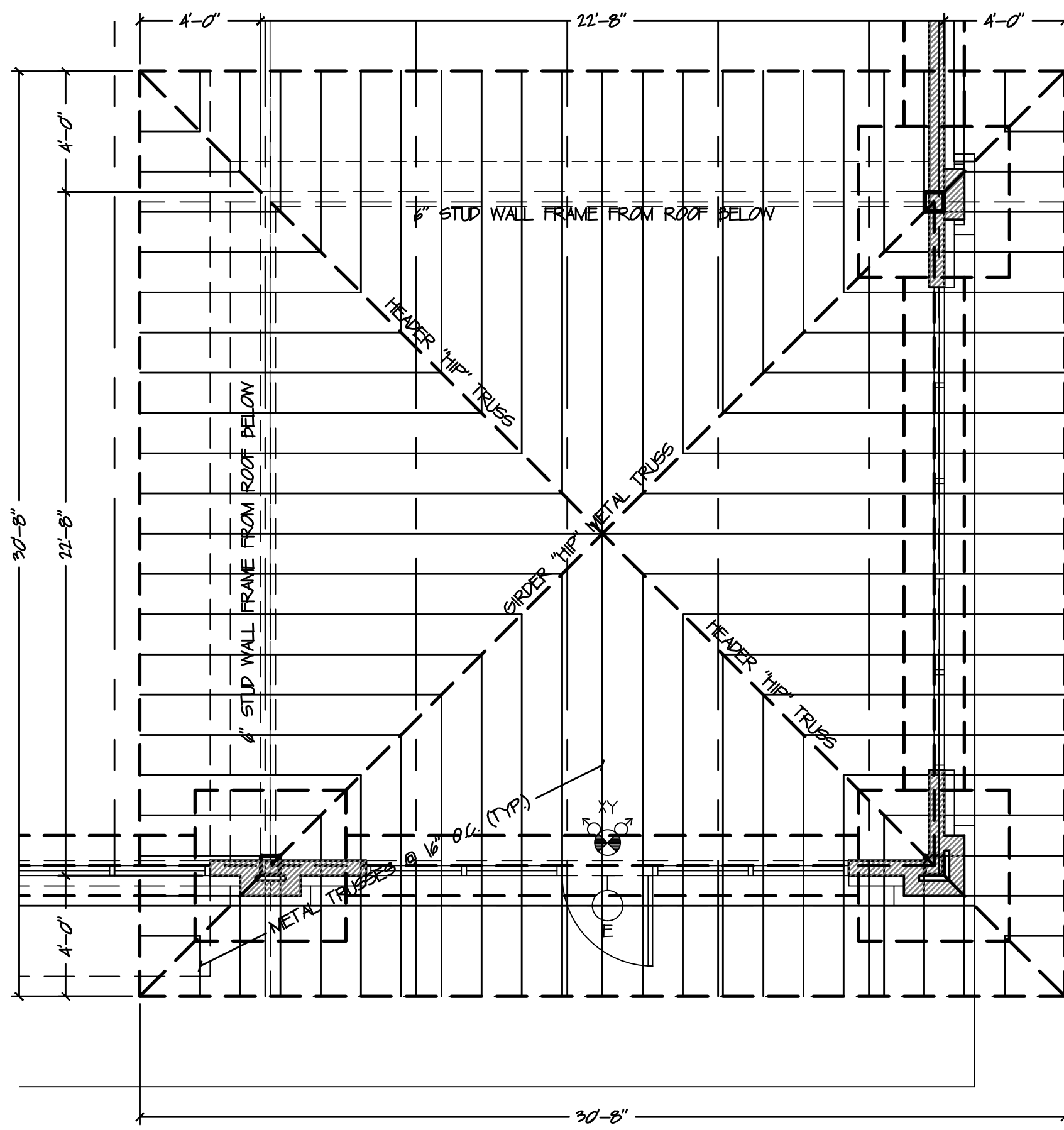
B FRONT ELEVATION
A-101.1 SCALE: 1/8" = 1'-0"



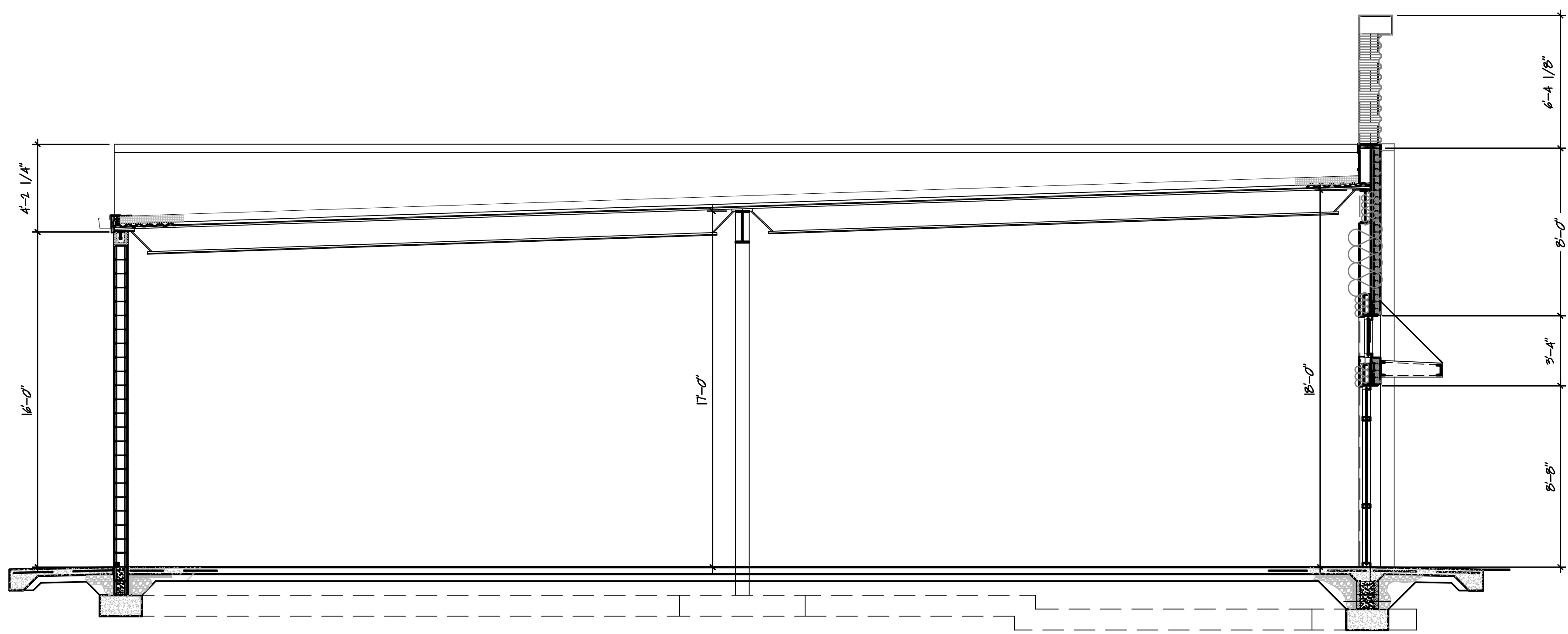
D REAR ELEVATION
A-101.1 SCALE: 1/8" = 1'-0"

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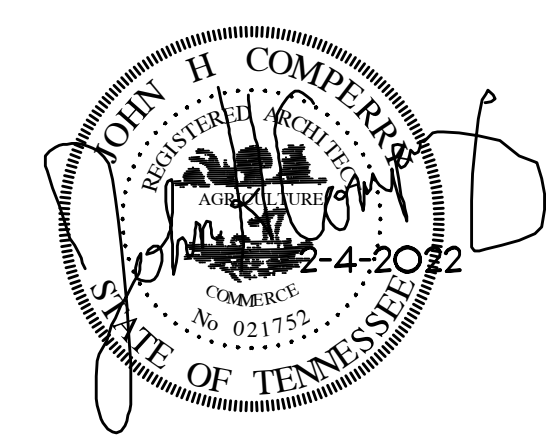
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APPROVED	-
JOB NO.	0622_Excell Retail
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OF	- SHEETS



B "HAT" FRAMING PLAN
A-201 SCALE: 1/4" = 1'-0"



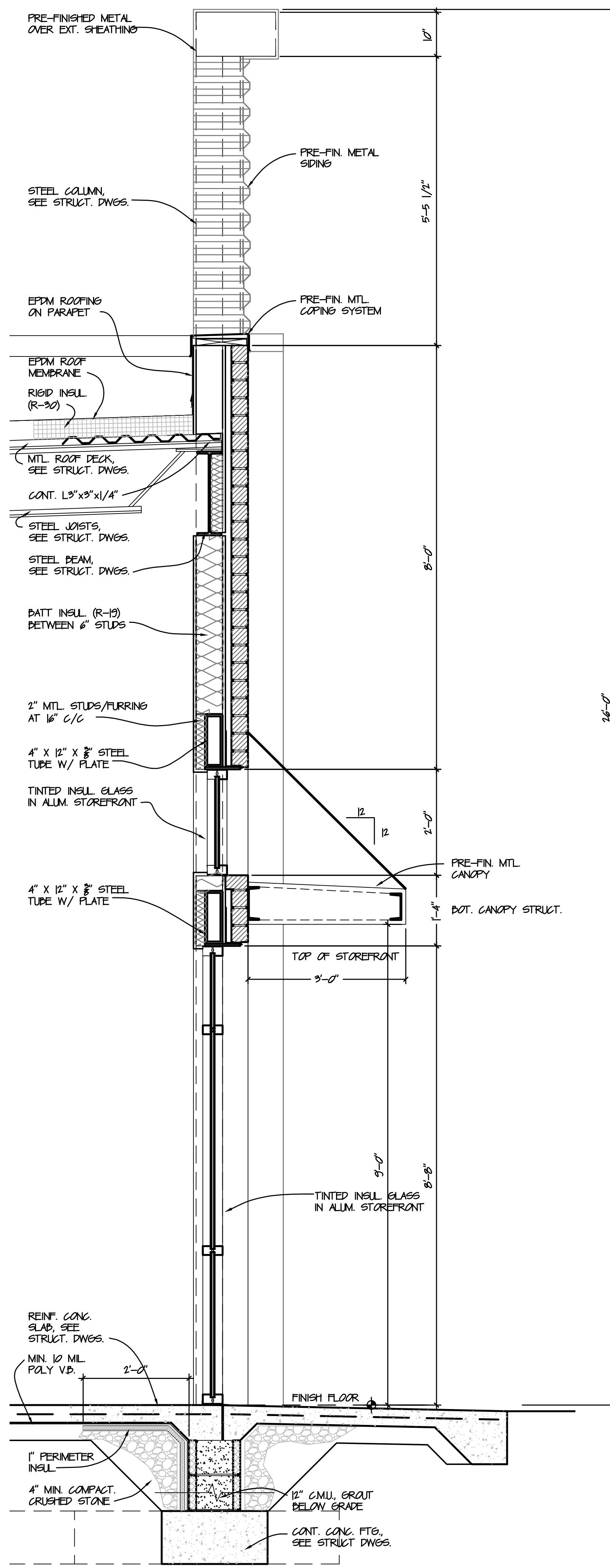
A BUILDING SECTION
A-201 SCALE: 1/4" = 1'-0"



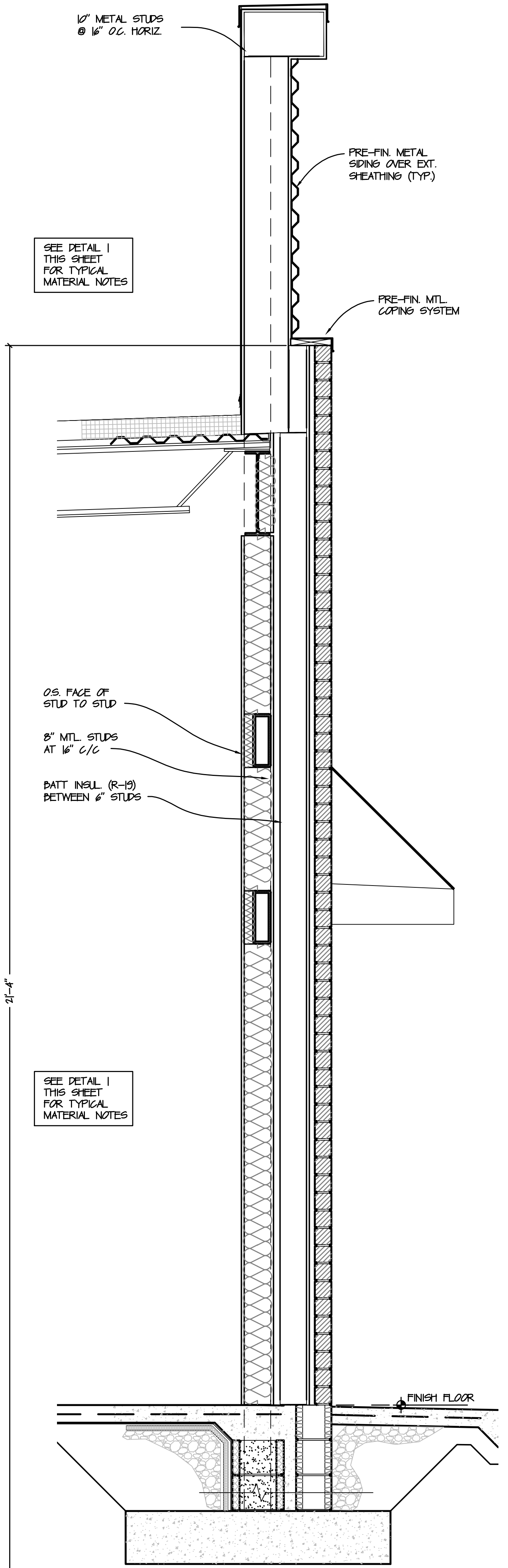
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PHASE I - BUILDING SECTION
EXCELL RETAIL PLAZA
 EXCELL RD. & MADISON STREET - CLARKSVILLE, TN MILLAN ENTERPRISES

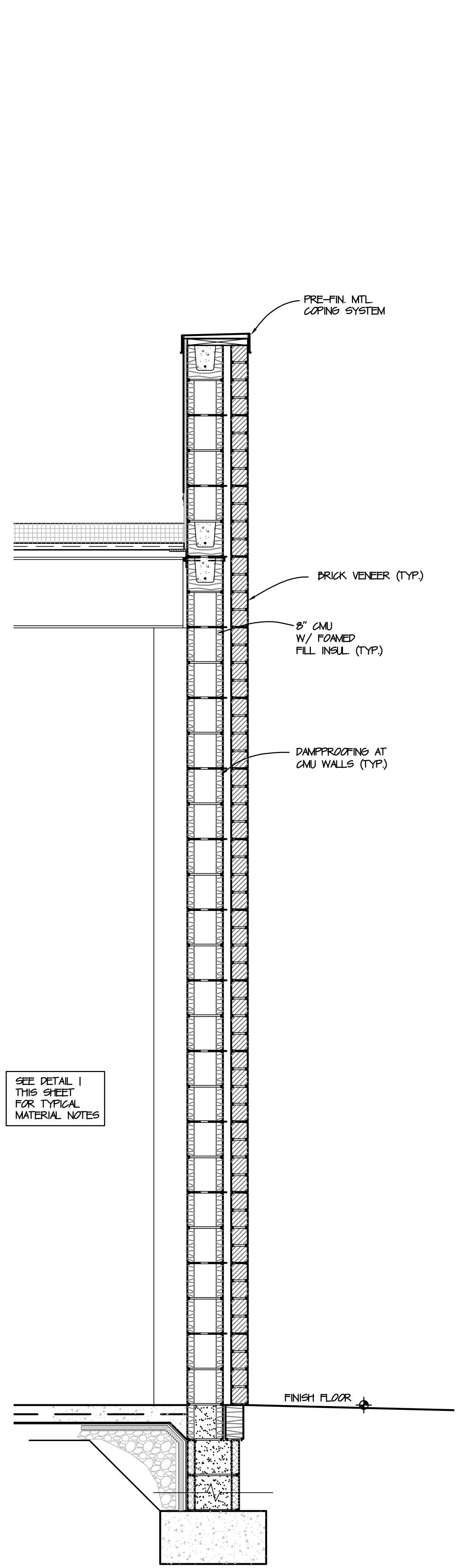
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APPROVED	-
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DATE	2-4-2022
SHEET	-



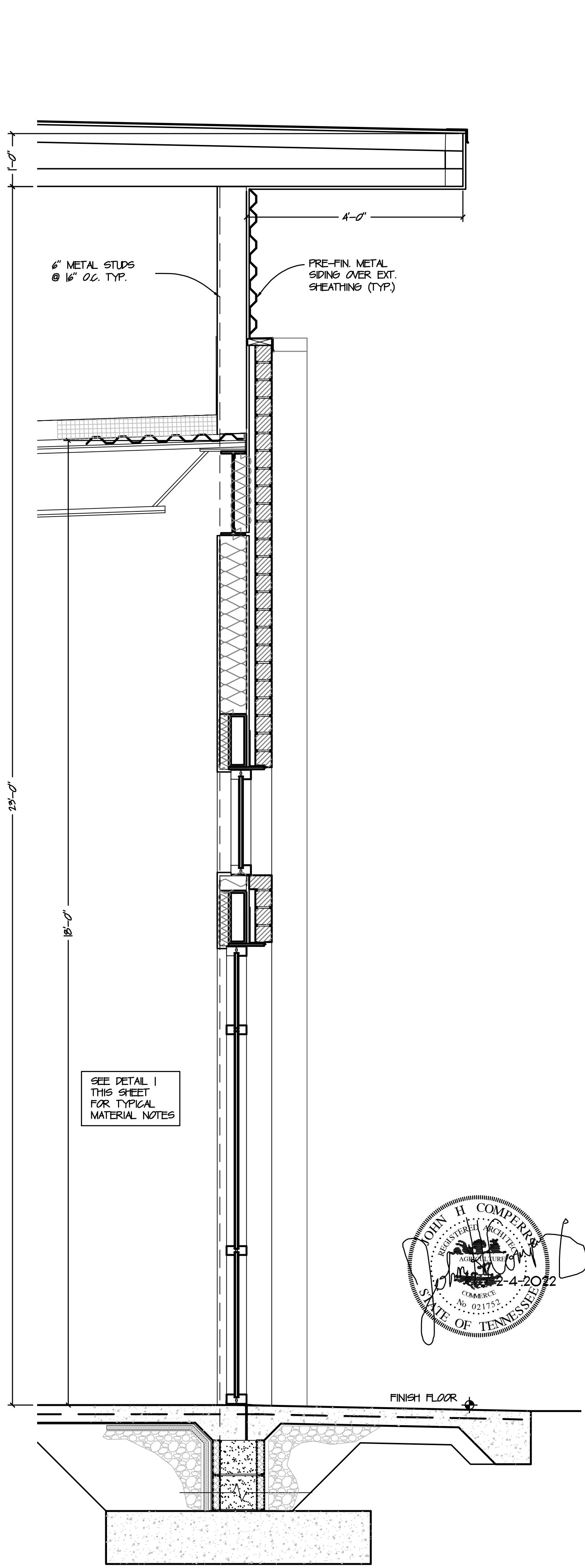
1 WALL SECTION
SCALE: 3/4" = 1'-0"



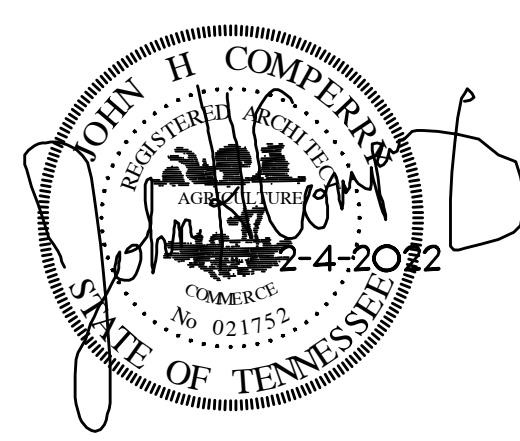
2 WALL SECTION
SCALE: 3/4" = 1'-0"



3 WALL SECTION
SCALE: 3/4" = 1'-0"



4 WALL SECTION
SCALE: 3/4" = 1'-0"



REVISIONS	BY

PHASE I - TYPICAL WALL SECTIONS
EXCELL RETAIL PLAZA
EXCELL RD. & MADISON STREET - CLARKSVILLE, TN
MILLAN ENTERPRISES

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APPROVED	
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DATE	2-4-2022
SHEET	A-301
OF	
SHEETS	

STRUCTURAL NOTES:

- NO PROVISION OF ANY REFERENCED STANDARD SPECIFICATION, MANUAL OR CODE (WHETHER OR NOT SPECIFICALLY INCORPORATED BY REFERENCE IN THE CONTRACT DOCUMENTS) WILL BE EFFECTIVE TO CHANGE THE DUTIES AND RESPONSIBILITIES OF OWNER, CONTRACTOR, DESIGNER, SUPPLIER, OR ANY OF THEIR CONSULTANTS, AGENTS, OR EMPLOYEES FROM THOSE SET FORTH IN THE CONTRACT DOCUMENTS. NOR SHALL IT BE EFFECTIVE TO ASSIGN TO THE DESIGNER OR ANY OF THE DESIGNER'S CONSULTANTS, AGENTS, OR EMPLOYEES ANY DUTY OR AUTHORITY TO SUPERVISE OR DIRECT THE FURNISHING OR PERFORMANCE OF THE WORK OR ANY DUTY OR AUTHORITY TO UNDERTAKE RESPONSIBILITIES CONTRARY TO THE PROVISIONS OF THE CONTRACT DOCUMENTS.
- REFERENCE TO STANDARD SPECIFICATIONS OF ANY TECHNICAL SOCIETY, ORGANIZATION, OR ASSOCIATION OR TO CODES OF LOCAL OR STATE AUTHORITIES, SHALL MEAN THE LATEST STANDARD, CODE, SPECIFICATION OR TENTATIVE SPECIFICATION ADOPTED AT THE DATE OF TAKING BIDS, UNLESS SPECIFICALLY STATED OTHERWISE.
- CONTRACT DOCUMENTS WILL GOVERN IN THE EVENT OF A CONFLICT WITH THE CODE OF PRACTICE OR SPECIFICATIONS OF ACI, PCI, AIA, SJI OR OTHER STANDARDS. WHERE A CONFLICT OCCURS WITHIN THE CONTRACT DOCUMENTS, THE STRICTEST REQUIREMENT WILL GOVERN.
- MATERIAL WORKMANSHIP, AND DESIGN SHALL CONFORM TO THE REFERENCED BUILDING CODE.
- CONTRACTOR WILL VERIFY EXISTING DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS BEFORE STARTING WORK. DESIGNER WILL BE NOTIFIED OF ANY DISCREPANCY.
- CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
- THE STRUCTURE IS STABLE ONLY IN ITS COMPLETED FORM. TEMPORARY SUPPORTS REQUIRED FOR STABILITY DURING ALL INTERMEDIATE STAGES OF CONSTRUCTION SHALL BE DESIGNED, FURNISHED, AND INSTALLED BY THE CONTRACTOR.
- CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA REGULATIONS.

CODE DESIGN CRITERIA

- STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, LATEST ADOPTED EDITION.
- DESIGN LIVE LOADS (REDUCED AS ALLOWED BY THE BUILDING CODE):

ROOF	= 20 PSF
GROUND SNOW LOAD	= 15 PSF
FLOOR LIVE LOAD	= 100 PSF
- ALL ROOF AREAS ARE TO BE DESIGNED FOR A COLLATERAL DEAD LOAD OF 7 PSF, TO INCLUDE WEIGHT OF ARCHITECTURAL INTERIOR CLADDING AND MECHANICAL AND ELECTRICAL SYSTEMS. COLLATERAL LOAD IS IN ADDITION TO THE SELF-WEIGHT OF THE STRUCTURAL FRAMING AND EXTERIOR CLADDING.
- SNOW LOADS

GROUND SNOW LOAD, P _g	= 15 PSF
FLAT ROOF SNOW LOAD, P _f	= 15 PSF (BLDG)
	= 17.6 PSF (CANOPY)
SNOW EXPOSURE COEFFICIENT, C _e	= 1.0
SNOW LOAD IMPORTANCE FACTOR, I	= 1.0
THERMAL FACTOR, C _t	= 1.0
- WIND LOADS: (PER ASCE 7-02 FOR BUILDINGS UNDER 60')

BASIC WIND SPEED	= 90 MPH
IMPORTANCE FACTOR	= 1.0
EXPOSURE CATEGORY	= C
VELOCITY PRESSURE, q _h	= 15.9 PSF
- EARTHQUAKE LOADS:

SEISMIC IMPORTANCE FACTOR	= 1.0
SEISMIC USE GROUP	= I
MAPPED SPECTRAL RESPONSE ACCELERATION, S _s	= 0.09g
MAPPED SPECTRAL RESPONSE ACCELERATION, S ₁	= 0.17g
SITE CLASS	= C (ASSUMED)
SPECTRAL RESPONSE COEFFICIENT, S _{ds}	= 0.410
SPECTRAL RESPONSE COEFFICIENT, S _{d1}	= 0.216
SEISMIC DESIGN CATEGORY	= C

BASIC SEISMIC-FORCE RESISTING SYSTEM: SPECIAL REINFORCED MASONRY SHEAR WALLS

R	= 5.0 (SHEAR WALLS)
C _d	= 3.5 (SHEAR WALLS)

ANALYSIS PROCEDURE: EQUIV. LATERAL FORCE PROCEDURE
- STRUCTURAL TESTS AND SPECIAL INSPECTIONS SHALL BE PERFORMED PER SECTION 1704 OF THE INTERNATIONAL BUILDING CODE.

FOUNDATION

- ALL FOUNDATIONS SHALL BE INSTALLED UNDER THE GUIDANCE OF A REGISTERED PROFESSIONAL GEOTECHNICAL ENGINEER IN THE PROJECT STATE. THE GEOTECHNICAL ENGINEER SHALL GIVE CONSIDERATION TO THE TYPE OF BUILDING AND FOUNDATION LOADS INVOLVED AS WELL AS THE REQUIREMENTS OF THESE DOCUMENTS. DESIGNER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.
- STRUCTURAL TESTING/INSPECTION AGENCY SHALL CERTIFY THE BEARING MEDIUM.
- INDIVIDUAL SPREAD FOOTINGS AND CONTINUOUS FOOTINGS SHALL BEAR ON SOIL CAPABLE OF SUPPORTING 1,000 PSF AND 2,000 PSF, RESPECTIVELY.
- NO FOOTINGS SHALL BEAR ON ROCK UNDERCUT ROCK A MINIMUM OF 2 FEET BELOW BOTTOM OF FOOTING AND REPLACE WITH STRUCTURAL FILL.
- STRUCTURAL FILL SHALL CONTAIN NO ORGANIC MATERIAL AND BE APPROVED BY A GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT. STRUCTURAL FILL UNDER SLABS AND WITHIN 12'-0" OF THE BUILDING FOOTPRINT SHALL BE PLACED IN LIFTS OF THICKNESS DETERMINED BY THE INDEPENDENT TESTING AGENCY AND COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698. THE TOP 2" SUB-BASE UNDER SLABS ON GRADE SHALL BE COMPACTED TO AT LEAST 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY. ALL BACKFILL, COMPACTION AND PROOF ROLLING OPERATIONS SHALL BE OBSERVED BY AN INDEPENDENT TESTING LABORATORY.
- SLABS-ON-GRADE SHALL BE PLACED ON A 4" GRANULAR BASE, COMPACTED TO 98% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D698, AND COVERED WITH A 10 MIL CONTINUOUSLY SEALED VAPOR BARRIER. THE BASE FOR SLABS-ON-GRADE SHALL BE INSPECTED BY A GEOTECHNICAL ENGINEER PRIOR TO EACH PLACEMENT OF CONCRETE.
- ALL FOOTINGS AND TURN DOWN SLAB EDGES SHALL PENETRATE TO A MINIMUM DEPTH OF 12" BELOW FINISHED GRADE.

REINFORCEMENT

- REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A655 AND HAVE MINIMUM SIDE AND END LAPS OF 8".
- SPICES SHALL BE CLASS D IN ACCORDANCE WITH ACI-308, UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL BE SPICED ONLY AT LOCATIONS SHOWN OR NOTED IN THE STRUCTURAL DOCUMENTS, EXCEPT REINFORCEMENT MARKED "CONTINUOUS" CAN BE SPICED AT LOCATIONS DETERMINED BY CONTRACTOR. SPICES AT OTHER LOCATIONS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER.
- PROVIDE DOWELS FROM FOUNDATIONS THE SAME SIZE AND NUMBER AS THE VERTICAL WALL OR COLUMN REINFORCING, UNLESS NOTED OTHERWISE.
- PLACE REINFORCEMENT AS FOLLOWS, UNLESS NOTED OTHERWISE:
 - CONCRETE REINFORCEMENT COVER BELOW GRADE: UNFORMED 3" CLEAR, FORMED 2" CLEAR
 - MASONRY REINFORCING STEEL SHALL BE PLACED IN THE CENTER OF THE WALL UNLESS NOTED OTHERWISE.
- REINFORCING STEEL DESIGNATED CONTINUOUS SHALL BE LAPPED AS FOLLOWS:

CONCRETE REINFORCEMENT:	CLASS B TENSION LAP
MASONRY REINFORCEMENT:	4B BAR DIAMETERS
- ADHESIVE FOR REINFORCING DOWELS IN EXISTING CONCRETE SHALL BE EITHER THE EPON SYSTEM (EPOXY & EPOXY ADHESIVE SUPPLIED BY ITW RAMBET RED HEAD OR THE HIT HYBRID INJECTION ADHESIVE SYSTEM BY HILTI FASTENING SYSTEMS, OR APPROVED EQUAL MINIMUM EMBEDMENT LENGTH SHALL BE 12 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
- ALL DOWELS AND TERMINATING BARS SHALL HAVE A STANDARD 90 DEGREE HOOK.
- ALL HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL AND/OR CONSTRUCTION JOINTS AND AROUND CORNERS.

CAST-IN-PLACE CONCRETE

- CONCRETE WORK SHALL CONFORM TO ACI-308 AND CRSI STANDARDS.
- CONCRETE WALL HAVE THE FOLLOWING MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH:

21) NORMAL WEIGHT STRUCTURAL CONCRETE:			
MIN. 28 DAY	AIR	W/C %	
FOOTINGS	3,000 PSI	N/A	0.6
SLABS-ON-GRADE	4,000 PSI	N/A	0.5
EXTERIOR EXPOSED	4,000 PSI	6%	0.5
- REFER TO ARCHITECTURAL DRAWINGS FOR WALDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE ENCASED IN CONCRETE AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS.
- DEFECTIVE AREAS IN CONCRETE INCLUDING, BUT NOT LIMITED TO, HONEY-COMBING, WALLS, AND CRACKS WITH WIDTHS EXCEEDING 0.01 INCH SHALL BE REPAIRED. EXTENT OF DEFECTIVE AREA TO BE DETERMINED BY THE DESIGNER.
- CONCRETE MIX DESIGN FOR 3,000 PSI CONCRETE SHALL BE BASED ON A MAXIMUM AGGREGATE SIZE OF 1 IN. MAXIMUM WATER CEMENT RATIO OF 50 FOR NON-AIR-ENTRAINED CONCRETE AND 45 FOR AIR-ENTRAINED CONCRETE AND A MAXIMUM SLUMP OF 4 IN. AIR ENTRAINMENT CONCRETE SHALL BE USED FOR EXTERIOR EXPOSED CONCRETE WITH AN AIR CONTENT BETWEEN 5% AND 7% PERCENT.
- CONCRETE SLABS ON GRADE SHALL NOT BE LOADED UNTIL A MINIMUM CONCRETE STRENGTH OF 800 PSI HAS BEEN ATTAINED AND THE CONCRETE IS AT LEAST THREE DAYS OLD. ALL OTHER CONCRETE MEMBERS SHALL NOT BE LOADED UNTIL THE SPECIFIED CONCRETE DESIGN STRENGTH HAS BEEN ATTAINED.
- CONCRETE WALL BE TESTED IN ACCORDANCE WITH ACI-301 AND THE SPECIFICATIONS FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS. AT A MINIMUM CONCRETE SPECIMENS WILL BE TAKEN FOR EVERY 100 YARDS OR PORTION THEREOF FOR EACH MIX DESIGN PLACED IN A DAY. CONCRETE TEST REPORTS WILL BE AVAILABLE ON SITE FOR INSPECTION.
- C/J ON THE SLAB AND FOUNDATION PLAN INDICATES A KEY-FORMED CONSTRUCTION JOINT OR SAW-CUT CONTROL JOINT IN THE CONCRETE SLAB ON GRADE. SAW-CUT CONTROL JOINTS WILL BE INSTALLED WITHIN 12 HOURS OF SLAB PLACEMENT. CONTINUE REINFORCEMENT THROUGH JOINTS. DO NOT CUT REINFORCING STEEL. CONSTRUCTION JOINTS AND/OR CONTROL JOINTS SHALL BE SPACED NO FARTHER APART THAN 20 FT O.C. IN EACH DIRECTION CREATING PANELS WITH AN ASPECT RATIO NOT GREATER THAN 3:1.
- UNLESS OTHERWISE NOTED, ALL REINFORCING SHALL BE CONTACT LAP SPICED WITH A CLASS D SPICE IN ACCORDANCE WITH ACI-308-89. FOR BARS WITH MINIMUM COVER AND SPACING GREATER THAN 2db AND 3db RESPECTIVELY, THE MINIMUM SPICE LENGTH OF NOT LESS THAN 4db (db=BAR DIAMETER) WILL BE USED. SPICE LENGTH WILL BE INCREASED BY A FACTOR OF 1.5 FOR TOP REINFORCEMENT LAP WELDED WIRE FABRIC (W/WF) ONE SPACE PLUS 2 IN. ON ALL SIDES AT SPICES.
- CONCRETE SHALL RECEIVE THE FOLLOWING FINISHES:

INTERIOR EXPOSED SLABS	— STEEL TROWEL FINISH (UNO. OR REQUESTED BY THE OWNER)
EXTERIOR SLABS	— BROOM FINISH IN DIRECTION OF SLOPE
- MAINTAIN CONCRETE AFTER PLACEMENT WITH MINIMAL MOISTURE LOSS AT RELATIVELY CONSTANT TEMPERATURE FOR THE PERIOD NECESSARY FOR HYDRATION OF CEMENT AND HARDENING OF CONCRETE (NOT LESS THAN 1 DAYS). COMPLY WITH THE REQUIREMENTS OF ACI-308 STANDARD PRACTICE FOR CURING CONCRETE. AMERICAN CONCRETE INSTITUTE. A COMBINATION CURING AND SEALING COMPOUND SHALL BE APPLIED AFTER THE CONCRETE HAS BEEN FINISHED OR THE FORMS REMOVED. COMPOUND SHALL MEET THE REQUIREMENTS OF ASTM C108.

METAL STUD NOTES:

- ALL METAL STUDS SHALL BE FORMED FROM GALVANIZED STEEL WITH A MINIMUM YIELD STRENGTH OF 39,000 PSI, CONFORMING TO ASTM A446.
- STRUCTURAL STUDS SHALL BE C-SHAPED WITH A 1-5/8" FLANGE WIDTH AND 1" FLANGE RETURN L.P. SEE DRAWINGS FOR REQUIRED STD SIZES AND GAGES.
- GALVANIZED COATING SHALL MEET THE REQUIREMENTS OF ASTM A925, G-60.
- PROVIDE BRIDGING SPACED AT 5'-0" O.C. MAX. AND ATTACHED TO EACH STD WITH CLIPS.
- CONFORM TO AISI CODE, AISI CODE, AND ALL STATE AND LOCAL CODES.

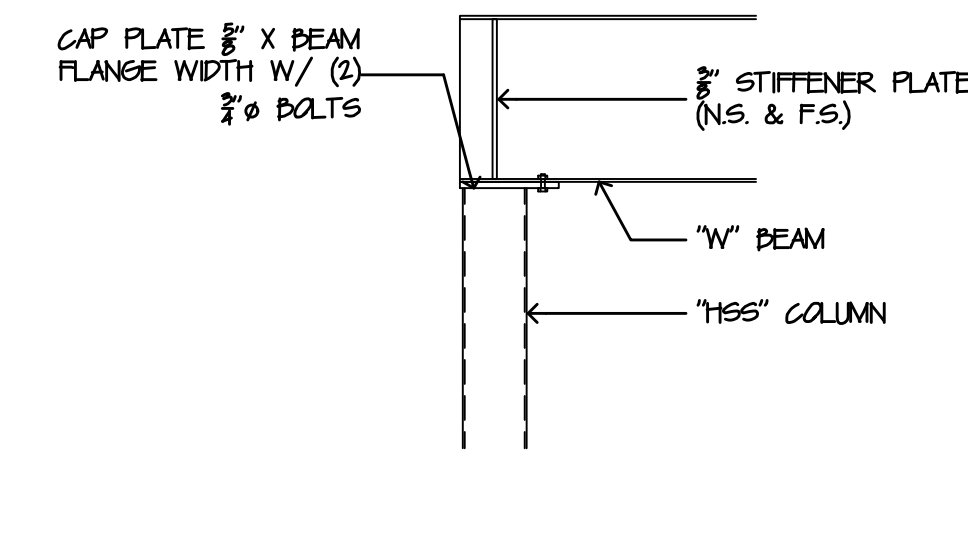
CONCRETE MASONRY

- MINIMUM 28-DAY COMPRESSIVE STRENGTH OF CONCRETE-MASONRY WALL BE FM = 1500 PSI.
- MORTAR WALL COMPLY WITH THE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY AND WALL BE OF THE FOLLOWING TYPE:

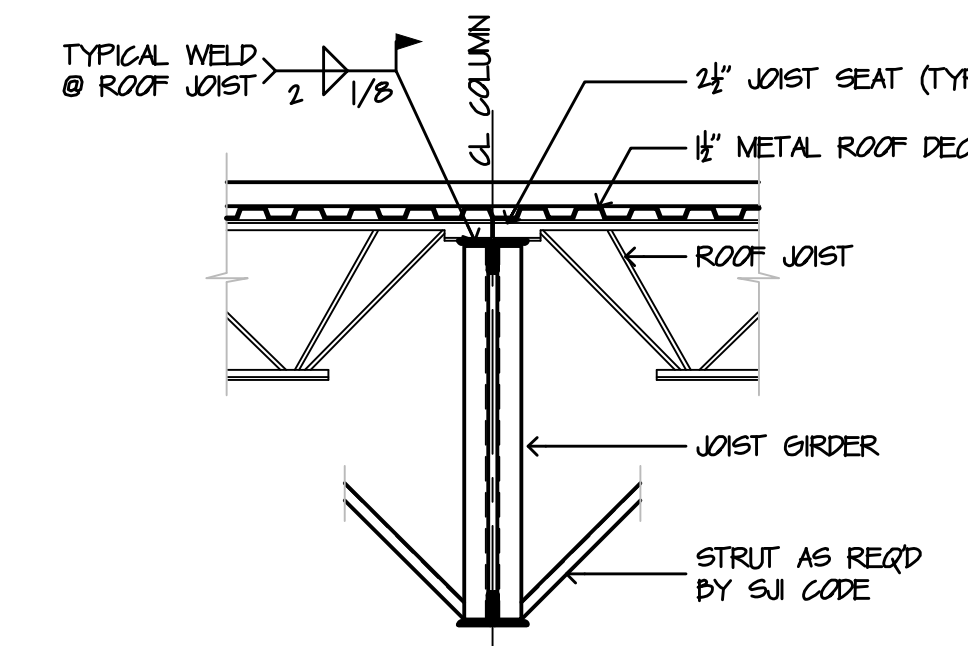
WALLS BELOW GRADE	TYPE M
BEARING WALLS	TYPE M OR S
- CONCRETE MASONRY UNITS SHALL BE GRADED WITH 2,500 PSI COARSE GRAIT AS SHOWN IN THE STRUCTURAL DOCUMENTS. GRAIT FOR REINFORCED AND NON-REINFORCED MASONRY WALL CONFORM TO ASTM C46.
- PROVIDE HORIZONTAL JOINT REINFORCEMENT WITH NO. 3 GAGE LONGITUDINAL WIRES AT 16 O.C. VERTICALLY, UNLESS NOTED OTHERWISE. PROVIDE SPECIAL ACCESSORIES FOR CORNERS, INTERSECTIONS, ETC.
- PROVIDE OPEN BOTTOM BEAM BLOCK UNITS WITH 9" DEEP MINIMUM WEED OPENINGS AT HORIZONTAL REINFORCEMENT LOCATIONS. A MINIMUM CLEAR SPACING OF ONE BAR DIAMETER WILL BE PROVIDED BETWEEN THE REINFORCING BARS AND THE FACE OF MASONRY UNITS.
- PROVIDE CONTROL JOINTS IN ALL CONCRETE MASONRY WALLS AT LOCATIONS APPROVED BY THE ARCHITECT AT A MAXIMUM SPACING OF 3 TIMES THE WALL LENGTH OR 40'-0", WHICHEVER IS LESS.
- PROVIDE DOWEL ANCHORS AT 16 O.C. UNLESS NOTED OTHERWISE, WERE MASONRY WALLS ABUT CONCRETE SURFACES.
- SUBMIT WRITTEN CONSTRUCTION PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
- MINIMUM VERTICAL WALL REINFORCEMENT SHALL BE # 5S @ 48" O.C. UNLESS NOTED OTHERWISE.

STRUCTURAL STEEL NOTES:

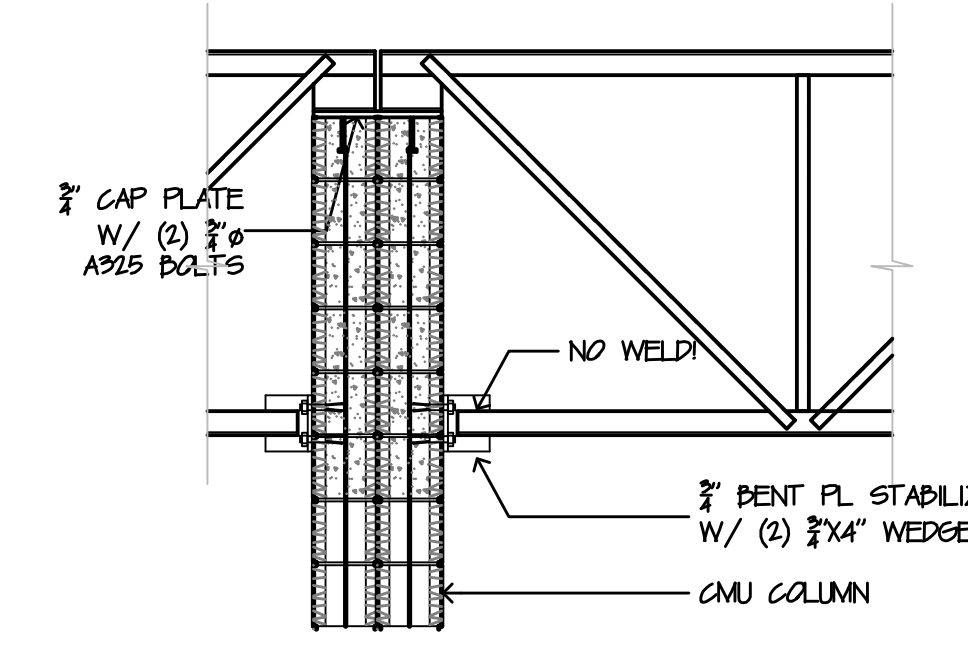
- FABRICATION AND ERECTION OF ALL STEEL TO BE IN ACCORDANCE WITH AISI SPECS.
- STRUCTURAL STEEL TO BE ASTM A992 UNLESS NOTED OTHERWISE. TUBE COLUMNS TO BE ASTM A500, GRADE B, WITH YIELD STRENGTH OF 46,000 PSI MIN.
- DOLTS TO BE ASTM A325-N UNLESS NOTED OTHERWISE. MINIMUM SIZE TO BE 3/4", UNLESS NOTED OTHERWISE.
- ANCHOR DOLTS SHALL MEET THE REQUIREMENTS OF ASTM 307.
- IN NO CASE SHALL THE STRENGTH OF THE FRAMED CONNECTION BE LESS THAN ONE-HALF THE ALLOWABLE UNIFORM LOAD FOR LATERALLY SUPPORTED BEAM AS SHOWN IN TABLES IN PART 2 OF THE AISI MANUAL.



A TYP. BEAM BEARING @ COLUMN
1/2" = 1'-0"



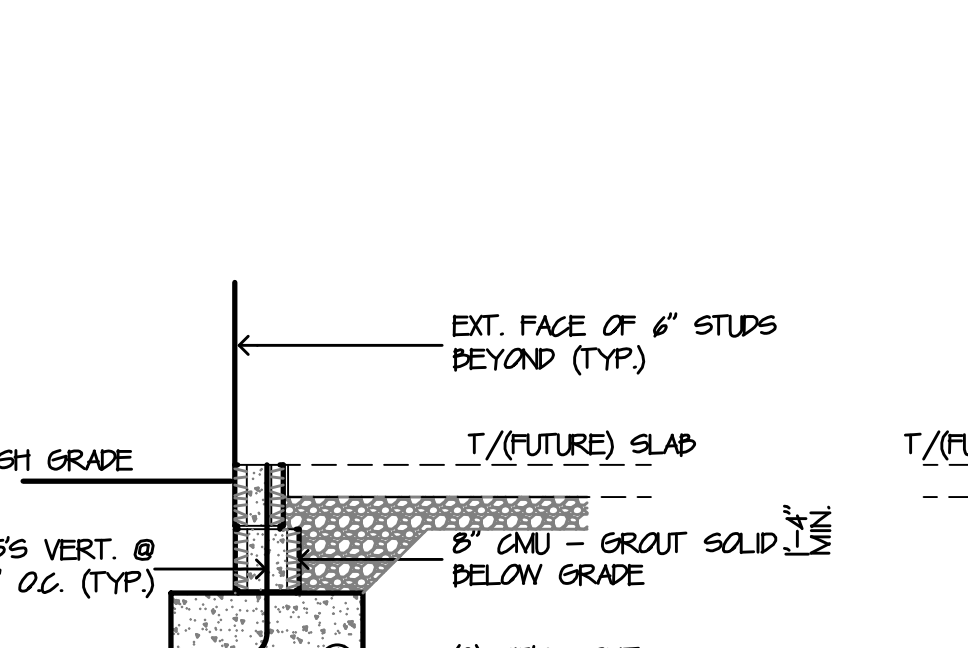
B TYP. BEAM BEARING @ COLUMN
1/2" = 1'-0"



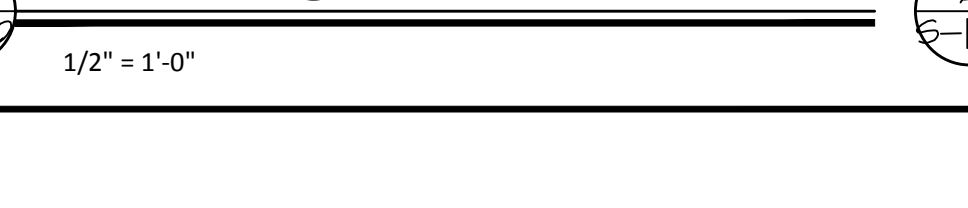
C ROOF JOISTS BEARING @ GIRDER
1/2" = 1'-0"



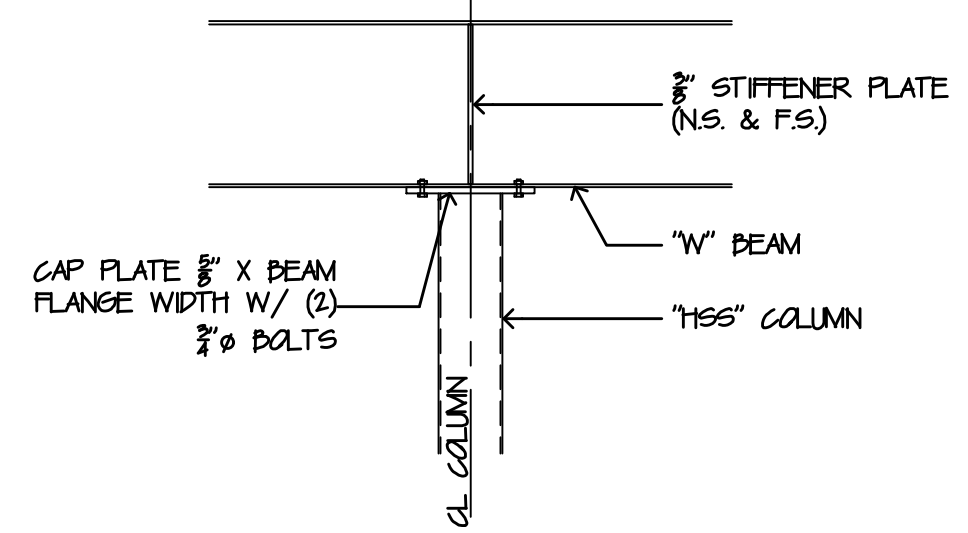
D TYP. JOIST GIRDER BEARING
1/2" = 1'-0"



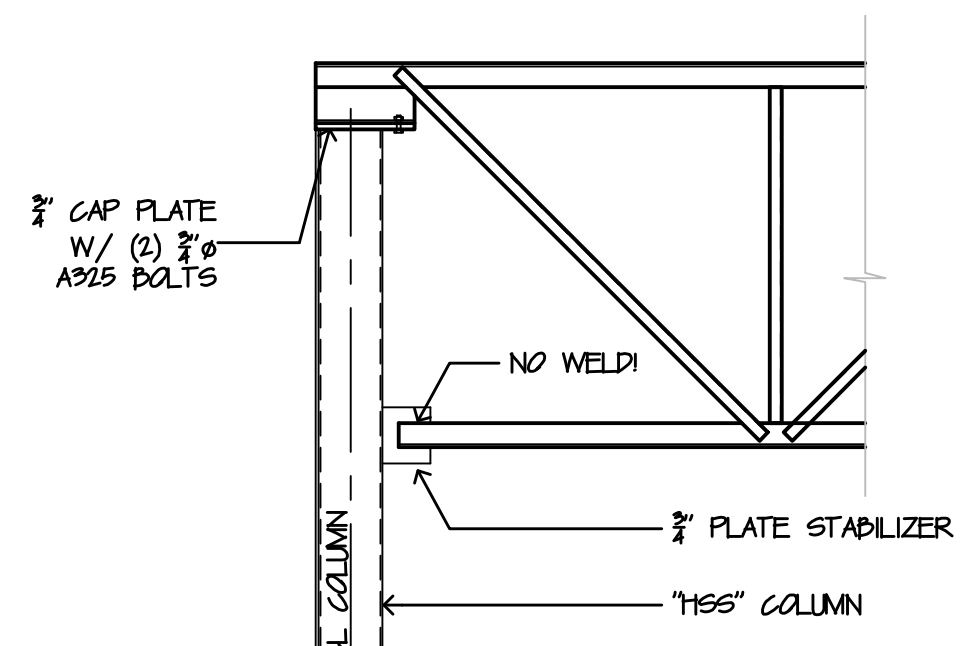
E GIRDER BEARING @ CMU
1/2" = 1'-0"



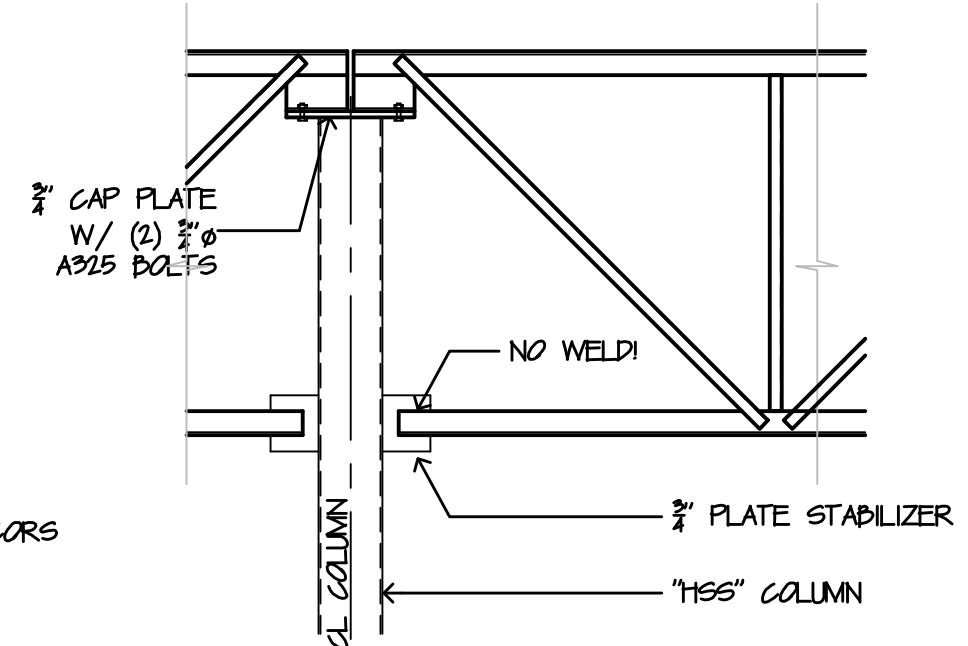
F TYP. JOIST GIRDER BEARING
1/2" = 1'-0"



G TYP. BEAM BEARING @ COLUMN
1/2" = 1'-0"



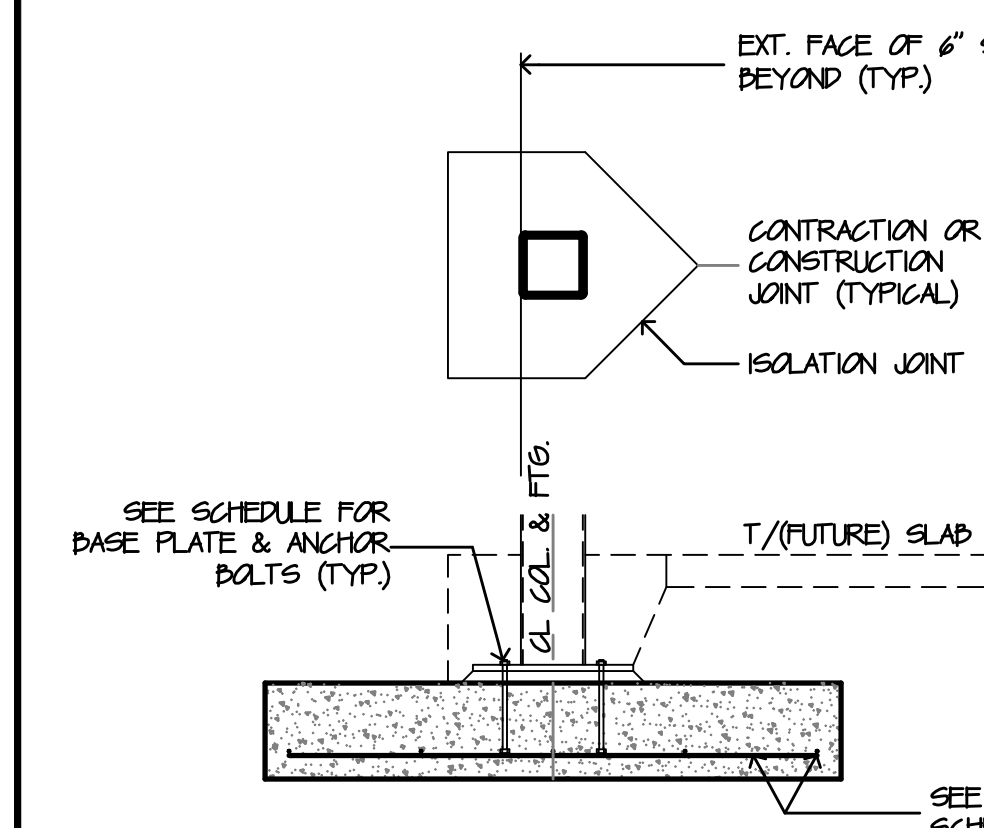
H TYP. JOIST GIRDER BEARING
1/2" = 1'-0"



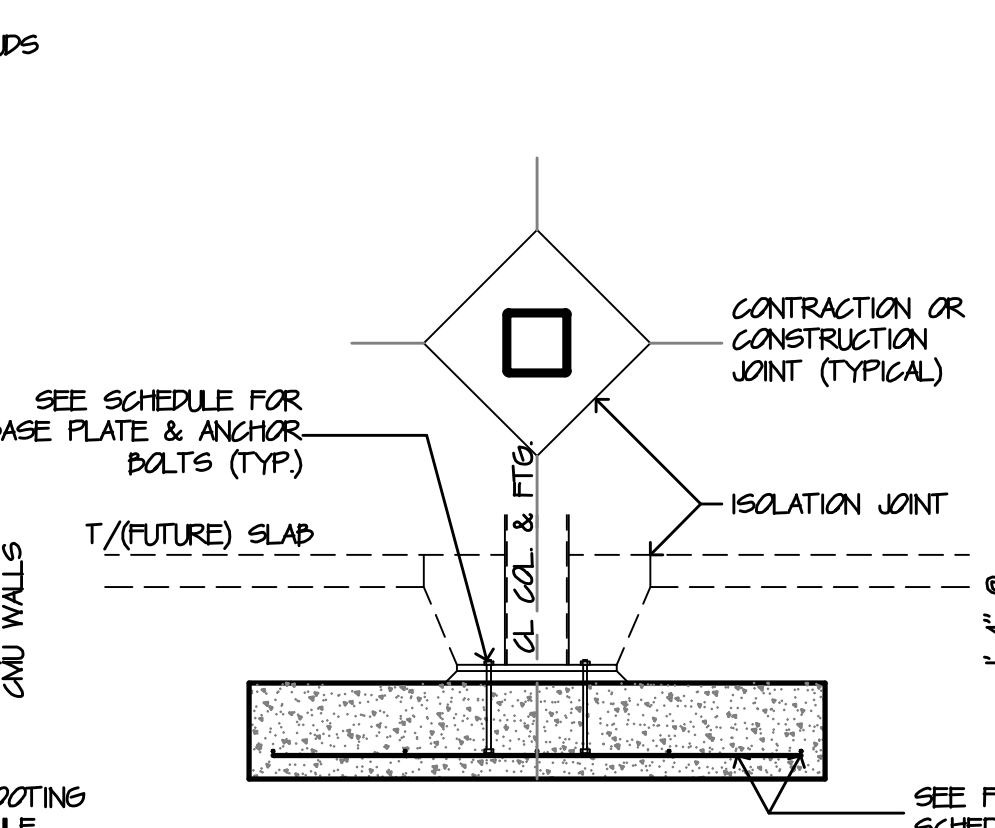
I GIRDER BEARING @ CMU
1/2" = 1'-0"



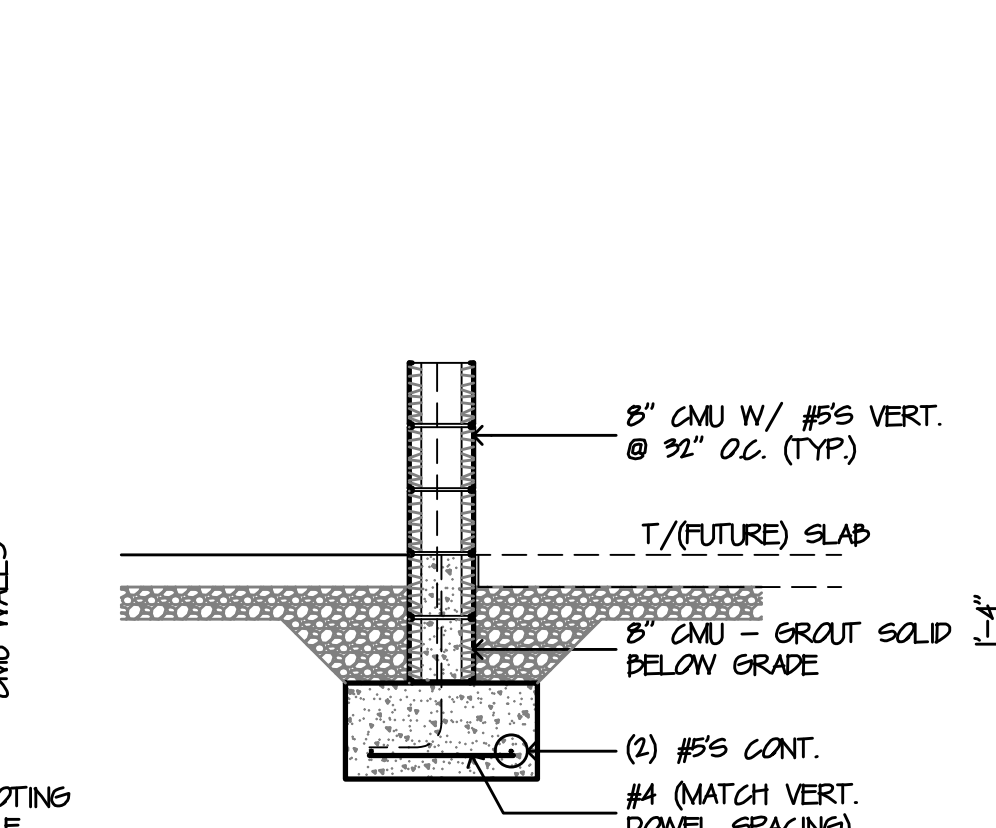
J TYP. JOIST GIRDER BEARING
1/2" = 1'-0"



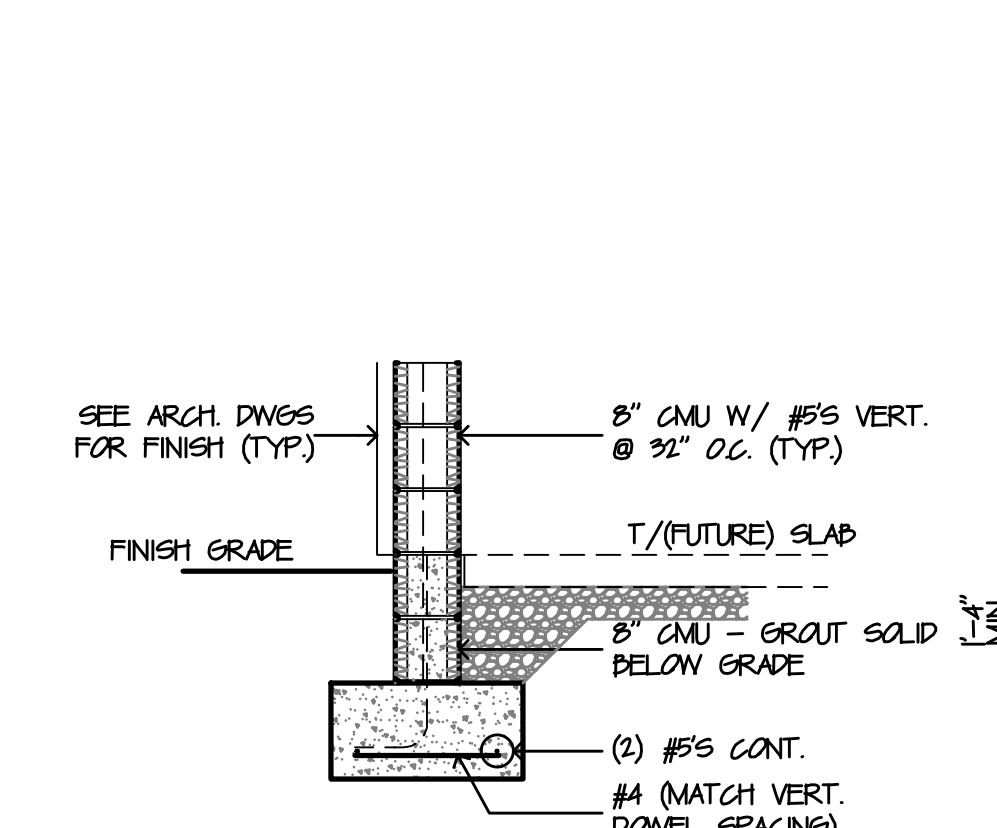
1 EXTERIOR COLUMN FOOTING
1/2" = 1'-0"



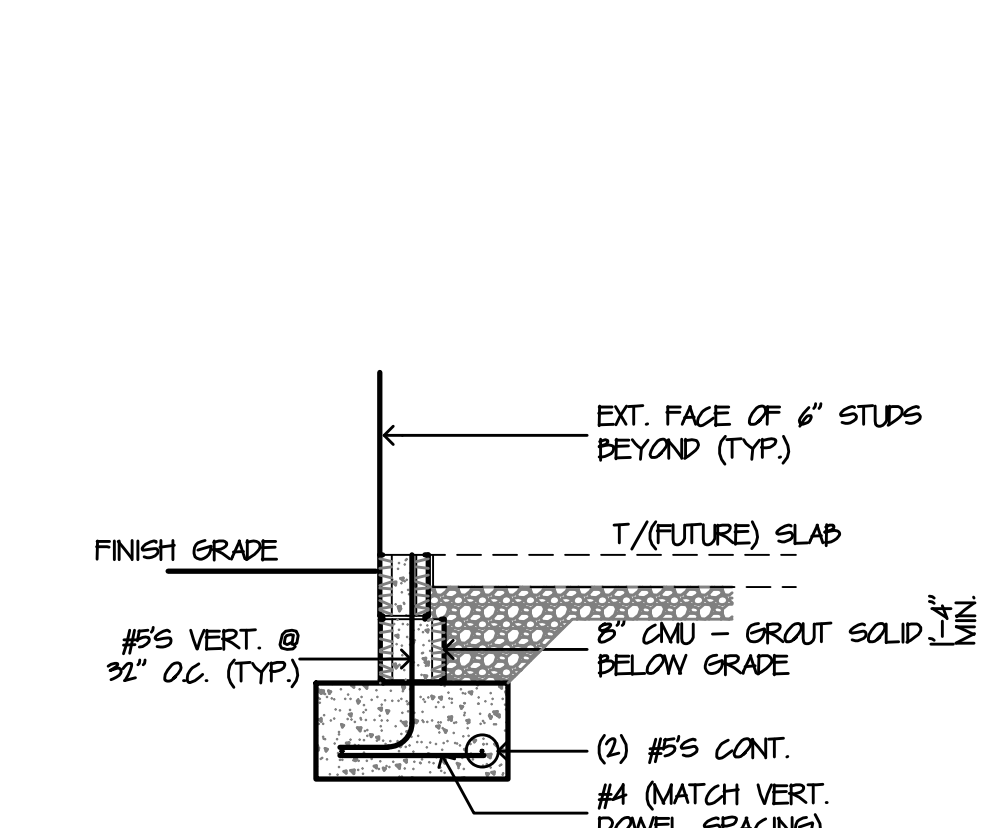
2 INTERIOR COLUMN FOOTING
1/2" = 1'-0"



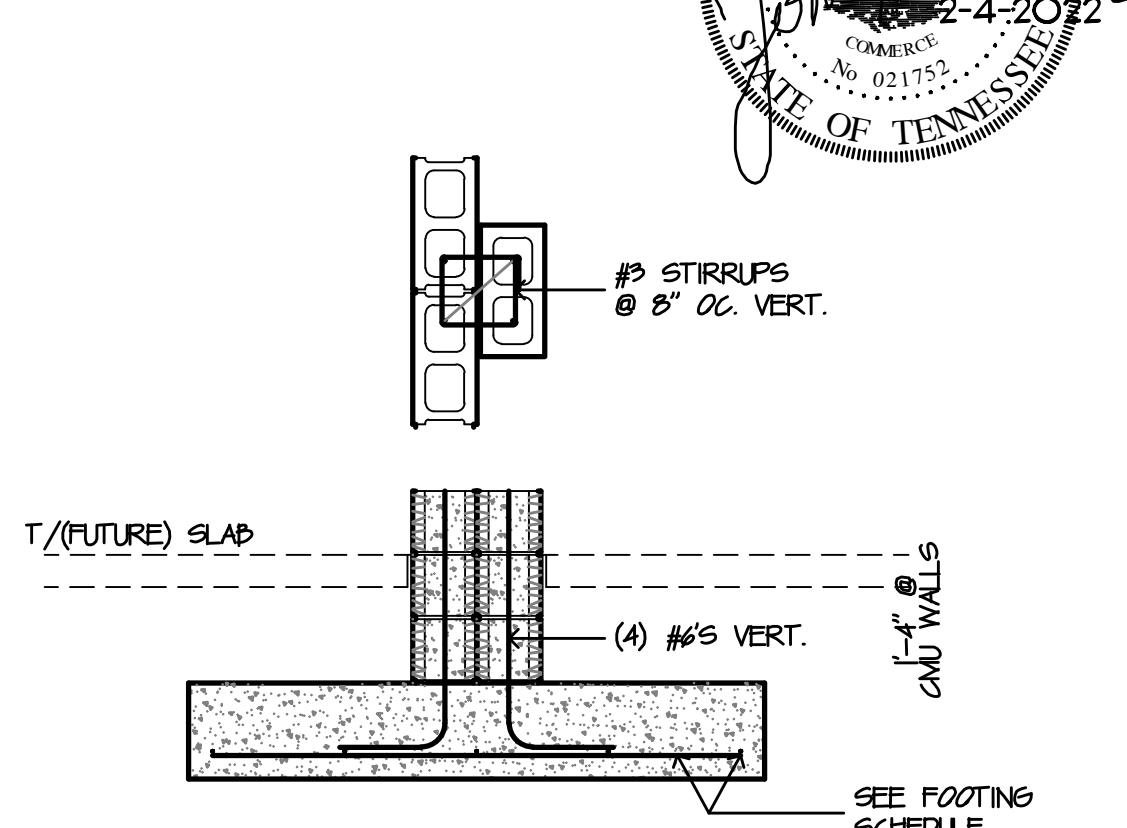
3 FOOTING @ INTERIOR WALL
1/2" = 1'-0"



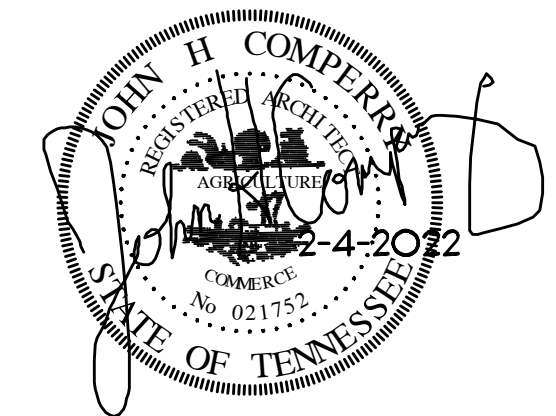
4 FOOTING @ EXTERIOR WALL
1/2" = 1'-0"



5 FOOTING @ EXTERIOR WALL
1/2" = 1'-0"



6 CMU COLUMN FOOTING
1/2" = 1'-0"

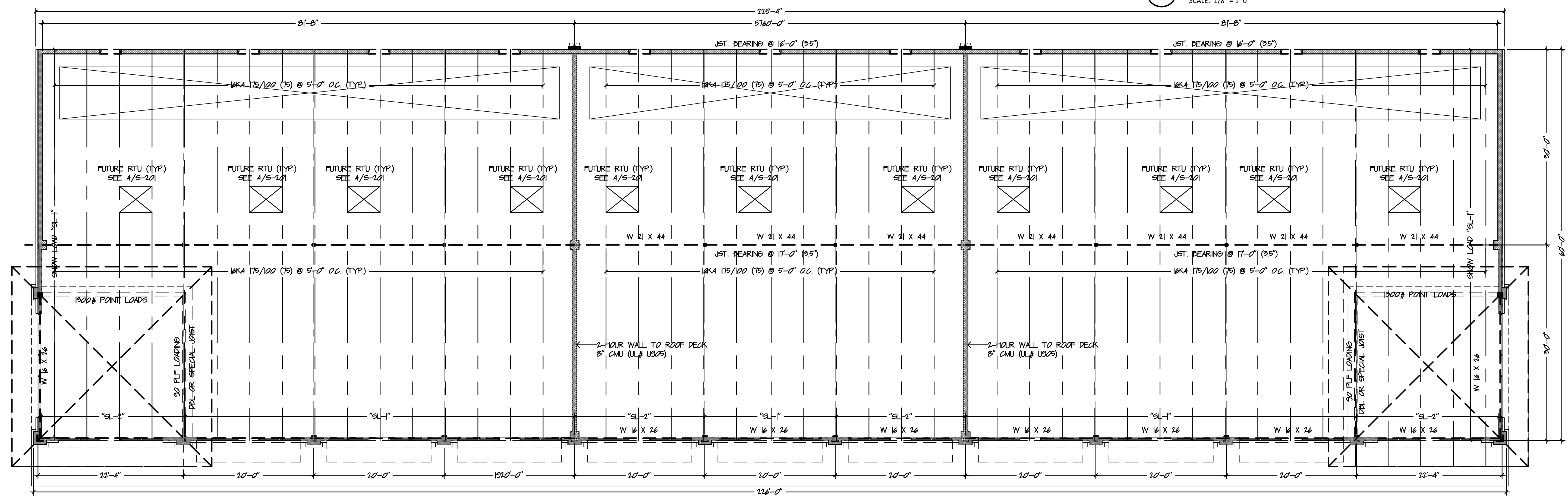
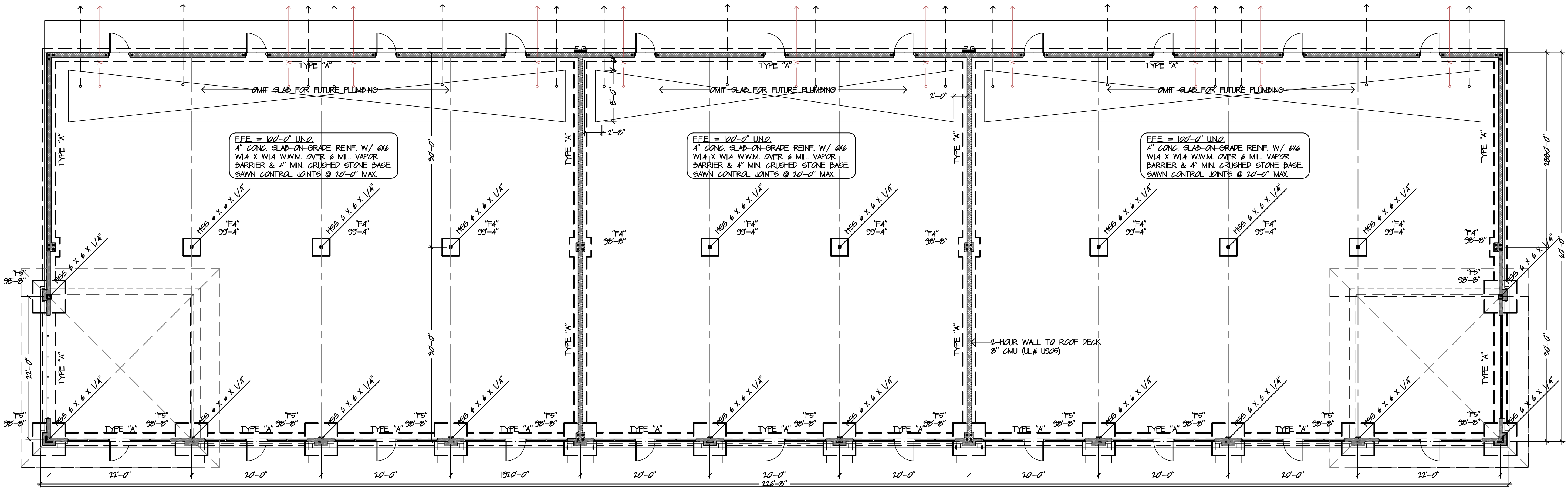


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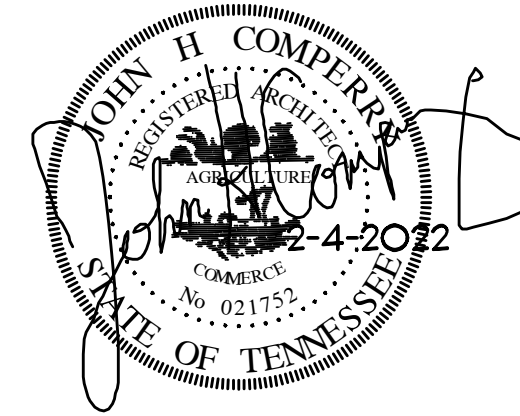
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COLUMN FOOTING & PIER SCHEDULE		
TYPE	SIZE (WIDTH X LENGTH X DEPTH)	REINF. (EACH WAY)
A'	2'-0" X CONT. X 1'-0"	2-#5 CONT.
F-3	3'-0" X 3'-0" X 1'-0"	4-#5 X 2'-6"
F-4	4'-0" X 4'-0" X 1'-0"	5-#5 X 3'-6"
F-5	5'-0" X 5'-0" X 1'-0"	6-#5 X 4'-6"
F-6	6'-0" X 6'-0" X 1'-0"	8-#5 X 5'-6"
F-8	8'-0" X 8'-0" X 1'-4"	8-#6 X 7'-6"

BASE PLATE & ANCHOR BOLT SCHEDULE								
COLUMN SIZE	BASE PLATE				ANCHOR BOLTS			
	"A"	"B"	"C"	"D"	"E"	"F"	"G"	# REQD
HSS 4 X 4 X 1/4"	1/2"	10"	10"	3-1/2"	3-1/2"	3/4"	1'-0"	4
HSS 6 X 6 X 1/4"	3/4"	12"	12"	4-1/2"	4-1/2"	3/4"	1'-0"	4
HSS 8 X 8 X 3/8"	3/4"	14"	14"	5-1/2"	5-1/2"	3/4"	1'-4"	4

STANDARD LINTEL SCHEDULE		
WALL SIZE	LINTEL TYPE	REMARKS
8" CMU	8" X 8" BOND BEAM W/ (1) #5 T & B	OPENINGS UP TO 6'-4" WIDE
8" CMU	8" X 24" BOND BEAM W/ (2) #5 T & B	OPENINGS UP TO 11'-4" WIDE
6" STLD	DBL 6CSJ X 16 GA. BY DIETRICH IND. W/ 6" TRACK T & B	OR APPROVED EQUAL
8" STLD	DBL 8CSJ BY DIETRICH IND. W/ 8" TRACK T & B	OR APPROVED EQUAL, MATCH STLD GA.



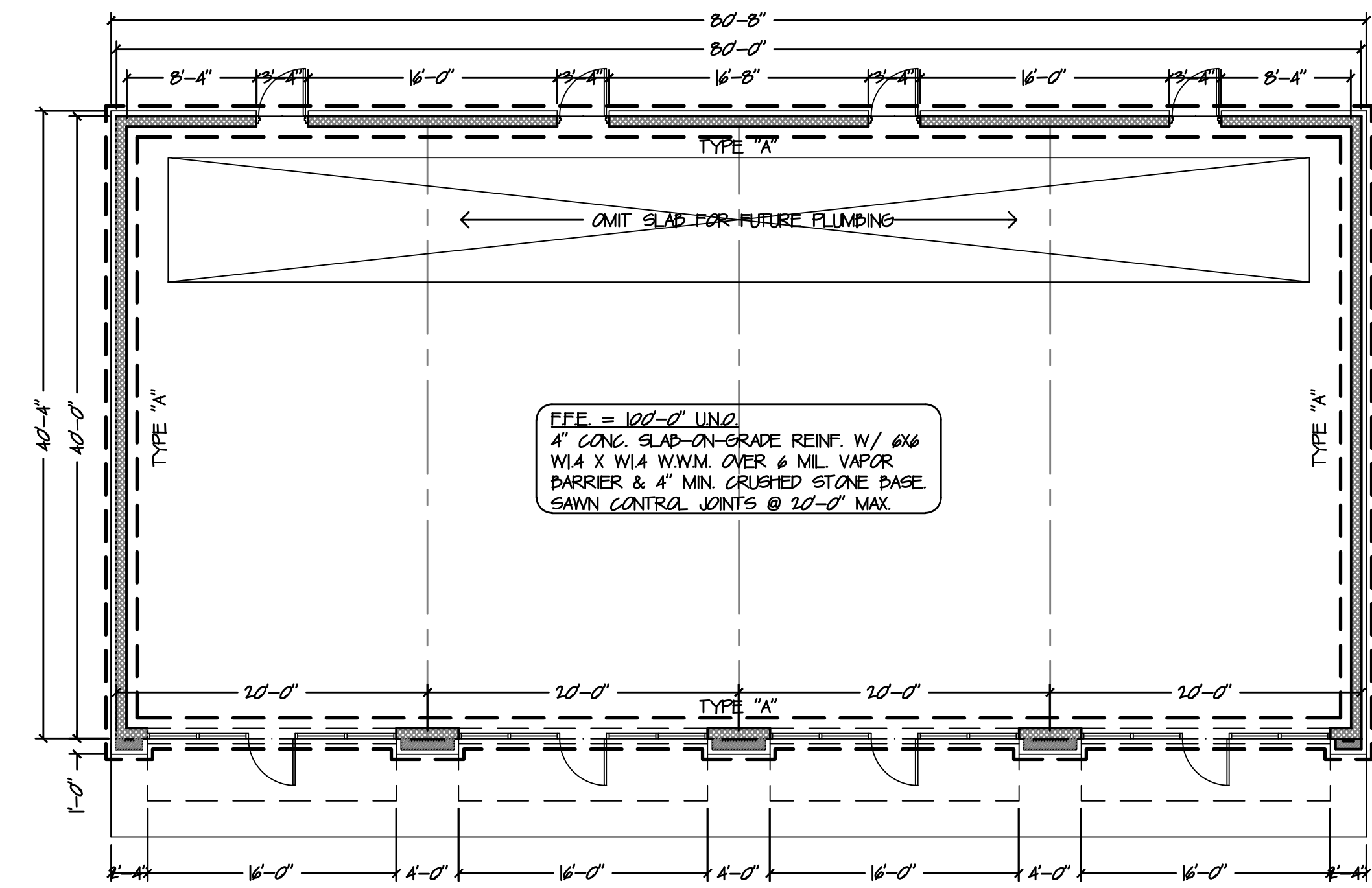
COLUMN FOOTING & PIER SCHEDULE		
TYPE	SIZE (WIDTH X LENGTH X DEPTH)	REINF. (EACH WAY)
A	2'-0" X CONT. X 1'-0"	2-#5 CONT.
F-3	3'-0" X 3'-0" X 1'-0"	4-#5 X 2'-6"
F-4	4'-0" X 4'-0" X 1'-0"	5-#5 X 3'-6"
F-5	5'-0" X 5'-0" X 1'-0"	6-#5 X 4'-6"
F-6	6'-0" X 6'-0" X 1'-0"	6-#5 X 5'-6"
F-8	8'-0" X 8'-0" X 1'-4"	8-#6 X 7'-6"

BASE PLATE & ANCHOR BOLT SCHEDULE								
COLUMN SIZE	BASE PLATE				ANCHOR BOLTS			
	A	B	C	D	E	F	G	# REQ'D
HSS 4 X 4 X 1/4"	1/2"	10"	10"	3-1/2"	3-1/2"	3/4"	1'-0"	4
HSS 6 X 6 X 1/4"	3/4"	12"	12"	4-1/2"	4-1/2"	3/4"	1'-0"	4
HSS 8 X 8 X 3/8"	3/4"	14"	14"	5-1/2"	5-1/2"	3/4"	1'-4"	4

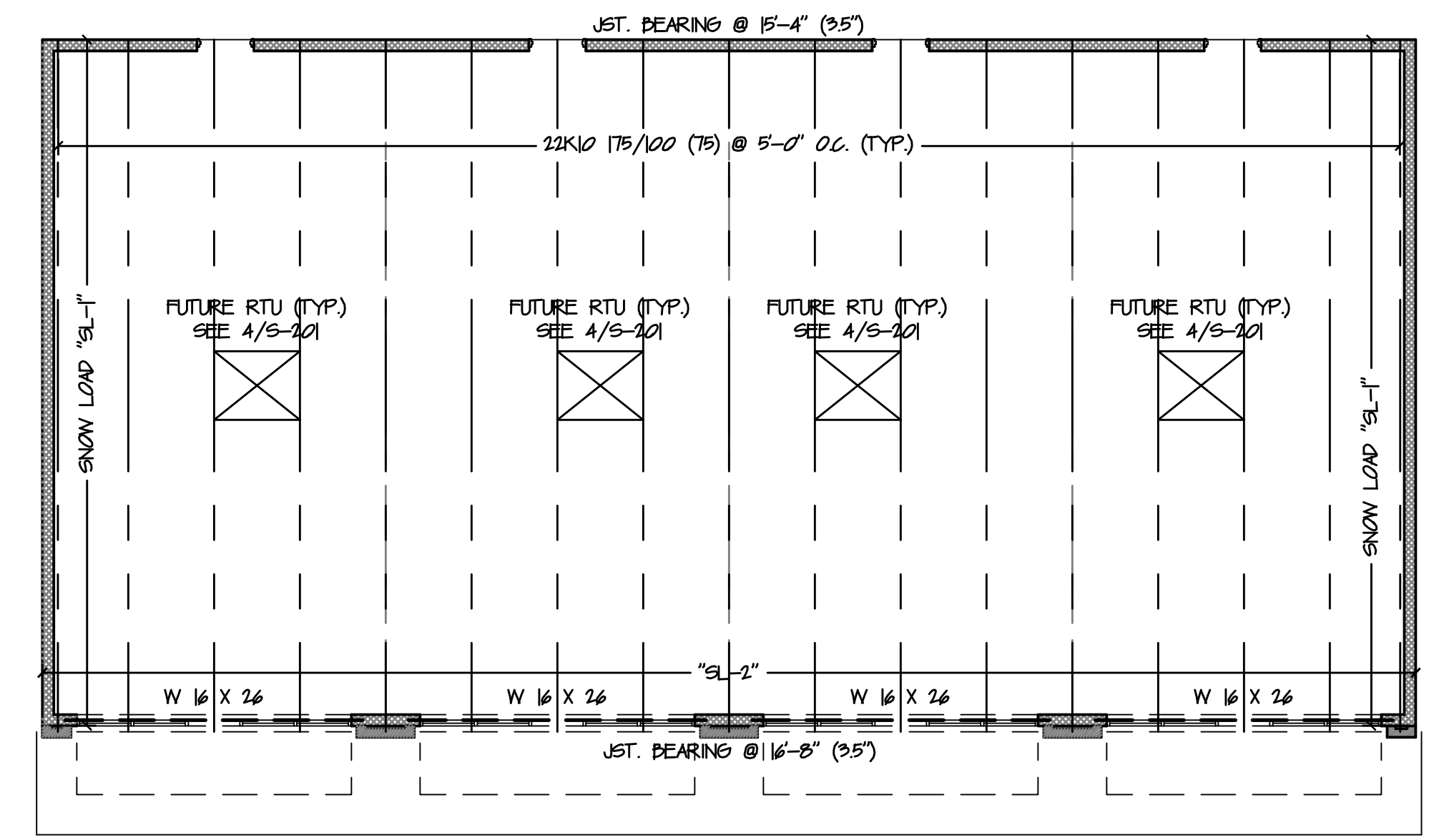
NOTE: ALL ANCHOR BOLTS SHALL HAVE 4" THREADED PROJECTION

STANDARD LINTEL SCHEDULE		
WALL SIZE	LINTEL TYPE	REMARKS
8" CMU	8" X 8" BOND BEAM W/ (1) #5 T & B	OPENINGS UP TO 6'-4" WIDE
8" CMU	8" X 24" BOND BEAM W/ (2) #5 T & B	OPENINGS UP TO 11'-4" WIDE
6" STD	DBL. 6CSJ X 16 GA. BY DIETRICH IND. W/ 6" TRACK T & B	OR APPROVED EQUAL
8" STD	DBL. 8CSJ BY DIETRICH IND. W/ 8" TRACK T & B	OR APPROVED EQUAL, MATCH STD. GA.

NOTES:
 1. ALL LINTELS SHOWN IN NOMINAL SIZES
 2. LINTELS SHALL BE PROVIDED FOR MECH. & ARCH. OPENINGS.



A FOUNDATION PLAN
 SCALE: 1/8" = 1'-0"



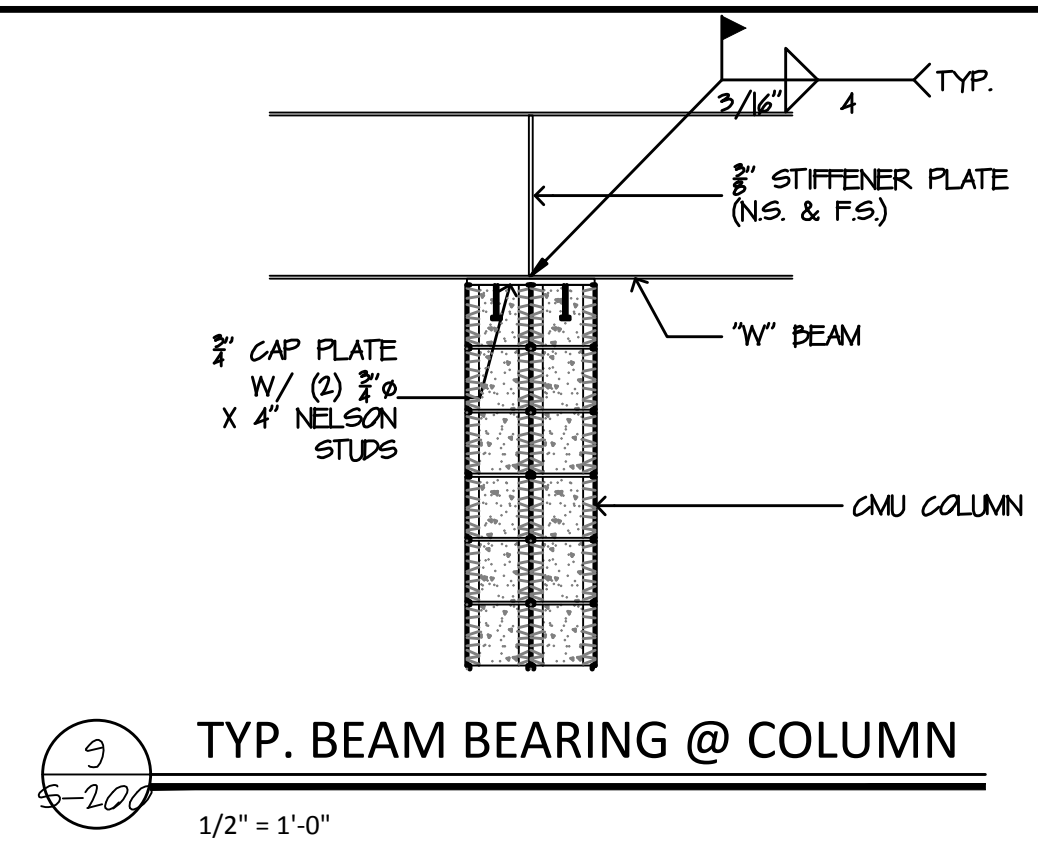
B ROOF FRAMING PLAN
 SCALE: 1/8" = 1'-0"

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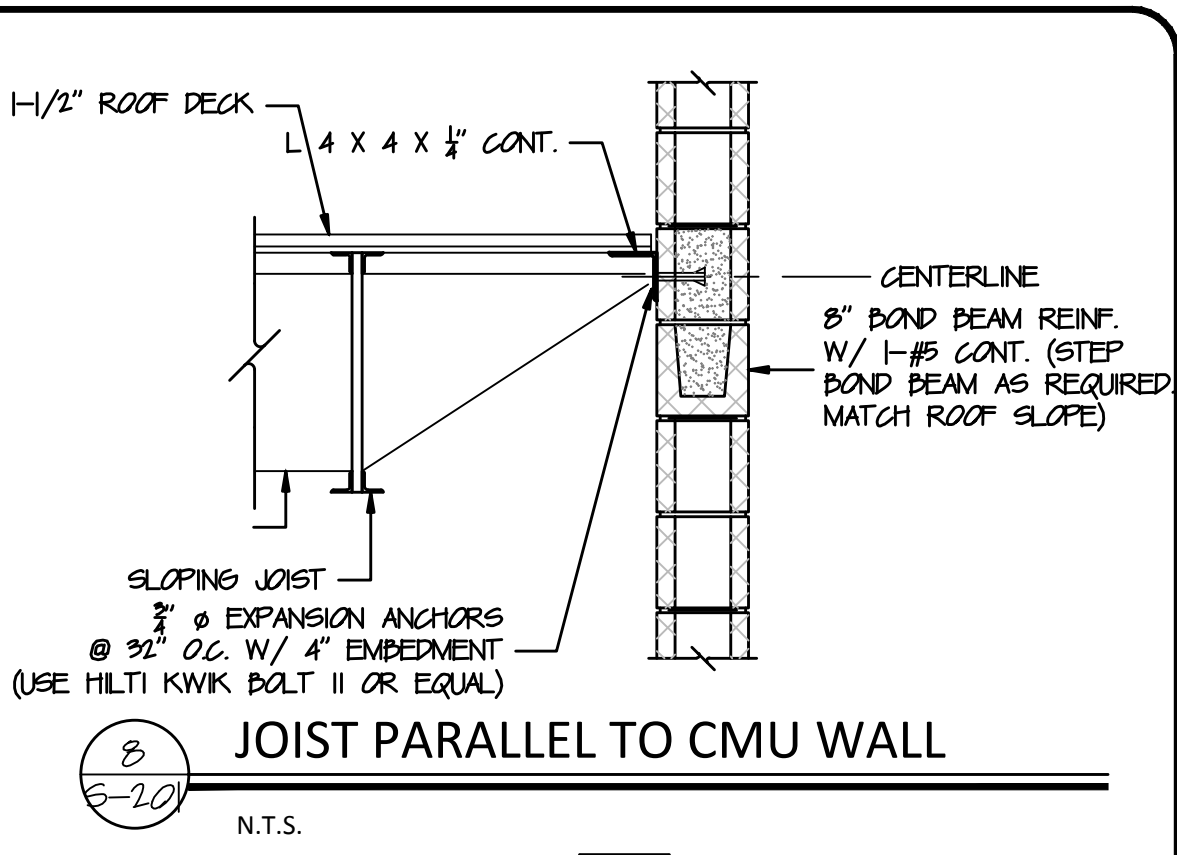
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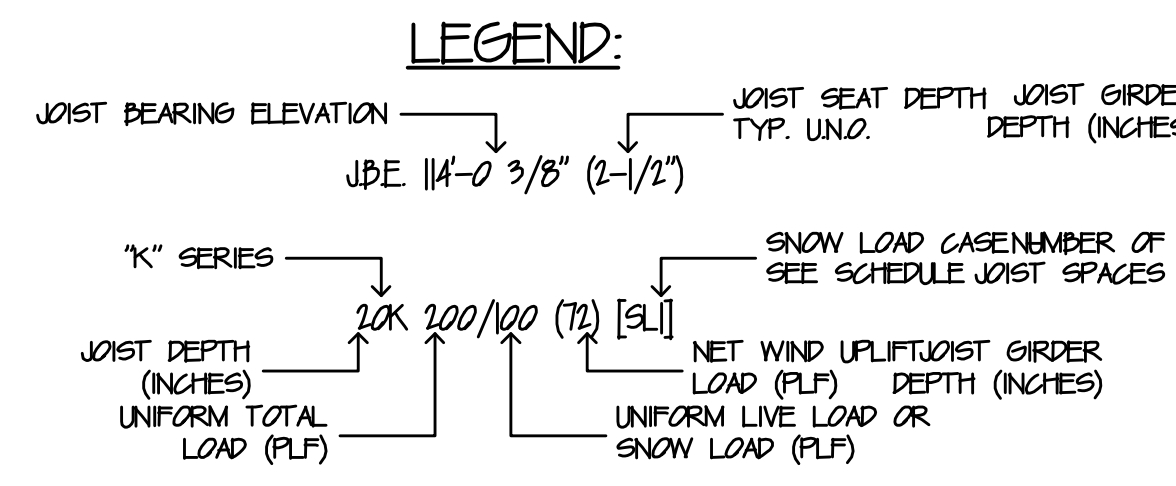
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S-201	
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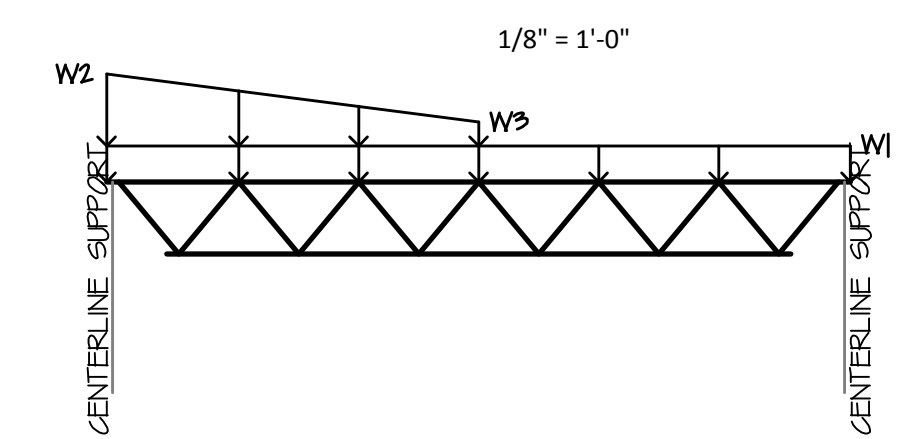
9
 5-20
 TYP. BEAM BEARING @ COLUMN
 1/2" = 1'-0"



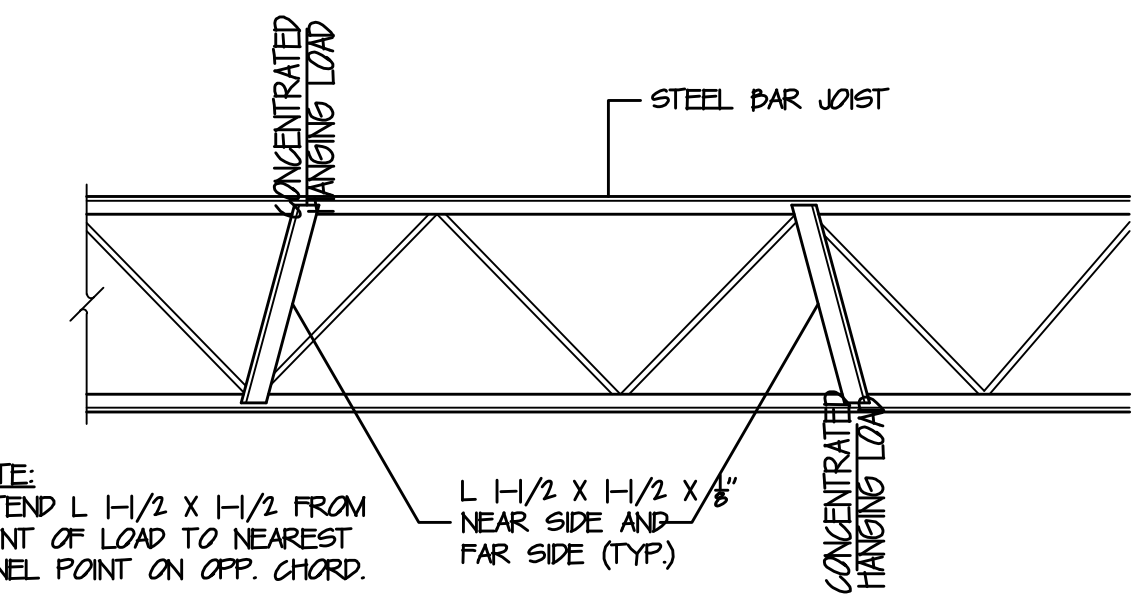
8
 5-20
 JOIST PARALLEL TO CMU WALL
 N.T.S.



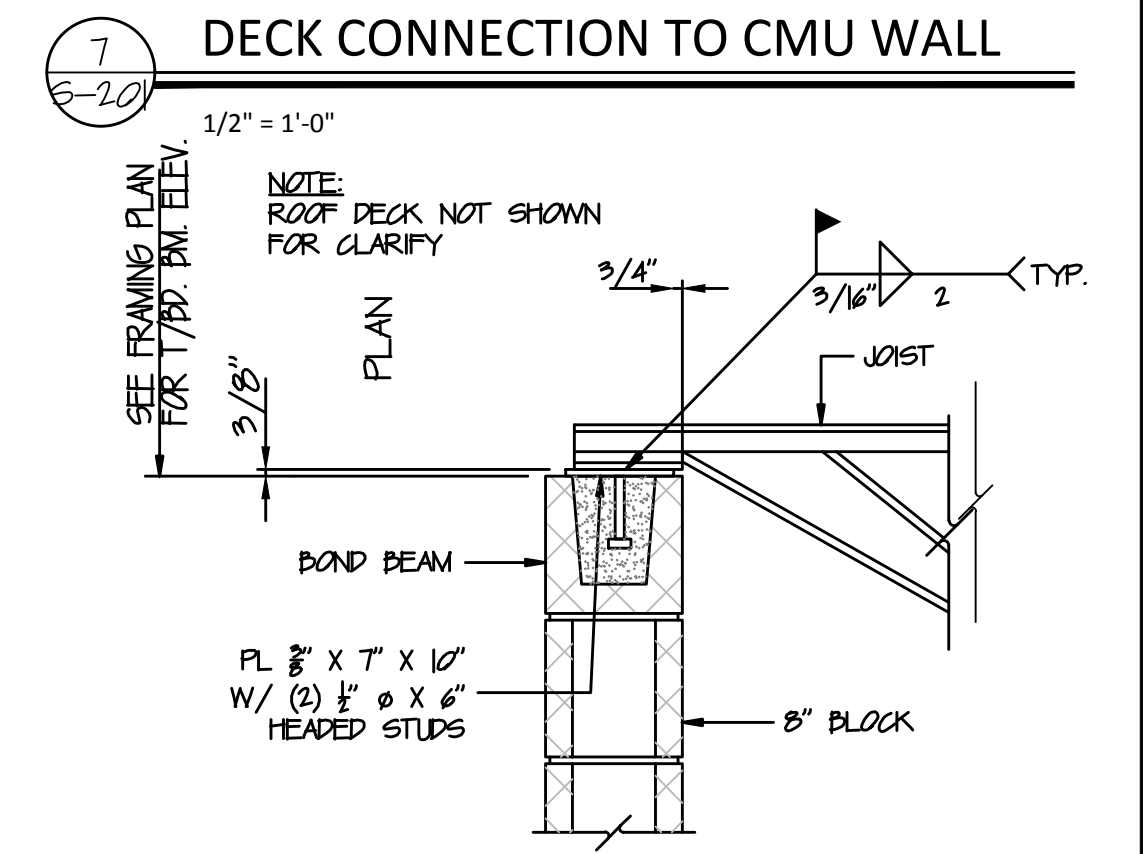
SNOW LOAD CASE	WIDEAD LOAD +SNOW PLF	W2-SNOW LOAD PLF	W3-SNOW LOAD PLF	SNOW-SNOW DISTANCE A (FEET)
SL-1	175	146	0	100
SL-2	182	152	0	100



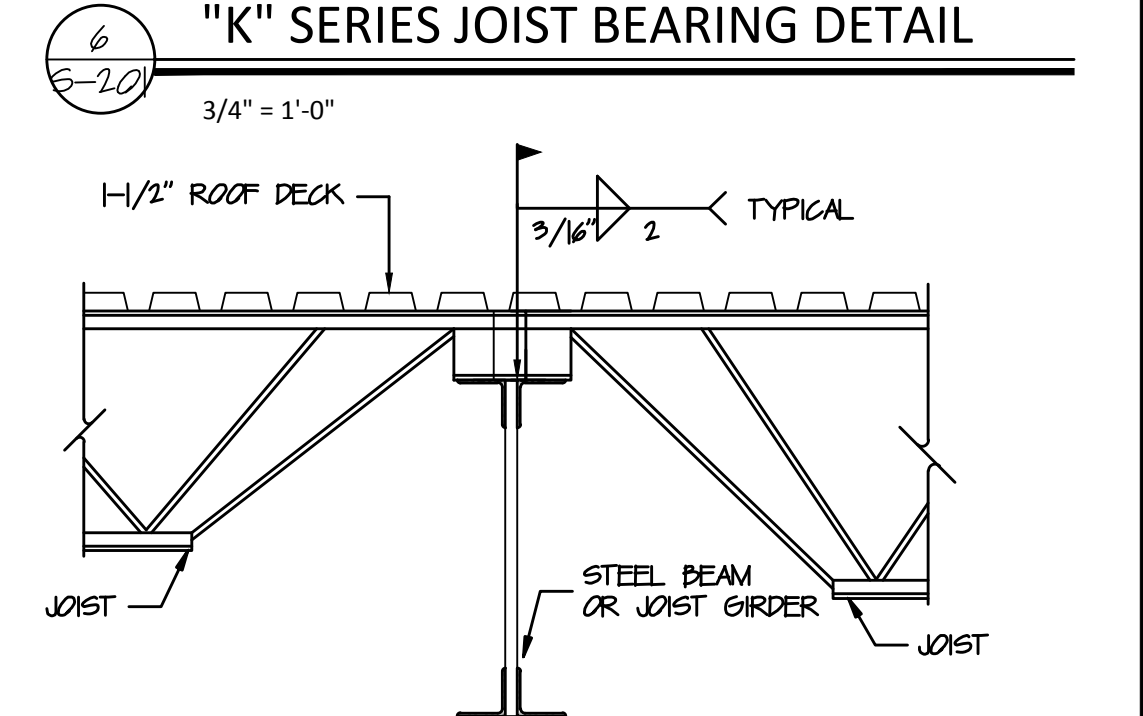
- GENERAL NOTES:
 1. SEE SHEET S-100 FOR GENERAL NOTES.
 2. SEE ARCH. DRAWINGS FOR DIMENSIONS AND DETAILS NOT SHOWN.
 3. UNL. - "UNLESS NOTED OTHERWISE"
 4. JOIST MANUFACTURER SHALL DESIGN ALL ROOF JOISTS FOR AN ADDITIONAL 500 LB. CONCENTRATED LOAD LOCATED AT ANY POINT ALONG THE BOTTOM CHORD.



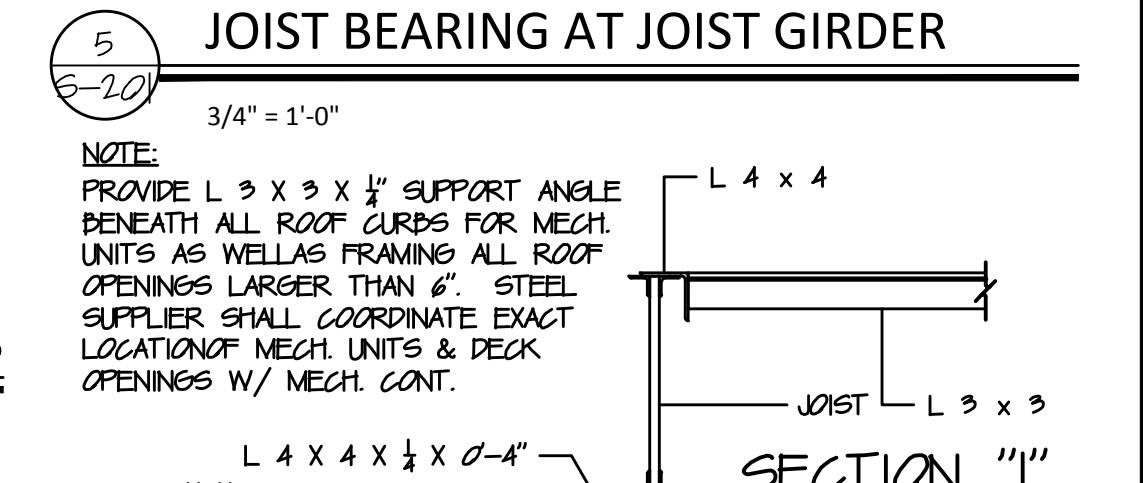
3
 5-20
 JOIST REIN. DET. FOR +200 LB LOADS
 N.T.S.



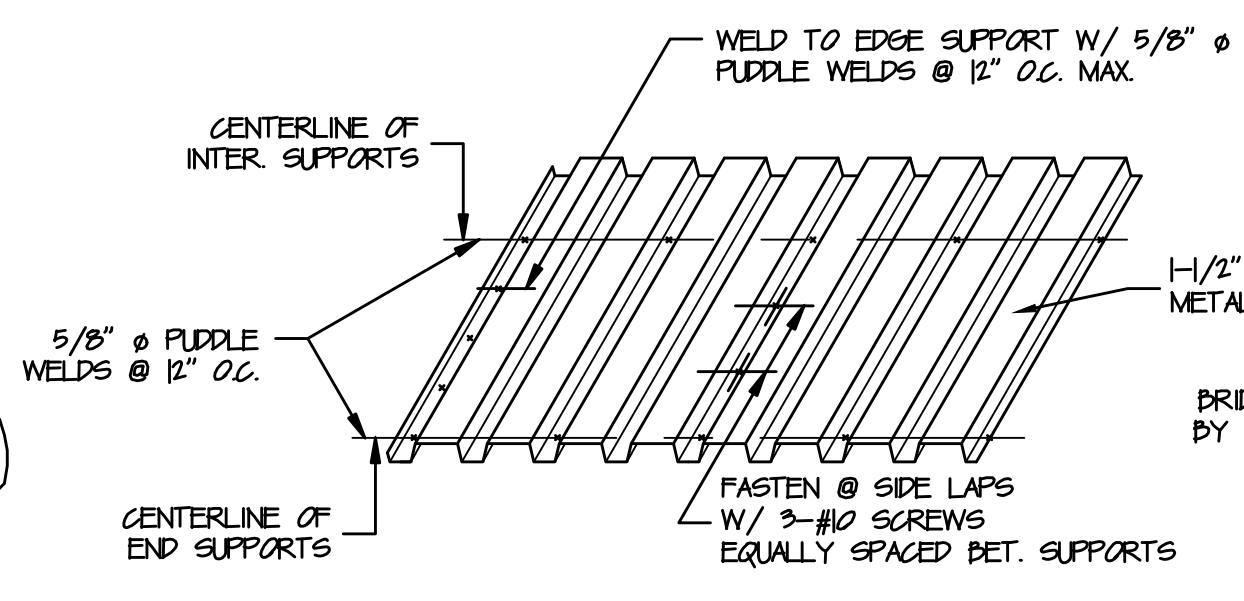
7
 5-20
 DECK CONNECTION TO CMU WALL
 1/2" = 1'-0"



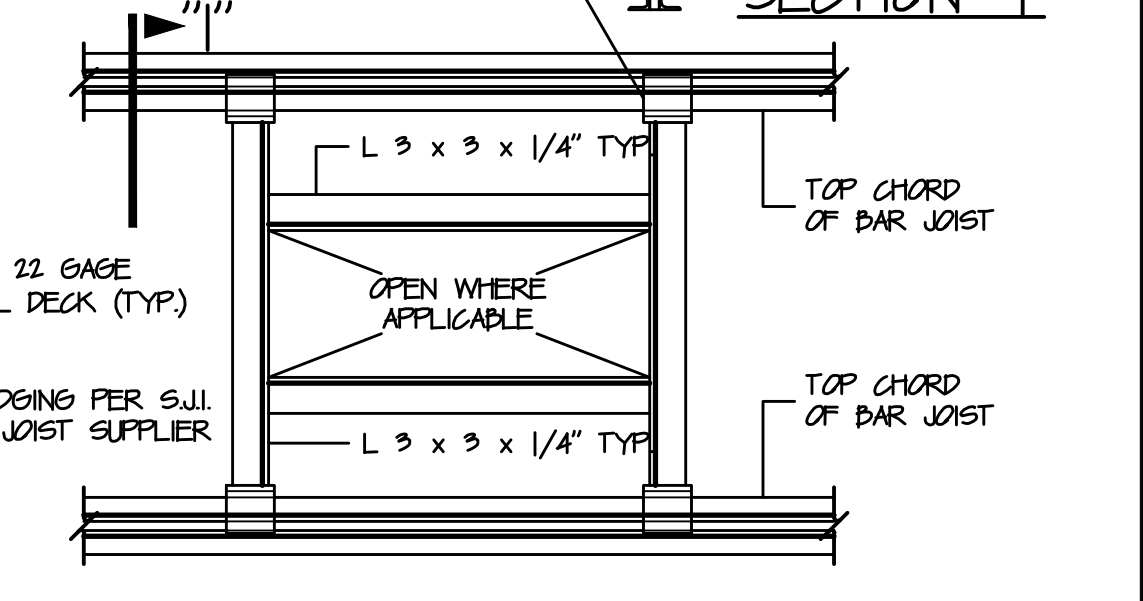
6
 5-20
 "K" SERIES JOIST BEARING DETAIL
 3/4" = 1'-0"



5
 5-20
 JOIST BEARING AT JOIST GIRDER
 3/4" = 1'-0"



2
 5-20
 ROOF DECK ATTACHMENT PATTERN
 N.T.S.



4
 5-20
 ROOF OPENING DETAIL
 N.T.S.

