



GROUNDED

Engineering Services

Report of Limited Geotechnical Engineering Services

PROJECT:

Dixon Drive Development
438 Dixon Drive
Cleveland, White County, Georgia
Project No.: 25MAX001

PREPARED FOR:

Maximum Development General Contractors
4607 Cromwell Court
Flowery Branch, Georgia 30542

ATTENTION:

Mr . Zach Lurie

Physical Address:

4148 Industry Way, Suite E
Flowery Branch, Georgia 30542

December 4, 2025



GROUNDDED
Engineering Services

December 4, 2025

Maximum Development General Contractors
4607 Cromwell Court
Flowery Branch, Georgia 30542

VIA EMAIL to: zach.lurie@outlook.com

Attention: Mr. Zach Lurie

***Report of Limited Geotechnical Engineering Services
Dixon Drive Development
438 Dixon Drive
Cleveland, White County, Georgia
Project No.: 25MAX001***

Dear Mr. Lurie:

As instructed, ***GROUNDDED Engineering Services, LLC (GROUNDDED)*** has completed the Limited Geotechnical Engineering Services for the above referenced site in general accordance with GROUNDDED Proposal No.: 25MAX001 dated November 11, 2025. The results, together with our conclusions and recommendations, are to be found in the accompanying report, which is being transmitted herewith.

Often times questions arise concerning subsurface conditions due to design modifications and challenges related to construction that occur during a project. GROUNDDED would be pleased to continue its role as Geotechnical Engineer during the project implementation.

Should any questions arise regarding this report, or if we may be of further service in any manner, please do not hesitate to contact us at your convenience.

Respectfully submitted,
GROUNDDED Engineering Services, LLC

Christopher J. Settles, PE
Principal

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TABLE OF CONTENTS

1.0 INTRODUCTION1

1.1 Authorization..... 1

1.2 Purpose 1

1.3 Scope of Services 1

1.4 General..... 1

1.5 Report Reliance 2

2.0 DESCRIPTION OF SITE.....2

3.0 FIELD EXPLORATION & TESTING3

3.1 Scope 3

3.2 Test Pit Excavations 3

3.3 Groundwater Level Measurements..... 3

4.0 SUBSURFACE CONDITIONS3

4.1 General..... 3

4.2 Soil Conditions 4

 4.2.1 Initial Surface Layer 4

 4.2.2 Residual Soils 4

 4.2.3 Partially Weathered Rock and/or Refusal Materials 4

4.3 Groundwater Conditions..... 5

5.0 DISCUSSION AND RECOMMENDATIONS5

5.1 Project Description 5

5.2 Site Preparation 6

5.3 Buried Debris and Organics 6

5.4 Sub-Surface Materials 7

 5.4.1 Residual Soils 7

 5.4.2 Partially Weathered Rock/Refusal Materials..... 7

5.5 Structural Fill Soils 8

5.6 Groundwater 9

5.7 Earthen Slopes 9

6.0 LIMITATIONS10

7.0 GENERAL COMMENTS.....10

8.0 SIGNATURES OF GEOTECHNICAL PROFESSIONALS11

APPENDIX

Test Pit Location Plan

Test Pit Logs (13)

1.0 INTRODUCTION

1.1 Authorization

This Limited Report of Geotechnical Engineering Services presents the results of our exploration, field observations, recommendations and conclusions for the proposed residential development located at 438 Dixon Drive, in Cleveland, White County, Georgia.

Our services were performed for Maximum Development General Contractors in general accordance with our proposal number 25MAX001 dated November 11, 2025, with signature acceptance of our proposal provided by Mr. Zach Lurie on November 11, 2025.

1.2 Purpose

The purpose of this exploration was to observe and evaluate the soil and groundwater conditions at the site. This report has been prepared to detail the observations and findings of our exploration as well as to present the conclusions and pertinent recommendations relating to the planned earthwork activities.

1.3 Scope of Services

The scope of services included a reconnaissance of the subject site, the subsurface exploration, field testing, and engineering analysis and review of the subsurface conditions encountered.

No environmental assessments of the site for wetlands, hazardous or toxic materials, on the ground, in the water, or in the air with regard to the subject property were included in this exploration. It should also be noted that any statements in this report or on the Test Pit Logs regarding odors, colors, or other suspicious items or conditions is for the information of the client only.

1.4 General

The general subsurface conditions used in the analysis were based upon interpolation of the subsurface data between the test pit excavations. There is a possibility that varying conditions may be encountered between test pit locations. If deviations from the noted subsurface conditions are encountered during construction, they should be brought to the attention of the Geotechnical Engineer.

The findings, conclusions, and recommendations or other professional advice have been prepared and presented in general accordance with accepted and customary professional engineering practices in the field of geotechnical engineering and engineering geology. No other warranties are implied or expressed.

Test pit locations were provided to GROUNDED as detailed on the annotated Concept Plan provided by Mr. Zach Lurie via email November 10, 2025. The recommendations submitted for the proposed development are based on the available soil information coupled with our understanding of the development based on the provided concept plan.

Any revision in the plans for the proposed development, from those described in this report, should be brought to the attention of the Geotechnical Engineer to determine if changes in the earthwork recommendations are required.

1.5 Report Reliance

This report has been prepared for the exclusive use of Maximum Development General Contractors for specific application to the planned residential development located at 438 Dixon Drive in Cleveland, White County, Georgia. Use of this report by any other party is not intended, and without explicit written authorization from GROUNDED, is done so at the risk of that party. No warranties or other representations are made to any other party, either expressed or implied.

2.0 DESCRIPTION OF SITE

The subject property is approximately 9.6-acres and is located at 438 Dixon Drive in Cleveland, White County, Georgia. The site is bounded by existing residential properties to the north and west and undeveloped wooded land to the south and east. The site is partially wooded along the eastern portion of the property.

Research of historical imagery indicates that portions of the property have been wooded since the late 1940s.

GROUNDED was provided with a Concept Plan from Mr. Zach Lurie, on November 10, 2025. The site plan details the proposed residential development and property boundaries along with existing topography. Based on our observations during field activities, the topography of the site generally consists of moderate slopes, sloping downward in the northwest direction.

We assume that civil design drawings are currently being prepared. Once the drawings are available, it is recommended that copies be forwarded to GROUNDED so that we may perform a thorough review. As appropriate or necessary, GROUNDED reserves the right to amend or change the recommendations provided below in this report based on the final design of the proposed development.

3.0 FIELD EXPLORATION & TESTING

3.1 Scope

The scope of field services to evaluate the engineering characteristics of the sub-surface materials included a reconnaissance of the project site and performing test pit excavations. The depth to groundwater was recorded in each excavation during our exploration.

A total of twelve (12) test pit excavations were performed to depths ranging from two (2) to fifteen (15) feet below existing grades. Test pits were performed in the locations determined by Maximum Development General Contractors as detailed on the attached Test Pit Location Plan and were located in the field by GROUNDED representatives utilizing a survey grade GPS unit. The elevations recorded on the test pit logs and tables in this report were interpolated from existing topographic information published by the National Oceanic and Atmospheric Administration (NOAA).

It is presumed that the test pits are accurately located to within several feet, as shown on the attached Test Pit Location Plan. After completion of our field observations, the test pits were backfilled with the excavated soil.

3.2 Test Pit Excavations

The test pits were excavated utilizing a CAT 323 track mounted excavator. This is considered a medium-sized excavator and has a maximum bucket digging force of approximately 31, 496 pounds-force.

3.3 Groundwater Level Measurements

The groundwater level in each test pit was measured during excavation. The water level measured in each test pit was recorded on the Test Pit Logs, which are included in the Appendix.

4.0 SUBSURFACE CONDITIONS

4.1 General

The types of subsurface materials encountered have been visually classified and are described on the Test Pit Logs. The results of the field testing and the groundwater conditions observed have also been recorded on the Test Pit Logs. The Test Pit Logs are attached in the Appendix for review.

4.2 Soil Conditions

4.2.1 Initial Surface Layer

Topsoil is a dark-colored surficial material with a high organic content and is generally unsuitable for structural support. Each of the test pit excavations encountered a layer of topsoil measuring approximately two (2) to three (3) inches in thickness.

4.2.2 Residual Soils

Residual soils are defined as naturally occurring soils formed by the in-place weathering of the parent bedrock materials. Typically, in the Piedmont Physiographic Province, these soils are comprised of sandy silts and silty sands with varying amounts of clay and often some mica.

Underlying the surficial materials, residual soils were encountered in each of the test pit excavations. The residual soils extended to either the test termination depth or to partially weathered rock.

As per typical for the parent geology of the region, the residual soils encountered in our test pit excavations generally consisted of sandy clays with PWR/rock fragments throughout.

4.2.3 Partially Weathered Rock and/or Refusal Materials

The following definitions, for the purposes of this report, are used to describe partially weathered rock (PWR) and refusal materials.

Partially Weathered Rock (PWR)	Materials that have been subjected to less weathering than materials classified as residual soils and typically noted as significantly more dense/hard and difficult to excavate.
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Refusal Materials	These are generally materials that are sufficiently dense/hard so as to refuse excavation by the track mounted equipment used in this exploration.
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These materials may require difficult excavation techniques, including ripping and/or blasting to facilitate excavation. PWR and refusal materials were encountered in several of our test pit locations, at the depths and approximate elevations detailed in the following table:

Table 4.2.3 – PWR Materials/Refusal Materials Encountered

Test Pit ID	Depth of Partially Weathered Rock Materials Encountered (feet)		Approximate Elevation of Partially Weathered Rock Encountered (feet)		Depth of Refusal Materials Encountered (feet)	Approximate Elevation of Refusal Materials Encountered (feet)
	Top	Bottom	Top	Bottom		
TP-2	4 1/2	11	1651 1/2	1645	11	1645
TP-3	4	6	1654	1652	6	1652
TP-4	NE	NE	NE	NE	2	1678
TP-5	2 1/2	4	1690 1/2	1689	4	1689
TP-6	4	6	1682	1680	6	1680
TP-7	2	3 1/2	1660	1658 1/2	3 1/2	1658 1/2
TP-8	4	9	1646	1641	9	1641
TP-9	3	6	1623	1620	6	1620
TP-10	4 1/2	13	1645 1/2	1637	13	1637
TP-11	4	8 1/2	1624	1619 1/2	8 1/2	1619 1/2
TP-12	12	13	1596	1595	13	1595

4.3 Groundwater Conditions

Groundwater levels were measured during excavation of each test pit. As typical with test pit excavations, it creates an “un-safe” condition to allow large excavations to remain open. Therefore, each excavation was backfilled immediately after completion. Groundwater was not encountered in any test pit excavations during this exploration.

5.0 DISCUSSION AND RECOMMENDATIONS

5.1 Project Description

GROUNDED was provided with a Concept Plan from Mr. Zach Lurie on November 10, 2025. Based on the information provided, we understand the proposed development will consist of twelve (12) single-family residential lots. Typical civil construction will accompany the lots including asphaltic concrete roadways, sanitary sewerage, storm drain and storm water management, and other utilities.

GROUNDED assumes the residential structures will be typical wood or fiber-cement framing, one (1) to two (2) stories with concrete slab on grade or basement construction with cast in place concrete foundation walls and cast in place concrete basement slab on grade. Generally, residential structures of this type are designed for an allowable foundation bearing pressure of 2,000 pounds per square foot (psf).

We assume that final civil drawings have not been completed at this time. Once the civil drawings are available, a copy should be provided to GROUNDED so that we can review the findings and recommendations of this report based on that data.

An analysis of the subsurface conditions as they relate to earthwork activities for this project is discussed in detail below. General recommendations are discussed as well.

5.2 Site Preparation

The existing topsoil and/or vegetative materials encountered should be removed from structural areas of the site. GROUNDED recommends that the topsoil be stockpiled for re-use in non-structural (landscape) areas, as needed, and/or discarded from the site. The topsoil encountered should be considered as compressible and unsuitable for use as fill materials and/or for support of the planned structure.

To avoid construction delays and increased costs, earthwork activities should be performed during the drier months of the year, if possible. Clearing, stripping, and grubbing should be performed only during dry weather conditions.

Increased topsoil/unsuitable soil volumes and degradation of the surface soils are very likely when these types of activities are performed during periods of wet weather. Care should be taken to minimize construction traffic over unprotected areas of the site after periods of rain to prevent unnecessary disturbance of the soils. After clearing and grubbing activities are finished, GROUNDED recommends that areas to receive structural fill soils be proof rolled using a fully loaded tandem-axle dump truck (at least 15 tons) to evaluate the subgrade and identify any isolated areas of soft soils. Remedial measures should be determined by the Geotechnical Engineer at the time of the proof roll but may include removal and replacement or stabilization using stone, geotextile fabric, or a combination of these methods.

5.3 Buried Debris and Organics

No evidence of buried debris and/or organics was encountered at any time during this exploration. Burn pits, trash pits, or other previously buried materials may be encountered in other areas of the site. Any buried debris that may be encountered during construction activities should be brought to the immediate attention of the Geotechnical Engineer. Recommendations for removal and/or stabilization of the isolated area will be provided at that time based on the field conditions.

5.4 Sub-Surface Materials

5.4.1 Residual Soils

The residual soils encountered are consistent with soils of the Piedmont Physiographic Province and generally consisted of sandy clays with PWR intermixed.

However, the in-place residual soils will require extensive proof rolling and further field evaluations to determine the suitability of the sub-grade for support. Remedial recommendations for the encountered residual soils should be given by the project Geotechnical Engineer during construction activities.

These materials will be highly moisture sensitive and efforts to limit rubber-tired construction traffic will serve to protect the sub-grade soils from degradation due to typical construction traffic.

5.4.2 Partially Weathered Rock/Refusal Materials

Partially Weathered Rock and refusal materials were encountered underlying the residual soils as detailed above in Table 4.2.3.

The impact the encountered partially weathered rock and/or refusal materials may have on the planned development, with regard to earthwork activities, will be dependent upon final grading design. Care should be taken to avoid or minimize excavation in areas where refusal materials were encountered during design. If excavation into these materials is required, significant contingency should be allotted for difficult excavation techniques including ripping and/or blasting.

Should excavation into or below the refusal materials be required, GROUNDED recommends the following definitions be utilized:

Rock – General Excavation

Any material that cannot be excavated with a single-tooth ripper drawn by a crawler tractor having a draw bar pull rated at not less than 56,000 pounds (Caterpillar D-8 or equivalent) or excavated by a front-end loader with a minimum bucket breakout force of 25,600 pounds (Caterpillar 977 or equivalent).

Rock – Trench Excavation

Any material that cannot be excavated with a backhoe having a bucket curling force rated at not less than 18,300 pounds (Caterpillar 215 or equivalent).

Further, it should be noted that difficult excavation will require significant funds and again, as such, GROUNDED recommends that contingency funds be allotted for excavation of and removal of any partially weathered rock materials and/or refusal materials.

5.5 Structural Fill Soils

The in-place residual soils visually appeared suitable for use/re-use as structural fill. In general, fill soils should consist of clean material free of organic matter and placed in a controlled manner under the direction of a Geotechnical Engineer.

A representative of the Geotechnical Engineer should be onsite on a part-time basis to observe the fill placement and perform a sufficient number of in-place field density tests at regular intervals to verify the fill was placed in a controlled manner and with satisfactory compactive effort.

All structural fill materials within building and paved areas, including utility excavations, should be free of organic matter, rock fragments larger than four (4) inches in size, or other debris and should be compacted to a minimum of 95% of the maximum dry density within $\pm 3\%$ of the optimum moisture content as determined by Standard Proctor (ASTM D698). Structural fill materials within the top two (2) feet of building slabs, paved areas, and utility excavations within those areas should be compacted to a minimum of 98% of the maximum dry density within $\pm 3\%$ of the optimum moisture content as determined by Standard Proctor (ASTM D698).

Depending on the weather conditions at the time of grading activities, it may be necessary to moisture condition the in-situ materials prior to placing and compacting. These soils, particularly in the wetter seasons of the year, become relatively moisture sensitive. *It should be expected that wetting or drying of the in-situ soils may be required to achieve proper compaction.*

Prior to placing fill material, areas receiving structural fill should be proof rolled using a fully loaded tandem-axle dump truck weighing at least 15 tons, or other similar construction equipment. Proof rolling is performed to identify isolated soft or weak areas in the subgrade soils.

If soft or weak areas are encountered, remediation of these areas may include stabilization techniques, removal and replacement, moisture conditioning and re-compaction, or a combination of these methods. Recommendations for remediation of weak subgrades should be made by the Geotechnical Engineer in the field at the time of construction activities.

If partially weathered rock materials or any shot rock materials are planned for use as structural fill, they should be placed in deeper areas, mixed with soils and/or crushed to create materials of suitable sizing to meet the specifications as structural fill materials.

5.6 Groundwater

Groundwater was not encountered in any test pit excavation during this exploration. Because of variations in weather and seasonal changes, groundwater levels are subject to change depending on the time of year grading activities start.

If groundwater is encountered during earthwork and/or utility construction, it should be brought to the attention of the geotechnical engineer. Groundwater control and remediation of subgrades/bearing surfaces for pipes or other structures may be required. Additionally, soils excavated from below the groundwater table will likely require drying prior to re-use as fill/backfill.

5.7 Earthen Slopes

Temporary construction slopes, if required, should be designed in strict compliance with OSHA regulations. As a general rule, temporary construction slopes should be no steeper than 1H:1V for excavation depths of 20 feet or less. Temporary construction slopes below groundwater should be no steeper than 1.5H:1V. Temporary cut or fill slopes should be monitored closely by the Contractor's "competent person" for signs of movement, tension cracks, or sliding.

GROUNDED is not responsible for slope safety on this project. The stability of construction slopes and excavations is the complete and sole responsibility of the contractor performing the work.

Global stability analysis for site-specific slopes or retaining walls was not included in the scope of this exploration report. However, in general, cut or fill slopes should not be steeper than 2H:1V. Earthen dams for storm water detention ponds should be 3H:1V (if possible) and shall be constructed using clean structural fill soils properly placed and compacted in accordance with the recommendations found in this report. It is common for local municipalities to require a Professional Engineer's letter of certification regarding the placement and compaction of the fill soils within the earthen dam after construction is completed.

Fill slopes should be placed in relatively thin layers and properly compacted in accordance with the recommendations found in Section 5.5 "Structural Fill Soils" above. Fill should not be dumped over the top of a slope to extend it laterally. This will result in poorly compacted soils and weak zones within the slope that could lead to failure. Additions to existing slopes should be properly benched or "keyed-in" to tie-in the new fill soils.

The toe or crest of any fill or cut slope should be at least ten (10) feet from any building foundation and at least five (5) feet from the edge of any pavement. All slopes should be properly vegetated and/or stabilized to prevent erosion.

6.0 LIMITATIONS

The test pit excavation locations were limited to the locations depicted on the Test Pit Location Plan included in the Appendix. Exploration of the subsurface materials in the specific areas of site retaining walls, utility trenches, and detention ponds was not included in the scope of work. Varying subsurface conditions may be encountered in those areas.

There are several important limiting factors to consider with test pit excavations utilized during subsurface explorations. These include thin layers that are not detected because of the excavation method used and a limited number of test pits placed in pre-determined locations. It is assumed that subsurface conditions are similar between test locations. However, subsurface conditions can and often do vary between test pit excavations. If varying subsurface conditions are encountered during construction, they should be brought to the immediate attention of the Geotechnical Engineer.

While not anticipated, experience has shown that poorly compacted fill soils, cultivated soils, and/or isolated burial pits sometimes may be encountered on project sites in other areas not tested, including structural areas, even near test pit excavations. It is generally wise to provide a contingency fund for management of poor-quality soils, burial pits, and other unsuitable materials.

7.0 GENERAL COMMENTS

When the plans and specifications are complete, or if significant changes are made in the character or location of the proposed structures and appurtenant development, a consultation should be arranged to review the changes with respect to the prevailing soil conditions. At that time, it may be necessary to submit supplementary recommendations.

It is recommended that **GROUNDED Engineering Services, LLC** be engaged to test and evaluate the compaction of the new fill materials and to test and evaluate the bearing value of the foundation soils in the footing excavations. Additionally, with our knowledge and understanding of the subsurface conditions on this site and the years of experience, our presence during construction can be a valuable and critical component to the quality and success of this project.

8.0 SIGNATURES OF GEOTECHNICAL PROFESSIONALS

This Geotechnical Exploration report has been prepared by the following professionals at GROUNDED Engineering Services, LLC.



Bradley E. Arbo
Project Geologist



Charles D. Edwards, PE
Chief Operating Officer



TEST PIT LOCATION PLAN
DIXON DRIVE DEVELOPMENT
 438 DIXON DRIVE
 CLEVELAND, WHITE, COUNTY, GA

PREPARED FOR:
MAXIMUM DEVELOPMENT
GENERAL CONTRACTORS
 4607 CROMWELL COURT
 FLOWERY BRANCH, GEORGIA
 30542

PROJECT NO.: 26MAX001
 DESIGN: SWH DRAWN: SBM
 CHECKED BY: CDE
 ISSUE DATE: 11-26-2025
 SCALE: 1:80

SHEET
B100

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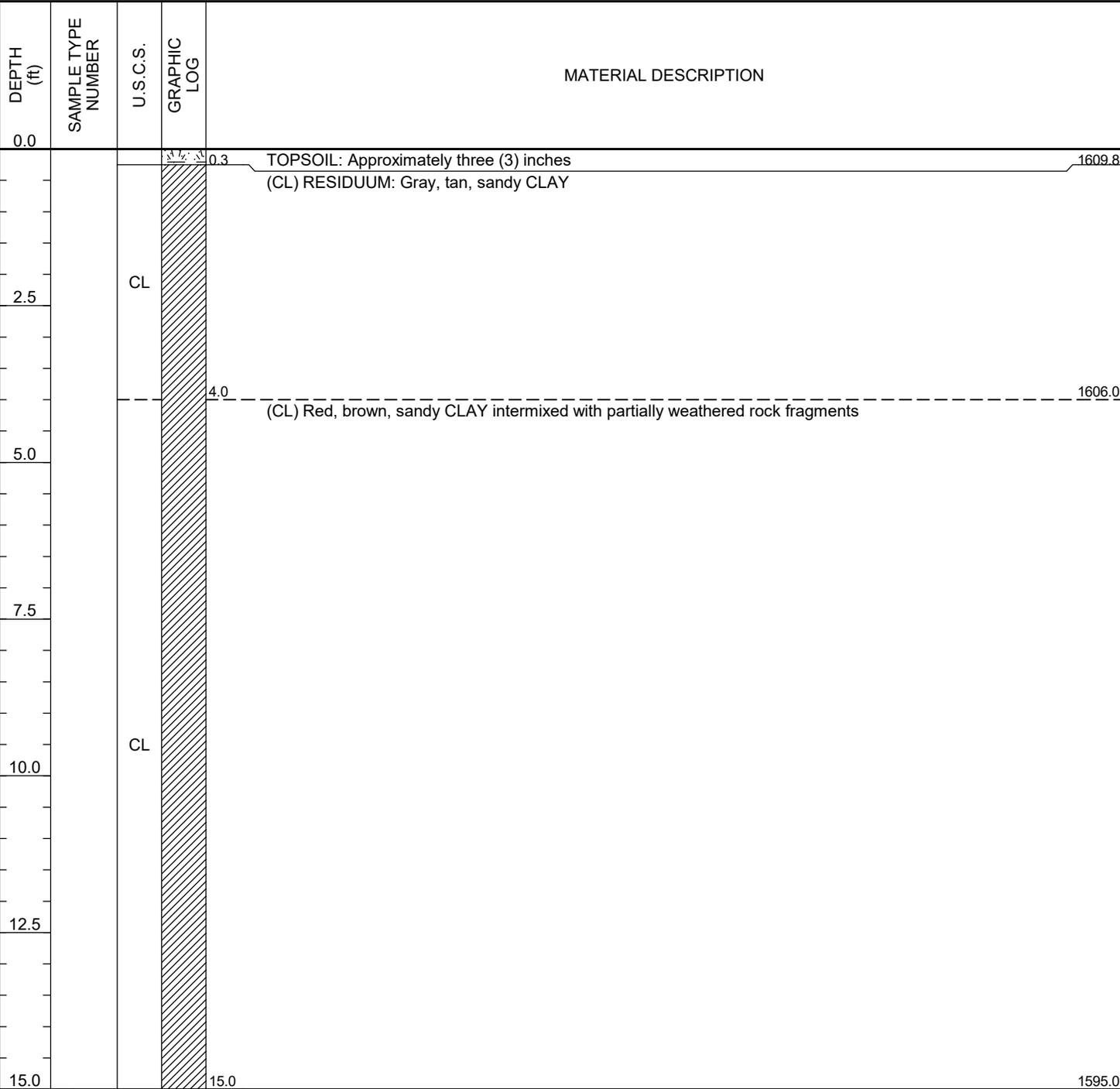
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TEST PIT NUMBER TP-1

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1610 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed



Bottom of test pit at 15.0 feet.



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TEST PIT NUMBER TP-2

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1656 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION
0.0					
0.3				TOPSOIL: Approximately three (3) inches	1655.8
2.5		CL		(CL) RESIDUUM: Red, brown, tan, sandy CLAY with partially weathered rock fragments	
4.5				PARTIALLY WEATHERED ROCK: Observed as brown, tan, gray, sandy clay; fragments measuring four (4) to six (6) inches in diameter	1651.5
5.0					
7.5					
10.0					
11.0					1645.0

Refusal at 11.0 feet.
 Bottom of test pit at 11.0 feet.



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TEST PIT NUMBER TP-3

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1658 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1657.8
2.5		CL			
4.0				PARTIALLY WEATHERED ROCK: Observed as white, gray, sand; fragments measuring four two (2) to four (4) inches in diameter	1654.0
5.0					
6.0				Refusal at 6.0 feet. Bottom of test pit at 6.0 feet.	1652.0



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TEST PIT NUMBER TP-4

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1680 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0.0					
		CL		TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Brown, red, tan, sandy CLAY	1679.8
				Refusal at 2.0 feet. Bottom of test pit at 2.0 feet.	1678.0



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TEST PIT NUMBER TP-5

CLIENT <u>Maximum Development General Contractors</u>	PROJECT NAME <u>Dixon Drive Development</u>
PROJECT NUMBER <u>25MAX001</u>	PROJECT LOCATION <u>Cleveland, Georgia</u>
DATE STARTED <u>11/21/25</u> COMPLETED <u>11/21/25</u>	GROUND ELEVATION <u>1693 ft</u> TEST PIT SIZE <u>36 inches</u>
EXCAVATION CONTRACTOR <u>Independence Drilling</u>	GROUND WATER LEVELS:
EXCAVATION METHOD <u>Test Pit Excavation</u>	AT TIME OF EXCAVATION <u>--- No Groundwater Encountered</u>
LOGGED BY <u>E. Rice</u> CHECKED BY <u>Edwards, PE</u>	AT END OF EXCAVATION <u>--- No Groundwater Encountered</u>
NOTES _____	AFTER EXCAVATION <u>--- Measurement Not Performed</u>

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0.0					
		CL	[Hatched Pattern]	TOPSOIL: Approximately two (2) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1692.8
2.5			[Cross-hatched Pattern]	PARTIALLY WEATHERED ROCK: Observed as gray, white, sand; fragments measuring eight (8) to ten (10) inches in diameter	1690.5
			[Diagonal Pattern]		1689.0

Refusal at 4.0 feet.
 Bottom of test pit at 4.0 feet.



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TEST PIT NUMBER TP-6

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1686 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1685.8
2.5		CL			
4.0				PARTIALLY WEATHERED ROCK: Observed as white, tan, gray, sandy clay; fragments measuring eight (8) to ten (10) inches in diameter	1682.0
5.0					
6.0					1680.0

Refusal at 6.0 feet.
 Bottom of test pit at 6.0 feet.

GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 12/1/25 13:06 - C:\USERS\BARBO\GROUNDED DROPBOX\GROUNDED PROJECTS\MAX - MAXIMUM DEVELOPMENT GENERAL CONTRACTORS\2025\25MAX001 - GA - CLEVELAND - DIXON DRIVE\GEOI2025 11 25



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 4148 Industry Way, Suite E
 Flowery Branch, GA 30542
 Telephone: 678-825-3690
 Fax: 678-825-3691

TEST PIT NUMBER TP-8

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1650 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, gray, sandy CLAY	1649.8
2.5		CL			
4.0				PARTIALLY WEATHERED ROCK: Observed as white, gray, tan, sand	1646.0
5.0					
7.5					
9.0					1641.0

Refusal at 9.0 feet.
 Bottom of test pit at 9.0 feet.

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 Fax: 678-825-3691

TEST PIT NUMBER TP-9

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1626 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1625.8
2.5		CL			
3.0				PARTIALLY WEATHERED ROCK: Observed as brown, tan, white, sandy clay; fragments measuring eight (8) to ten (10) inches in diameter	1623.0
5.0					
6.0				Refusal at 6.0 feet. Bottom of test pit at 6.0 feet.	1620.0

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 Fax: 678-825-3691

TEST PIT NUMBER TP-10

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1650 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1649.8
2.5		CL			
4.5				PARTIALLY WEATHERED ROCK: Observed as white, gray, tan, sandy clay; fragments measuring four (4) to six (6) inches in diameter	1645.5
5.0					
7.5					
10.0					
12.5					
13.0					1637.0

Refusal at 13.0 feet.
 Bottom of test pit at 13.0 feet.

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TEST PIT NUMBER TP-11

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1628 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1627.8
2.5		CL			
4.0				PARTIALLY WEATHERED ROCK: Observed as white, gray, tan, sand; fragments measuring four (4) to six (6) inches in diameter	1624.0
5.0					
7.5					
8.5				Refusal at 8.5 feet. Bottom of test pit at 8.5 feet.	1619.5

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 Fax: 678-825-3691

TEST PIT NUMBER TP-12

CLIENT Maximum Development General Contractors **PROJECT NAME** Dixon Drive Development
PROJECT NUMBER 25MAX001 **PROJECT LOCATION** Cleveland, Georgia
DATE STARTED 11/21/25 **COMPLETED** 11/21/25 **GROUND ELEVATION** 1608 ft **TEST PIT SIZE** 36 inches
EXCAVATION CONTRACTOR Independence Drilling **GROUND WATER LEVELS:**
EXCAVATION METHOD Test Pit Excavation **AT TIME OF EXCAVATION** --- No Groundwater Encountered
LOGGED BY E. Rice **CHECKED BY** Edwards, PE **AT END OF EXCAVATION** --- No Groundwater Encountered
NOTES _____ **AFTER EXCAVATION** --- Measurement Not Performed

DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
0.0					
0.3				TOPSOIL: Approximately three (3) inches (CL) RESIDUUM: Red, brown, tan, sandy CLAY	1607.8
2.5					
5.0		CL			
7.5					
10.0					
12.0					1596.0
12.5				PARTIALLY WEATHERED ROCK: Observed as brown, gray, sand; fragments measuring two (2) to four (4) inches in diameter	
13.0					1595.0

Refusal at 13.0 feet.
 Bottom of test pit at 13.0 feet.