DESIGN NOTES:

This is to certify that the following project has been designed in accordance with the 2018 $\ensuremath{\mathsf{IBC}}$

for the following loads and deflections:

COMMON DATA: Building Code Building Occupancy Wind Speed, Vult Wind Speed, Vasd Ground Snow, Pg Roof Snow, Pf Importance	= 2018 IBC = II - Normal = 105,00 mph = 81.33 mph = 5,0000 psf = 3,5000 psf
Seismic	= 1.0000
Design Category	= A
Importance	= 1.0000
Site Class	= d
Analysis Procedure	= Equivalent Lateral Force

PROJECT DIMENSIONS

/	PROJECT DIMENSIONS					
PROJECT:	WIDTH (ft)	LENGTH (ft)	HEIGHT (ft)	PITCH	POROSITY & Cpi	
А	70	150	24	2.0:12		
В	60	67	21	2.0:12		

	PROJEC ⁻	t loading	(Building S	Specific)	
PROJECT:	DEAD (psf)	LIVE (psf)	Coll. (psf)	SNOW (psf)	SNOW DRIFT
А	2.000	20.00	0.5	3.5000	
в	2.000	20.00	0.5	3.5000	43.88 psf and 11.98

Drawing Status:

FOR APPROVAL: THESE DRAWINGS, BY DEFINITION, ARE NOT FINAL, AND ARE FOR CONCEPTUAL USE ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED COMPLETE.

FOR PERMIT: THESE DRAWINGS, ARE FOR PERMIT USE ONLY, AND AS SUCH, PIECE MARKS MAY NOT BE IDENTIFIED, ONLY DRAWINGS LABELED "FOR CONSTRUCTION" CAN BE CONSIDERED COMPLETE.

X FOR CONSTRUCTION: FINAL DRAWINGS.

E29 - E30

DRAWING INDEX					
PAGE ID	DESCRIPTION				
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C2 of 2	General Notes				
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E1 – E2	Roof Framing/Sheeting				
E3 – E6	Endwall Framing/Sheeting				
E7 — E10	Sidewall Framing/Sheeting				
E11 - E21	Rigid Frame Drawings				
	Partition Walls				
E22 - E24	Crane Drawings				
E25 — E26	Mezzanine Drawings				
	Jack Beams				
E27 – E28	Wall/Roof Insulation				
-					

PANELS/INS	SULATION	
Roof Panels: PBR		
Roof Insulation: VR	Thick. 3	in.
Wall Panels: PBR		
Wall Insulation: VR	Thick. 3	in.

DEFLECTION LIMITS						
EW COLUMN	H/180					
EW RAFTER	L/ 180					
GIRT (live)	L/90					
PURLIN (live)	L/180					
PANEL (live)	L/ 60					
FRAME COLUMN	Н/120					
FRAME RAFTER	L/180					

SEISMIC VALUES

А	PROJECT	Ss 0.1481	S1 0.0362	Sds 0.1579	Sd1 0.0576	Cs (long.) 0.053	Cs (trans) 0.049	Long. SFRS OSCBF	Trans. SFRS OSMF	R_long 3	R_trans 3
в			0.0362	0.1579	0.0576	0.053	0.053	OSMF	OSCBF	3	3

BUILDING COLORS					
ROOF	Polar White				
WALL	Light Stone				
EAVE					
RAKE	Dark Bronze				
JAMB	Dark Bronze				
CORNER	Dark Bronze				
BASE	Dark Bronze				
GUTTER	Dark Bronze				

Wall/Roof Liner Panels

Wind Brace

Details

ADDITIONAL NOTES:

TBUILD IT WITH STEEL BUILD IT TO LAST
STEEL COMPONENTS, INC. [

430 US Hwy 180 West Seminole, TX 79360 (432) 758-9900 (432) 758-9903 (fax)

			PURCHASER:
SALES	S PERSON: I	BOBBY	PRDJECT:
$ \land $			JOB NUMBER:
			JOB ADDRESS:
\square			
REV,	DATE	REVISION	

HAYLEE	
PB50586	
DDESSA TEXAS 79336	
	Sheet No.
	C1 of 2

General Notes

This drawing, and all information contained within, remains the property of JNL Steel. The building shall be erected using members called out in this drawing.

In the event that the design changes, it is the responsibility of the contractor to ensure that a stamped letter from the sealing engineer is provided, giving the approval.

3000 psi concrete is assumed for all calculations involving concrete strength.

Anchor Bolts

The anchor bolt diameters are determined in accordance with AISC using A307 anchors. Anchor bolt lengths are to be determined by a foundation engineer, and as such, cannot be provided with the building. It is the responsibility of the erector to purchase anchor bolts separately and no liability will be placed on JNL Steel or the stamping engineer for incorrect anchor bolt lengths.

Field Work

Minimal field work is expected with erecting metal buildings. If a customer does not tell JNL where to place an opening, we will "field locate" the building and it is the erectors responsibility to drill holes in the girt and place the opening. All field welding should be done by a certified welder in accordance with AWS D1.1.

Structural Bolts

Bolts not subject to tension or vibration loads should only be snug tightened. Bolts in slotted connections should be hand tightened with the thread damaged afterwards to prevent nut from backing out. This allows for vertical deflection of the connecting member. ASTM A307 bolts are used for girt to frame, purlin to frame, and all flange bracing connections unless noted. ASTM A325 are used everywhere else unless noted. Threads are assumed in the shear plane.

Material Delivery

JNL will attempt to deliver the building on an agreed upon date. Sometimes, through un-forseen events, this cannot be possible. JNL does not accept responsibility for costs associated with the customer renting personnel or equipment to unload the truck. Responsibility to unload the truck is placed on the customer, unless specifically stated in the contract.

Approval Drawings

Approval drawings are meant to show the erector general building geometry, framed opening locations, and clearances. If member sizes appear on drawings, they are not supposed to be taken as true size. Customer shall sign all pages of the approval drawings and return to JNL before building gets sent to stamping engineer. To ensure current pricing, approval drawings will need to be signed within (2) weeks of issue.

Contracts

Customer should review the contract thoroughly and make sure everything is clear and accounted for. Customer is responsible for supplying JNL with the appropriate design code. In the event that such data is not made available, customer should call the local building official. JNL or the stamping engineer will not be held liable for incorrect building codes. Building will not get sent to stamping engineer without a signed contract. Verbal confirmation over email,phone, or in-person will not be accepted.

Material Specifications

Hot Rolled: A992 Gr. 55HSS: ASTM A500 Gr. 46Built up Webs: A36Built up Flanges: A529 Gr. 55Cold Formed: ASTM A1011 Gr. 55DiagonalBracing:Steel cable is extra high strength per ASTM A475Bolts:Secondary member bolts conform to ASTM A307 (U.N.)

Primary Members conform to ASTM A325 (U.N.)

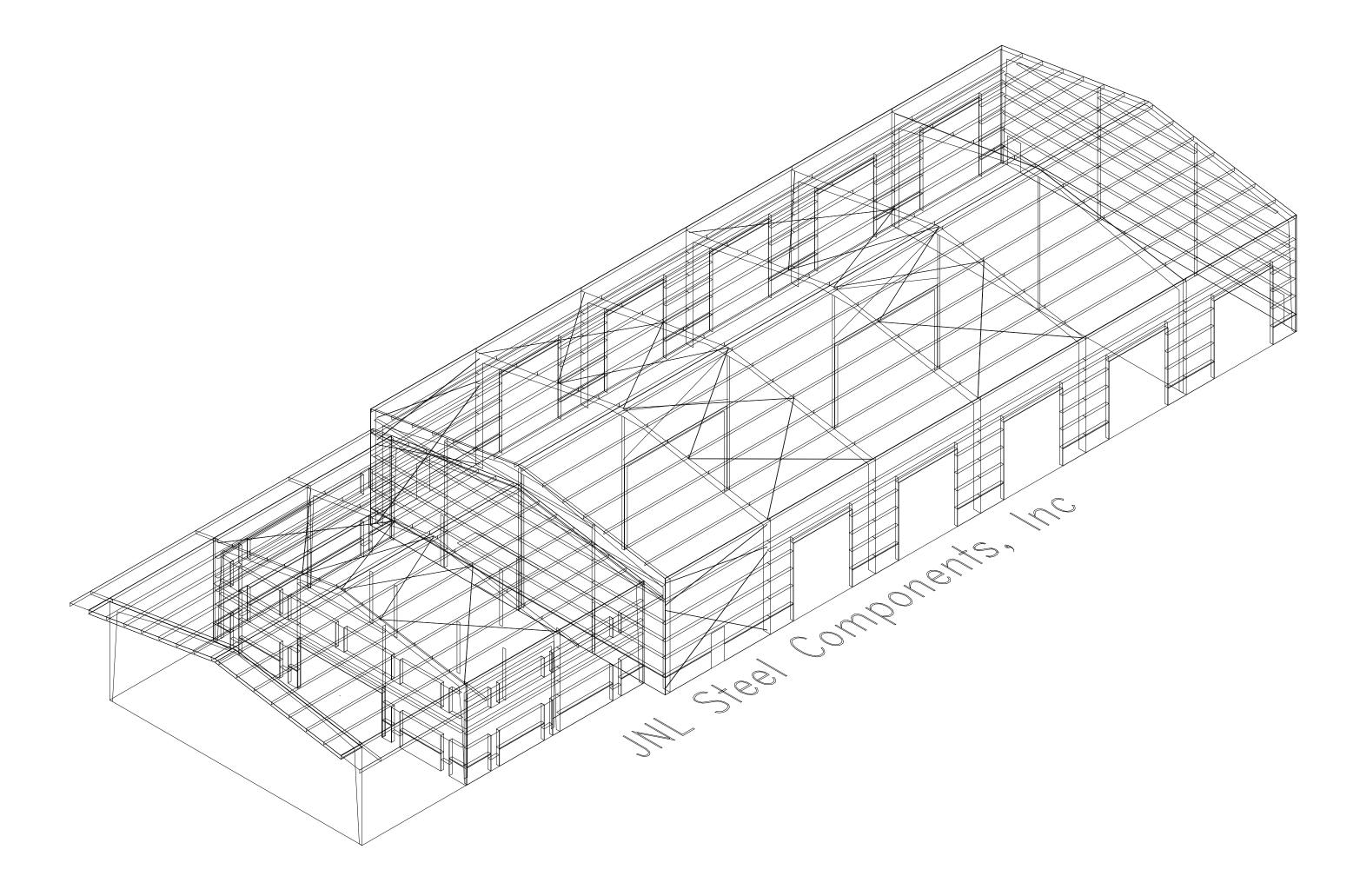
Material Storage

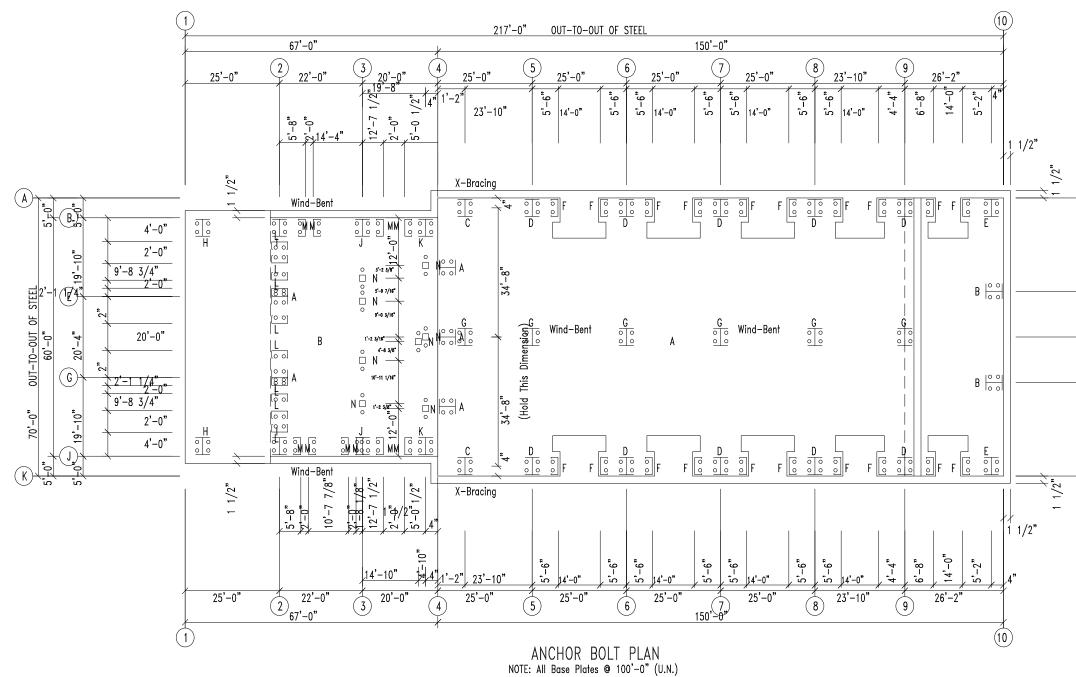
Panels should be placed off the ground and slopped enough to provide good drainage. Board should be placed at close enough intervals to prevent the panels from sagging and resulting in water accumulation. Galvanized, and colored materials are subject to corrosion and discoloration if stored improperly.

Bracing

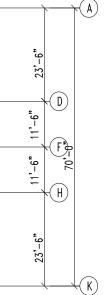
Bracing is an integral part of the building system. All bracing should be installed taut and free of slack. Walls utilizing panel shear should have temporary bracing until all panels are installed. During erection, contractor should install temporary bracing to provide stability, and help square the building. Do not remove bracing due to "inconvenience". If bracing needs to be moved/removed, contact a licensed professional for approval.

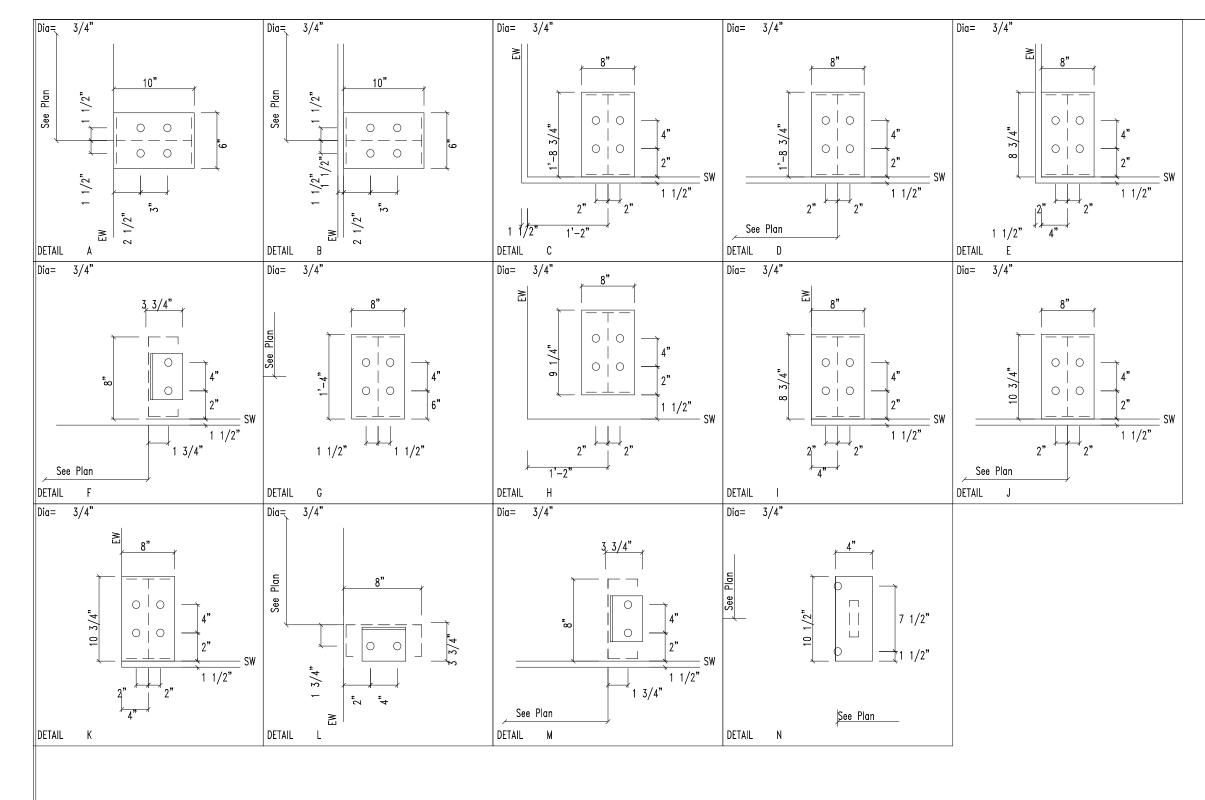
JBUILD IT WITH STE	EX BUIL		T TU LASK
<u>)steel comf</u> 432-7 430 US H SEMINDLE	58-9 4WY	9 9 18	30 W
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PROJECT PB50586 HAYLEE			
ODESSA TEXA	S 79336	5	
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РР Снескед ву		DAT	
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APPROVE			DAT 7 SCA NT	7/26/23 NE
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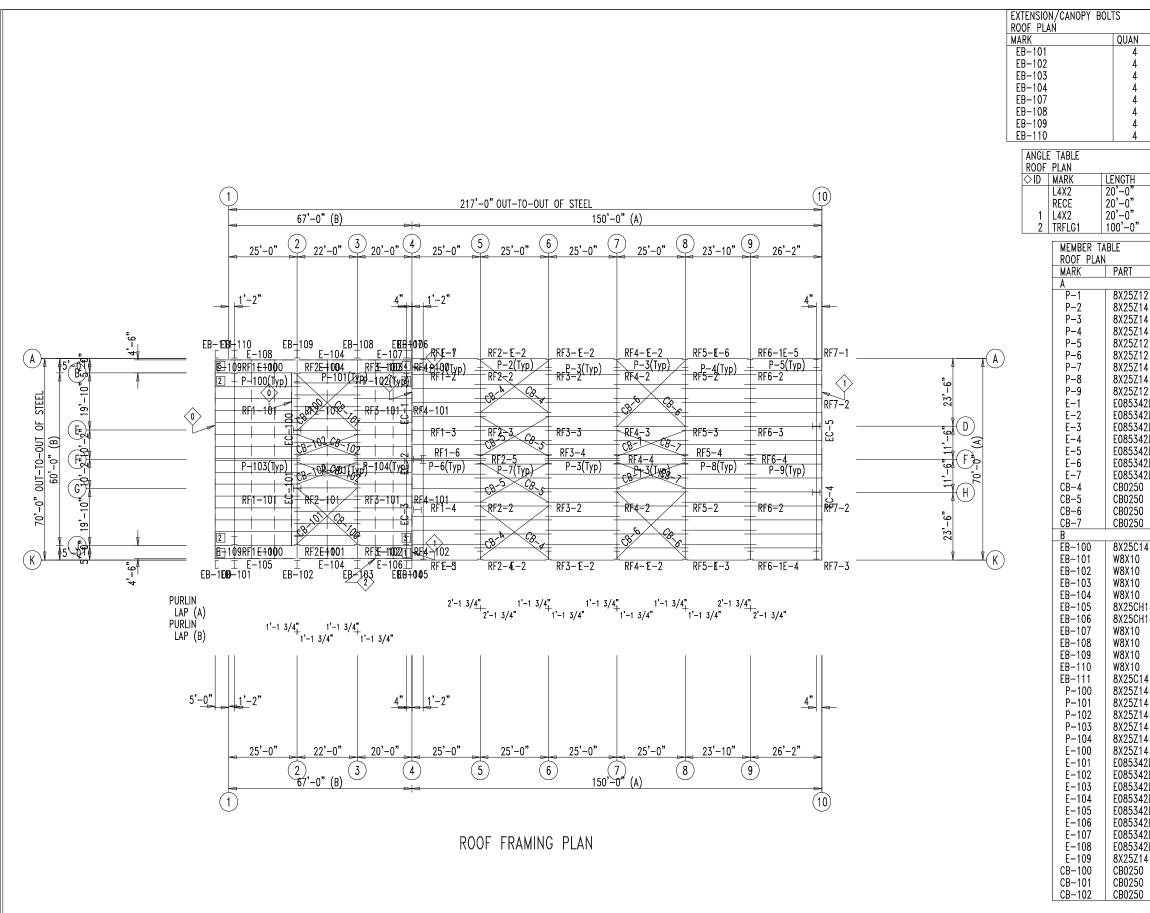
BUILD IT WITH STEEL BUI IS TEEL COMPONI 432-758-9 430 US HWY SEMINOLE,TX	900 180 W
PROJECT	
PB50586 HAYLEE DDESSA TEXAS 79 Sheet Name/Title ANCHOR BOLT DE	
DRAWN BY CHECKED BY ·BN APPROVED BY	DATE 7/26/23 DATE 7/26/23 DATE 7/26/23
SHEET NO. REV. A2 of 4 REVISIONS NO. NOTES 	Scale NTS BY DATE 00-00-00

		FRAME LINES: 4 5 6 7 8 9
RIGID FRAME: BASIC COLUMN REACTIONS (K) Frame ColumnDecidCollateralNesNesSnewWind_teff1Wind_Right1	RIGID FRAME: maximum reactions	() () () () () () () () () () () () () (
Line Horiz Vert Horiz Horiz Vert Horiz Vert <t< td=""><td>Frm Col Load Himax V Load Himin V Line Line Id H Vimax Id H Vimin</td><td></td></t<>	Frm Col Load Himax V Load Himin V Line Line Id H Vimax Id H Vimin	
4 F 0.0 2.2 0.0 0.2 0.0 5.7 0.0 1.7 0.0 -8.5 0.0 -8.5 Frame ColumnWind_Left2Wind_Right2Wind_Long1Wind_Long2Selsmic_Left Selsmic_Right	10 A 1 3.2 7.7 2 -3.5 -5.3	
Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 4 A – 4.1 – 4.4 2.0 – 0.4 1.3 – 5.9 0.5 – 4.1 – 0.5 – 0.4 0.5 0.4	10 K 3 3.5 -5.3 1 -3.2 7.7 1 -3.2 7.7 3 3.5 -5.3	
4 K 4 F 0.0 -6.0 0.0 -6.0 0.0 -7.1 0.0 -7.1 0.0 0.0 0.0 0.0 0.0	ENDWALL COLUMN: BASIC COLUMN REACTIONS (K) Wind Wind Sels	
Frame Column –VIN_SNOWFICRANEA1FICRANEA2FICRANEA3FICRANEA3FICRANEA5- Line Line Hortz Vert Hortz Vert Hortz Vert Hortz Vert Hortz Vert Hortz Vert Hortz Vert 4 A 0.2 1.1 -0.9 17.0 1.5 18.4 0.3 2.9 2.6 4.4 -0.9 2.0	Frm Col Dead Press Suct Long Line Vert Horz Horz Horz 4 C 0.3 -4.1 4.5 0.0	
4 K -0.2 1.1 -2.6 4.4 -0.3 2.9 -1.5 18.4 0.9 17.0 -1.4 3.5 4 F 0.0 2.4 0.6 21.3 1.7 21.3 -1.7 21.3 -0.6 21.3 -0.5 37.2	4 F 0.3 -4.5 5.0 0.1 4 I 0.3 -4.1 4.5 0.0	
Frame Columnn -F1CRANEAG- F1PAT_LL_1- F1PAT_LL_2- F1UNB_SL_L- F1UNB_SL_R- Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert A A 14 3.5 0.2 2.7 0.2 -0.1 0.1 0.9 0.1 0.9	10 H 0.3 -5.4 6.0 0.1 10 D 0.3 -5.4 6.0 0.1	<u> </u>
4 A 1.4 3.5 0.2 2.7 0.2 -0.1 0.1 0.9 0.1 0.2 4 K 0.9 2.0 -0.2 -0.1 0.2 2.7 -0.1 0.2 -0.1 0.9 4 F 0.5 37.2 0.0 2.9 0.0 1.9 0.0 1.9	ENDWALL COLUMN: MAXIMUM REACTIONS	tv tv t
Frame ColumnDeadCollateralLive SnowWind_Laff1Wind_Right1- Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 5 A 0.2 1.6 0.0 0.2 0.9 4.9 0.3 1.4 -5.1 -9.6 4.3 -3.6	Frm Col Hmax V Lood Himin V Line Line Id H Vmax Id H Vmin	FRAME LINES: 10
5 K 0.2 1.6 0.0 0.2 0.9 4.9 0.3 1.4 -5.1 -9.6 4.3 -3.6 5.1 -3.6 5.1 -9.6 5.5 F 0.0 3.0 0.2 -0.9 4.9 -0.3 1.4 -4.3 -3.6 5.1 -9.6 5.5 F 0.0 3.0 0.0 0.5 0.0 10.7 0.0 3.1 -1.1 -12.5 1.1 -12.5	4 C 37 2.7 0.2 38 -2.4 0.2	
Frame ColumnWind_Leff2Wind_Right2Wind_Long1Wind_Long2Seismic_Left Seismic_Right Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert	39 2.7 0.3 4 F 37 3.0 0.2 38 -2.7 0.2 39 3.0 0.3	
5 A -6.7 -5.8 2.8 0.2 2.1 -8.4 1.8 -6.2 -0.5 -0.4 0.5 0.4 5 K -2.8 0.2 6.7 -5.8 -1.8 -6.2 -2.1 -8.4 -0.5 0.4 0.5 -0.4	4 [37 2.7 0.2 38 -2.4 0.2 39 2.7 0.3	
Frame Column -Seismic Long -VIN SNOWF2CRANEA1F2CRANEA2F2CRANEA3F2CRANEA4-	10 H 37 3.6 0.2 38 -3.2 0.2 39 3.6 0.3	
Line Horiz Vert Horiz Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Low Horiz Low Horiz Low H	10 D 37 3.6 0.2 38 -3.2 0.2 39 3.6 0.3	
5 F 0.0 -0.8 0.0 4.5 -0.6 23.0 1.5 23.0 -1.5 23.0 0.6 23.0 Frame Column -F2CRANEA5F2CRANEA6- F2PAT_LL_1- F2PAT_LL_2- F2UNB_SL_L- F2UNB_SL_R-	NOTES FOR REACTIONS	
Line Line Horiz Vent Horiz Vent Horiz Vent Horiz Vent Horiz Vent Horiz Vent Horiz Vent 5 A – 0.6 2.9 1.3 4.4 0.8 5.0 0.1 –0.2 0.3 1.6 0.1 0.3	 All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported. 	
5 F -1.0 39.1 1.0 39.1 -0.7 5.4 0.7 5.4 -0.2 3.5 0.2 3.5	 Positive reactions are as shown in the sketch. Foundation loads are in opposite directions. 	
Frame Column ————Dead——— ——Collateral —ververNow——— ——Wind_Leffi —Wind_Righti = Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 6 A 0.2 1.6 0.0 0.2 0.9 0.9 0.9 0.9 0.9 0.9 0.0 0.1 0.5 0.5 0.5 0.1 0.9 4.5 - 3.5	 Bracing reactions are in the plane of the brace with the h pointing away from the braced bay. The vertical reaction is downward. Building reactions are based on the following building date: 	¥
6 K -0.2 1.6 0.0 0.2 -0.9 5.0 -0.3 1.5 -4.5 -3.5 5.1 -9.6 6 F 0.0 3.0 0.0 0.5 0.0 11.0 0.0 3.2 -1.2 -12.3 1.2 -12.3	with (fi) - 70.0	RIGID FRAME: MAXIMUM REACTIONS Column. Reactions(k)
Frame Column —-Wind_Left2- — Wind_Right2- —-Wind_Long1- —-Wind_Long2- —Saismic_Left Saismic_Right Line Line Hortz Vert Hortz Vert Hortz Vert Hortz Vert Hortz Vert Hortz Vert 6 A — -6.8 — 5.5 2.8 0.4 2.2 — 8.4 1.9 — 6.2 — 0.5 — 0.4 0.5 0.4	Roof Slope (rise/12) = 2.0/´2.0 Dead Load (nsf) = 2.0	Frm Col Load Hmax V Load Hmin V Line Line Id H Ymax Id H Ymin
6 K -2.8 0.4 6.8 -5.6 -1.9 -6.2 -2.2 -8.4 -0.5 0.4 0.5 -0.4 6 F -1.2 -7.5 1.2 -7.6 0.6 -14.4 -0.6 -14.4 -0.4 0.0 0.4 0.0	Colliderati Lood (per) = 0.5 Roof Live Load(per) = 20.0 Frame Live Load(per) = 12.0	4 A 11 3.0 4.4 4 -2.4 -1.9 7 1.6 19.8 2 -2.0 -3.1
Frame Column -Seismic_Long -MIN_SNOWF3CRANEA1F3CRANEA2F3CRANEA3F3CRANEA4- Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 6 A 0.0 0.0 0.4 2.1 -0.2 18.0 1.7 19.5 0.3 3.9 2.2 5.4	Snow Load (psf ``) ´ = 3.5 Wind Speed (mph) = 105.0 Wind Code = IBC 18	4 K 5 2.4 -1.9 10 -3.0 4.4 8 -1.6 19.8 3 2.0 -3.1
6 K 0.0 0.0 -0.4 2.1 -0.2 16.0 1.7 15.3 -1.7 19.5 0.2 18.0 6 F 0.0 -0.8 0.0 4.6 -0.6 23.1 1.5 23.1 -1.5 23.1 0.6 23.1	Exposure = C Closure = Enclosed Importance Wind = 1.00	4 F 7 1.7 23.8 8 -1.7 23.8 9 0.5 39.6 3 0.0 -3.8
Frame Column -F3CRANEA5F3CRANEA6- F3PAT_LL_1- F3PAT_LL_2- F3UNB_SL_L- F3UNB_SL_F Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 6 A -0.6 2.9 1.3 4.4 0.8 5.2 0.1 -0.2 0.3 1.6 0.1 0.3	importante information = 1.00 importante Salamic = 1.00 Selamic Zone = A Selamic Coeff (rc#Sa) = 0.24	RIGID FRAME: MAXIMUM REACTIONS
6 A -0.6 2.9 1.3 4.4 0.8 5.2 0.1 -0.2 0.3 1.6 0.1 0.3 6 K -1.3 4.4 0.6 2.9 -0.1 -0.2 -0.8 5.2 -0.1 0.3 -0.3 1.6 6 F -0.1 3.4 0.6 2.9 -0.1 -0.2 -0.8 5.2 -0.1 0.3 -0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 1.6 0.1 0.3 0.2 3.6 0.2 3.6 0.2 3.6 0.2 3.6 0.2 3.6 <th>5. Loading conditions are:</th> <th>Frm Col Load Hmax V Load Hmin V Line Line Id H Ymax Id H Ymin</th>	5. Loading conditions are:	Frm Col Load Hmax V Load Hmin V Line Line Id H Ymax Id H Ymin
Frame ColumnDeadCollateralLive Snow	1 Dead+Callateral+Live 2 0.6Dead+0.6Wind Left1	5 A 16 3.3 5.5 4 -3.9 -2.5 12 2.0 21.2 2 -2.9 -4.8
7 K 0.2 1.6 0.0 0.2 0.9 5.0 0.3 1.5 -5.1 -9.6 4.5 -3.5 7 K -0.2 1.6 0.0 0.2 -0.9 5.0 -0.3 1.5 -4.5 -3.5 5.1 -9.6 7 F 0.0 3.0 0.0 0.5 0.0 1.10 0.0 3.2 -1.2 -12.3 1.2 -12.3	3 0.6Dad+0.6Wind_Right1 4 0.6Dad+0.6Wind_Laft2 5 0.5Dad+0.6Wind_Right2	5 K 5 3.9 -2.5 15 -3.3 5.5 13 -2.0 21.2 3 2.9 -4.8
Frame ColumnWind Left2Wind Right2Wind Long1Wind Long2Seismic Left Seismic Right	6 0.6Dead+0.6Wind_iong2L 7 Dead+Colleteri+F1 (F0RNEA2 8 Dead+Colleteri+F1 (F0RNEA3	5 F 12 1.5 26.4 13 -1.5 26.4 14 1.0 42.5 6 -0.4 -6.9
Line Horiz Vert Horiz <t< th=""><th>9 Dead+Collsteral+F1CRNEA6 10 Dead+0.755now+0.3Wind_Left1+0.7551ide_Snow+0.75F1CRANEA1 11 Dead+0.755now+0.3Wind_Right1+0.7551ide_Snow+0.75F1CRANEA4</th><th>RIGID FRAME: MAXIMUM REACTIONS Column. Reactions(k)</th></t<>	9 Dead+Collsteral+F1CRNEA6 10 Dead+0.755now+0.3Wind_Left1+0.7551ide_Snow+0.75F1CRANEA1 11 Dead+0.755now+0.3Wind_Right1+0.7551ide_Snow+0.75F1CRANEA4	RIGID FRAME: MAXIMUM REACTIONS Column. Reactions(k)
Frame Column -Seismic_Long -MIN_SNOWF4CRANEA1F4CRANEA2F4CRANEA3F4CRANEA4-	12. Deod+Collered+F2CRANEA2 13. Deod+Collered+F2CRANEA3 14. Deod+Collered+F2CRANEA6	Frm Col Load Hmax V Load Hmin V Line Line Id H Ymax Id H Ymin
7 A 7 K 0.0 0.0 0.4 2.1 -0.2 18.0 1.7 19.5 0.3 3.9 2.2 5.4 0.0 0.0 -0.4 2.1 -2.2 5.4 -0.3 3.9 -1.7 19.5 0.2 18.0	15 Dead+0.75Snow+0.3Wind_Left+0.7SSilde_Snow+0.75F2CRANEA1 16 Dead+0.75Snow+0.3Wind_Right+1.77SSilde_Snow+0.75F2CRANEA4 17 Dead+Collered+155CRNEA2	6 A 21 3.4 5.6 4 -4.0 -2.4 17 2.0 21.3 2 -2.9 -4.8
7 F 0.0 -0.8 0.0 4.6 -0.6 23.1 1.5 23.1 -1.5 23.1 0.6 23.1 Frame Column -F4CRANEA5F4CRANEA6- F4PAT_LL_1- F4PAT_LL_2- F4UNB_SL_L- F4UNB_SL_R-	10 bodd +Colliderat +T-SCRAUEA 110 bodd +Colliderat +T-SCRAUEA 120 bodd +Colliderat +T-SCRAUEA 20 bodd +Colliderat +T-SCRAUEA 20 bodd +Colliderat +T-SCRAUEA 20 bodd +Colliderat +T-SCRAUEA	6 K 5 4.0 -2.4 20 -3.4 5.6 18 -2.0 21.3 3 2.9 -4.8
Line Horiz Vert Horiz <t< td=""><td>21 Dead+0.75Snow+0.3Wind_Right1+0.75Slide_Snow+0.75F3CRANEA4 22 Dead+Colloteral+F4CRANEA2</td><td>6 F 17 1.5 26.6 18 -1.5 26.6 19 1.0 42.6 6 -0.4 -6.8</td></t<>	21 Dead+0.75Snow+0.3Wind_Right1+0.75Slide_Snow+0.75F3CRANEA4 22 Dead+Colloteral+F4CRANEA2	6 F 17 1.5 26.6 18 -1.5 26.6 19 1.0 42.6 6 -0.4 -6.8
7 F -1.0 39.1 1.0 39.1 -0.7 5.5 0.7 5.5 -0.2 3.6 0.2 3.6 Frame ColumnDadCollateralLiveSnowWind_Laft1Wind_Kight1-	23 Dead+Collared+F4CRANEA3 24 Dead+Collared+F4CRANEA6 25 Dead+Collared+F4CRANEA6	RIGID FRAME: MAXIMUM REACTIONS Column_Reactions(k)
Line Line Horiz Vert 8 A 0.2 1.6 0.0 0.2 0.9 4.9 0.3 1.4 -5.0 -9.3 4.4 -3.5 8 K -0.2 1.6 0.0 0.2 -0.9 4.9 -0.3 1.4 -4.4 -3.5 5.0 -9.3	26 Dead+0.7557av+0.3Wind_Right1+0.755Bide_Snow+0.75F4CRANEA4 27 Dead+Collateral+F5CRANEA3 28 Dead+Collateral+F5CRANEA3	Frm Col Load Hmax V Load Hmin V Line Line Id H Ymax Id H Ymin
8 F 0.0 3.0 0.0 0.5 0.0 10.7 0.0 3.1 -1.1 -12.1 1.1 -12.1	29 Dead+Colleteral+F5CRANEA6 30 Dead+0.755nore+0.3Wind_Left1+0.75S1ide_Snow+0.75F5CRANEA1 31 Dead+0.755nore+0.3Wind_Right1+0.75S1ide_Snow+0.75F5CRANEA4	7 A 26 3.4 5.8 4 -4.0 -2.4 22 2.0 21.3 2 -2.9 -4.8
Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 8 A – -6.6 – 5.5 2.7 0.4 2.2 – 8.2 1.9 – 6.1 – 0.5 – 0.4 0.5 0.4	32 Dead+Collcteral+F6CRNEA2 33 Dead+Collcteral+F6CRNEA3 34 Dead+Collcteral+F6CRNEA6	7 K 5 4.0 -2.4 25 -3.4 5.6 23 -2.0 21.3 3 2.9 -4.8
8 F -1.1 -7.4 1.1 -7.4 0.6 -14.1 -0.6 -14.1 -0.4 0.0 0.4 0.0	35 Dead+0.75Snow+0.3Wind_Lsti1+0.75Slide_Snow+0.75F6CRANEA1 36 Dead+0.75Snow+0.3Wind_Lsti1+10.75Slide_Snow+0.75F6CRANEA4 37 0.450ead+0.6Wind_Susciton_	7 F 22 1.5 26.6 23 -1.5 26.6 24 1.0 42.6 6 -0.4 -6.8
Frame Column -Selemic_LongMIN_SNOWFSCRANEA1FSCRANEA2FSCRANEA3FSCRANEA3- Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 8 A 0.0 0.0 0.4 2.0 -0.2 18.0 1.7 19.5 0.3 3.8 2.2 5.3	30 0.00bcdr0.00kmlc_Ngintz.00kmlc_Solution 33 0.00bcdr0.00kmlc_Ngintz.00kmlc_Solution 39 Deod+0.6Wind_Kight2+0.6Wind_Suction	RIGID FRAME: MAXIMUM REACTIONS Column_Reactions(k)
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	ANCHOR BOLT SUMMARY	Frm Col Load Hmax V Load Hmin V Line Line Id H Vmax Id H Vmin
Frame Column — FSCRANEAS FSCRANEAS- FSPAT_LL1- FSPAT_LL2- FSUNB_SL_L - FSUNB_SL_R- Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 8 A -0.6 2.9 1.3 4.4 0.8 5.0 0.1 -0.2 0.3 1.6 0.1 0.3	Dia Proj Qty Locate (in) Type (in)	8 A 31 3.3 5.6 4 -3.9 -2.4 27 2.0 21.2 2 -2.9 -4.7
8 A -0.6 2.9 1.3 4.4 0.8 5.0 0.1 -0.2 0.3 1.6 0.1 0.3 8 K -1.3 4.4 0.6 2.9 -0.1 -0.2 -0.8 0.0 0.3 1.6	○ 40 Jamb 3/4* A307 2.50 ○ 20 Endwall 3/4* A307 2.50 ○ 80 Frame 3/4* A307 2.50	8 K 5 3.9 -2.4 30 -3.3 5.6 28 -2.0 21.2 3 2.9 -4.7 8 F 27 1.5 26.4 28 -1.5 26.4
Frame Column ————————————————————————————————————	BUILDING BRACING REACTIONS	8 F 27 1.5 26.4 28 -1.5 26.4 29 1.0 42.5 6 -0.4 -6.7 RIGID FRAME: MAXIMUM REACTIONS
9 A 0.2 1.6 0.0 0.2 0.8 5.0 0.2 1.5 -5.7 -9.8 5.0 -3.7 -3.7 9.8 5.0 -3.7 1.5 -5.7 -9.8 5.0 -3.7 5.0 -3.7 5.7 -9.8 5.0 -3.7 5.7 -9.8 5.0 -3.7 5.7 -9.8 5.0 -0.2 1.5 -5.0 -3.7 5.7 -9.8 5.0 -3.7 5.7 -9.8 5.0 -0.2 1.5 -5.0 -3.7 5.7 -9.8 5.0 -0.2 1.5 -5.0 -3.7 5.7 -9.8 5.0 -1.2 5.0 -0.2 1.5 -5.0 -3.7 5.7 -9.8 5.0 -1.2 5.0 -1.2 3.0 -12.3 3.0 -12.3 3.0 -12.3 3.0 -12.3 3.0 -12.3 3.0 -12.3 3.0 3.0 3.0 3.0 3.0 3.0 3.0 3.1 3.0 3.1 <td>± Reactions(k) Panel_Shear —Wall —ColWindSeismic (lb/ff)</td> <td>Column_Reactions(k)</td>	± Reactions(k) Panel_Shear —Wall —ColWindSeismic (lb/ff)	Column_Reactions(k)
Frame ColumnWind_Left2Wind_Right2Wind_Long1Wind_Long2Seismic_Left Seismic_Right Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 9 A -7.2 -5.7 3.5 0.4 2.4 -8.8 1.4 -6.6 -0.6 0.6 0.4	Loc Line Line Horz Vert Horz Vert Wind Seis Note LEW 4 (h)	Frm Col Load Hmax V Load Hmin V Line Line Id H Ymax Id H Ymin
9 K -7.2 -5.7 3.5 0.4 2.4 -8.8 1.4 -6.6 -0.6 -0.4 0.6 0.4 9 K -3.5 0.4 7.2 -5.7 -1.4 -6.8 -2.4 -8.8 -0.6 0.4 0.6 -0.4 9 F 0.0 -7.9 0.0 -7.9 0.0 -10.0 0.0 -10.0 0.0 0.0 0.0 0.0	F_SW K 4,5 4.2 2.7 1.0 0.6 R_EW 10 (h)	9 A 36 3.8 4.9 4 -4.2 -2.4 32 1.7 20.3 2 -3.3 -4.9
Frame Column -VIN_SNOWF6CRANEA1F6CRANEA2F6CRANEA3F6CRANEA3F6CRANEA5- Line Line Hortz Vert Hortz Vert Hortz Vert Hortz Vert Hortz Vert	NT F 5,6 0.0 4.0 0.0 0.8 (d) F 7,8 0.0 4.0 0.0 0.8 (d)	9 K 5 4.2 -2.4 35 -3.8 4.9 33 -1.7 20.3 3 3.3 -4.9 9 F 32 1.7 24.9 33 -1.7 24.9
9 K 0.3 2.1 -0.9 17.0 1.5 18.5 0.3 2.9 2.6 4.4 -0.9 2.0 9 K -0.3 2.1 -2.6 4.4 -0.3 2.9 -1.5 18.5 0.9 17.0 -1.4 3.5 9 F 0.0 4.5 0.6 21.3 1.7 21.3 -1.7 21.3 -0.6 21.3 -0.5 37.2	(d)X-Bracing above wind beni, base above finish floor (h)Rigid frame at endwall	<u> </u>
Frame Colamn -F6CRANEA6- F6PAT_LL_1- F6PAT_LL_2- F6UNB_SL_L- F6UNB_SL_R- Line Lind Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert		-
9 A 1.4 3.5 0.4 5.1 0.4 -0.1 0.2 1.6 0.2 0.4 9 K 0.9 2.0 -0.4 -0.1 -0.4 5.1 -0.2 0.4 -0.2 1.6 9 F 0.5 37.2 0.0 5.4 0.0 5.4 0.0 3.5 0.0 3.5		
Frame ColumnDeadCollcteral		
10 A 0.7 1.9 0.1 0.2 2.4 5.6 0.7 1.6 -6.5 -10.8 -0.3 -6.8 10 K -0.7 1.9 -0.1 0.2 -2.4 5.6 -0.7 1.6 0.3 -6.8 6.5 -10.8		
Frame ColumnWind_Left2Wind_Right2Wind_Long1Wind_Long2Seismic_Left Seismic_Right Line Line Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert Horiz Vert 10 A -65.2 -7.4 0.0 -3.4 -1.2 -9.5 -1.9 -7.8 -0.2 -0.1 0.2 0.1		
10 K 0.0 -3.4 6.2 -7.4 1.9 -7.8 1.2 -9.5 -0.2 0.1 0.2 -0.1		
Frame Column — MIN.SNOW—— F7UMB_SL_L— F7UMB_SL_R— Line Line Horiz Vert Horiz Vert Horiz Vert 10 A 1.0 2.3 0.7 1.8 0.7 1.1		
10 K -1.0 2.3 -0.7 1.1 -0.7 1.8		

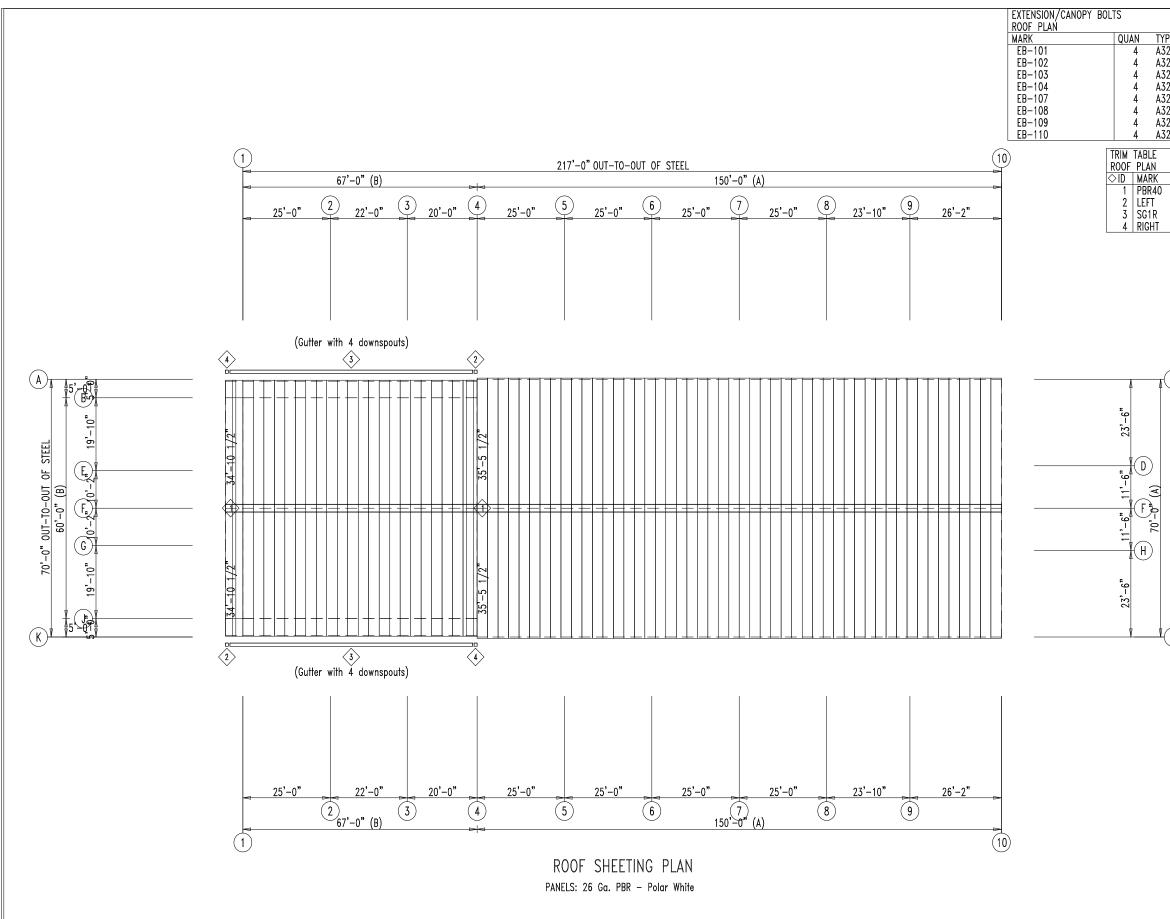
COUUMN LINE	IS TEEL COMPO 432-758- 430 US HW SEMINOLE,	-9900 /Y 180 W
	CUSTOMER INFO	
• •	PROJECT PB50586 HAYLEE DDESSA TEXAS	
	ANCHOR BOLT R DRAWN BY CHECKED BY	DATE 7/26/23 DATE
	BN APPROVED BY , SHEET NO. REV.	7/26/23 DATE 7/26/23 SCALE
	A3 of 4	NTS BY DATE 00-00-00 D D D D D D D D D D D D D D D

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k.) Floor Wind Wind Sels Frm Col Dead Collat Live Press Suct Long Line Line Vert Vert Horz Horz Vert	FRAME LINES: 1 2 3 4 B	Ų
Line Vint Vert Vert Horz Horz Verf 2 E 4.5 1.3 16.7 -5.8 4.2 0.0 2 G 4.5 1.3 16.7 -3.8 4.2 0.0		-
ENDWALL COLUMN: WAXIMUM REACTIONS		
Frm Col Load Hmox V Load Hmin V Line Line Id H Vmax Id H Vmin		
2 E 18 2.5 2.7 19 -2.3 2.7 11 0.0 22.6 2.7		
2 G 18 2.5 2.7 19 -2.3 2.7 11 0.0 22.6		
ENDWALL COLUMN: MAXIMUM REACTIONS		
Frm Col Load Himax V Load Himin V Line Line Id H Ymax Id H Ymin	RIGID FRAME: MAXIMUM REACTIONS	
NOTES FOR REACTIONS	─────────────────────────────────────	
 All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported. Positive reactions are as shown in the sketch. Foundation loads are in 	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
opposite directions. 3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.	1 J 8 3.1 -5.8 4 -4.2 10.8 4 -4.2 10.8 7 2.4 -7.1	
4. Building reactions are based on the following building data: Width (ff) = 60.0 Longth (ff) = 67.0	RIGID FRAME: MAXIMUM REACTIONS Column_Reactions(k) Frm Col Load Hmax V Load Hmin V	
Eewie Height (ft) = 21.0/21.0 Roof Slope (riss/12) = 2.0/ Dead Load (pef) = 2.0 Collideral Load (pef) = 0.5	Line Line Id H Vmax Id H Vman 2 B 6 3.3 -4.4 2 -2.4 -4.6	
Roof Live Load(part) = 20.0 Frame Live Load(pst) = 12.0 Snow Load (psf) = 3.5 Wind Speed (mph) = 105.0	12 -0.5 19.8 5 -1.7 -8.3 2 J 3 2.4 -4.6 5 -3.3 -4.4 12 0.5 19.8 6 1.7 -8.3	
Wind Code = IBC 18 Exposure = C Closure = Enclosed	RIGID FRAME: MAXIMUM REACTIONSColumn_Reactions(k)	
Importance Wind = 1.00 Importance Seismic = 1.00 Seismic Zorer = A Seismic Coreff (for\$s) = 0.24	Frm Col Load Hmax V Load Hmin V Line Line Id H Vmax Id H Vmin	
5. Loading conditions are:	3 B 6 3.2 -2.4 14 -2.4 22.1 12 -0.8 26.5 9 1.7 -6.7 3 J 15 2.4 22.1 5 -3.2 -2.4	
1 Dead+Collisteral 2 Dead+0.6Wind_tsft2 3 Dead+0.6Wind_tsft12 4 Dead+Collisteral+0.5Stive+0.4SWind_Long2R	12 0.8 26.5 10 -1.7 -6.7 RIGID FRAME: MAXIMUM REACTIONS	
5 0.60ead+0.6Wind_Lef1 6 0.60ead+0.6Wind_Right1 7 0.60ead+0.6Wind_Leff2 8 0.60ead+0.6Wind_Right2	− Column_Reactions(k) − − − − − − − − − − − − − − − − − − −	
9 0.6Dead+0.6Wind_Long1L 10 0.6Dead+0.6Wind_Long2L 11 Dead+Colletend+Floor Live	4 B 6 1.7 -1.6 16 -1.8 9.8 13 -1.0 12.7 5 -0.7 -3.8	
12 Dead+Collictera(+0.75Live+0.75Floor_Live 13 Dead+Collictera(+0.75Snow_DrtH+0.75Floor_Live 14 Dead+Collictera(+0.75Live+0.45Wind_Ltfl2+0.75Floor_Live 15 Dead+Collictera(+0.75Live+0.45Wind_Right)2+0.75Floor_Live	4 J 17 1.8 9.8 5 -1.7 -1.6 13 1.0 12.7 6 0.7 -3.8 RIGID FRAME: basic column reactions (k)	
16 Dead+Collaterai+0.755now+0.45Wind_1dft247.0755now_Drift+0.75Floor_Live 17 Dead+Collaterai+0.755now+0.45Wind_1gfta12+0.755now_Drift+0.75Floor_Live 18 0.6Dead+0.6Wind_1gfta12+0.6Wind_Suction 19 0.6Dead+0.6Wind_1ressur+0.6Wind_Long2L	Frame ColumnDeadCollateralLive Line Line Hortz Vert Hortz Vert Hortz Vert	Snow Horiz Vert Hor 0.9 2.2 -4.
ANCHOR BOLT SUMMARY	- 1 J -0.7 2.3 -0.1 0.3 -3.1 8.1 Frame ColumnWind_Left2Wind_Right2Wind_Long1-	-0.9 2.2 5. Wind_Long2Se
Oty Locate Dia (in) Proj (in) Proj (in)	Line Line Horiz Vert Horiz Vert Horiz Vert 1 B -5.9 -11.9 -4.8 -14.1 -5.3 -10.8 1 J 4.8 -14.1 5.9 -11.9 5.3 -10.8	
○ 40 Jamb 3/4" A307 2.50 8 Endwall 3/4" A307 2.50 32 Frame 3/4" A307 2.50 6 6 Frame 3/4" A307 2.50 5 6 6 Frame 3/4" A307 2.50 5 7 <th7< th="" th7<=""> <th7< th=""> <th7< td="" th7<=""><td>Frame Column -MIN_SNOW F1UNB_SL_L- F1UNB_SL_R- Line Line Horiz Vert Horiz Vert 1 B 1.3 3.1 1.0 2.5 1.0 1.5 1 J -1.3 3.1 -1.0 1.5 -1.0 2.5</td><td></td></th7<></th7<></th7<>	Frame Column -MIN_SNOW F1UNB_SL_L- F1UNB_SL_R- Line Line Horiz Vert Horiz Vert 1 B 1.3 3.1 1.0 2.5 1.0 1.5 1 J -1.3 3.1 -1.0 1.5 -1.0 2.5	
BUILDING BRACING REACTIONS	– Frame Column –––––Dead–––– ––Collateral– ––––Live–––– Line Line Horiz Vert Horiz Vert Horiz Vert	Floor Horiz Vert Hori
⊥ Reactions(k) Panel_Shear ——Wali —— Col ——Wind —— ——Seismic —— (ib/tf) Loc Loc Loc Vert Horz Vert Wind Seis Note	2 B -0.2 3.5 0.0 1.1 -0.8 10.4 2 J 0.2 3.5 0.0 1.1 0.8 10.4 - Frame ColumnWind_Right1Wind_Left2Wind_Right2-	Wind_Long1V
L_EW 1 (h) F_SW J 2,3 3.0 5.1 0.4 0.7 (b) R_EW 4 (h) B_SW B 2,3 3.0 5.1 0.4 0.7 (b)	Line Horiz Vert Horiz Vert Horiz Vert 2 B 5.6 -10.8 -3.8 -13.5 4.6 -10.4 2 J 2.7 -17.4 -4.6 -10.4 3.8 -13.5	1.8 -16.1 1.
(b)Wind bent in boy, base above finish floor (b)Rigid frame at endwail	Frame Column Seismic_Right Seismic_Long MIN_SNOW Line Line Horiz Vert Horiz Vert Horiz Vert 2 B 0.4 0.2 0.0 -0.7 -0.3 4.0 2 J 0.4 -0.2 0.0 -0.7 -0.3 4.0	F2UNB_SL_L- F2U Horiz Vert Hor -0.3 3.1 -0. 0.3 1.8 0.
FLOOR COLUMN REACTIONS	 Frame ColumnDeadCollateralLive Line Line Horiz Vert Horiz Vert Horiz Vert	Floor Horiz Vert Hori
Frame Col Max_Vert Vert Vert Vert AncBolt Base Plate (in) Grout Line Line Ld (k) (k) (k) (k) (k) Oty Dia Width Length Thick (in)	3 B -0.3 4.0 0.0 1.7 -1.3 9.4 3 J 0.3 4.0 0.0 1.7 1.3 9.4 - Frame Column Wind_Leff1- -Wind_Right1- Wind_Leff2-	-Wind Right?
3 015.2 11 25.0 4.5 1.5 18.9 2 0.750 4.000 10.50 0.6225 0.0 3 021.0 11 19.2 3.5 1.2 14.6 2 0.750 4.000 10.50 0.6225 0.0 3 025.9 11 19.2 3.5 1.2 14.6 2 0.750 4.000 10.50 0.6225 0.0 3 025.9 11 27.4 4 1.7 20.8 2 0.750 4.000 10.50 0.6225 0.0	Line Horiz Vert Horiz Vert Horiz Vert 3 B -2.0 -14.3 5.5 -8.0 -3.5 -9.9 3 J -5.5 -8.0 2.0 -14.3 -4.0 -3.6	Horiz Vert Hori 4.0 – 3.6 3.
3 046.8 11 25.8 4.7 1.6 19.6 2 0.750 4.000 10.50 0.625 0.0 4 012.0 11 16.3 3.1 1.0 12.2 2 0.750 4.000 10.50 0.500 0.0	Frame Column -Seismic_Laft Seismic_Right -Seismic_Long Line Line Horiz Vert Horiz Vert Horiz Vert 3 B -0.6 -0.3 0.6 0.3 0.0 -0.7	-MIN_SNOW F3U Hortz Vert Hor -0.5 3.6 -0.
4 630.0 11 19.0 3.6 1.1 14.3 2 0.750 4.000 10.50 0.500 0.0 4 648.0 11 16.3 3.1 1.0 12.2 2 0.750 4.000 10.50 0.500 0.0 661.8 631.2 1 0.0 0.0 0.0 2 0.750 4.000 10.50 0.500 0.0	3 J -0.6 0.3 0.6 -0.3 0.0 -0.7 Frame Column Dead Collateral- Live Line Horiz Vert Horiz Vert Horiz Vert	0.5 3.6 0. ²
	4 B -0.2 2.2 0.0 0.6 -0.6 4.6 4 J 0.2 2.2 0.0 0.6 0.6 4.6	0.2 5.6 -0. -0.2 5.6 0.
	Frame Column Wind_Left1- Wind_Right1- Wind_Left2- Line Line Horiz Vert Horiz Vert Horiz Vert 4 B -1.0 -8.6 3.0 -4.9 -1.7 -6.4 4 J -3.0 -4.9 1.0 -8.6 -2.3 -2.7	-Wind_Right2\ Horiz Vert Hor 2.3 -2.7 1. 1.7 -6.4 -1.
	Frame Column -Seismic_Left Seismic_RightMIN_SNOW Line Line Hortz Vert Hortz Vert Hortz Vert 4 B0.3 -0.2 0.3 0.20.3 1.7	F4UNB_SL_L F4U Horiz Vert Hori
	4 J -0.3 0.2 0.3 -0.2 0.3 1.7	

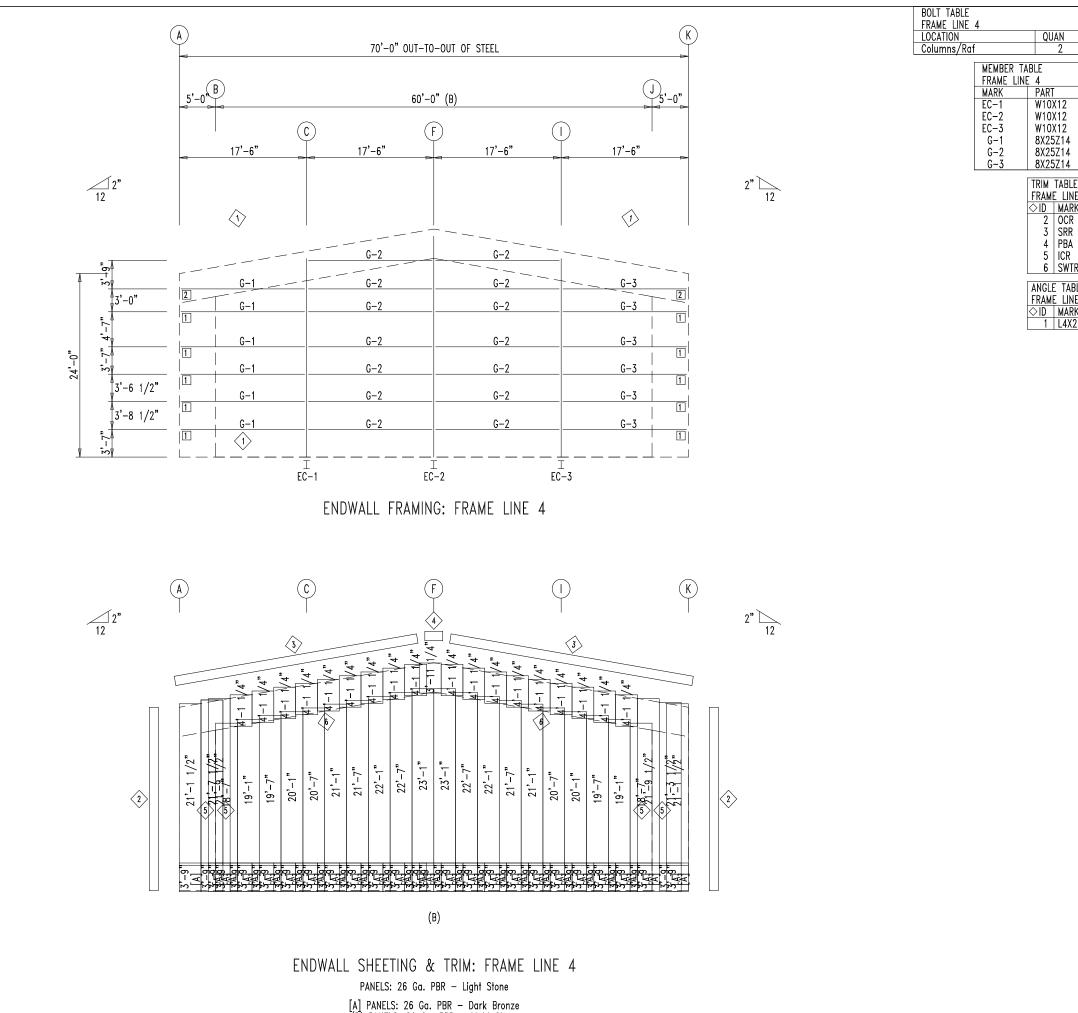
)COLUMN UNE	IS TEEL COMPO 432-758 430 US HW SEMINOLE,	-9900 /Y 180 W
v	CUSTOMER INFO	
	PROJECT PB50586 HAYLEE DDESSA TEXAS	79336
	SHEET NAME/TITLE ANCHOR BOLT R DRAWN BY	REACTIONS DATE 7/26/23
Wind_Leff1Wind_Right1- Horiz Vert Horiz Vert	CHECKED BY ·BN APPROVED BY · SHEET NO. REV.	DATE 7/26/23 DATE 7/26/23 SCALE
noniz veri noniz veri 4.9. −14.1 −5.7. −11.8 5.7. −11.8 4.9 −14.1 −Seismic_Lisff Seismic_Right Noniz Veri Horiz Veri −0.2 −0.1 0.2 0.1 −0.2 0.1 0.2 −0.1	A4 Of 4 REVISIONS NO. NOTES	NTS BY DATE 00-00-00
SnowWind_Left1- Horiz Vert Horiz Vert -0.2 2.82.7 -17.4 0.2 2.8 -5.6 -10.8 		
0.3 3.1Snow		



$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	TYPE A325 A325 A325 A325 A325 A325 A325 A325	DIA LENGTH 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" 3/4" 2 1/2" MAGLE_6 ANGLE_6	BUILD IT WITH STEEL BUILD IT TO LAST STEEL COMPONENTS, INC. 432-758-9900 430 US HVVY 180 W SEMINOLE, TX 79360
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4 4 2 2 4 4 2 2 2 2 2 2 2 2 2 2 2 2 2 2	$\begin{array}{c} 27'-1 \ 1/2"\\ 28'-3 \ 1/2"\\ 27'-3 \ 1/2"\\ 27'-1 \ 1/2"\\ 28'-3 \ 1/2"\\ 27'-1 \ 1/2"\\ 28'-3 \ 1/2"\\ 27'-1 \ 1/2"\\ 28'-3 \ 1/2"\\ 24'-11 \ 1/2"\\ 24'-11 \ 1/2"\\ 23'-9 \ 1/2"\\ 26'-1 \ 1/2"\\ 23'-9 \ 1/2"\\ 24'-11 \ 1/2"\\ 23'-9 \ 1/2"\\ 24'-11 \ 1/2"\\ 31'-6 \ 1/2"\end{array}$	PB50586 HAYLEE DDESSA TEXAS 79336 SHEET NAME/TITLE RODF FRAMING DRAWN BY DATE 7/26/23 CHECKED BY DATE
ROOF PLAN	114 114 4 4 4 4 4 4 4 4 4 4 4 4 4 4 2 L 2 L 2	34'-8 1/4" 27'-1 3/4" 8'-3 9/16" 6'-0 9/16" 6'-2 15/16" 5'-9 3/16" 8'-3 9/16" 8'-3 9/16" 8'-3 9/16" 6'-2 15/16" 6'-2 15/16" 6'-2 15/16" 6'-2 15/16" 6'-2 15/16" 6'-2 15/16" 8'-3 9/16" 31'-1 1/2" 24'-3 1/2" 21'-1 1/2" 21'-1 1/2" 21'-1 1/2" 21'-1 1/2" 21'-1 1/2" 21'-1 1/2" 21'-1 1/4" 21'-3 1/4" 18'-11 1/4" 18'-11 1/4" 18'-11 1/4" 19'-7 3/8" 19'-7 3/8" 19'-7 3/8" 29'-2" 30'-0 1/4" 23'-10"	. 7/26/23 SHEET NO. REV. SCALE E1 of 30 NTS REVISIONS NO. NOTES BY

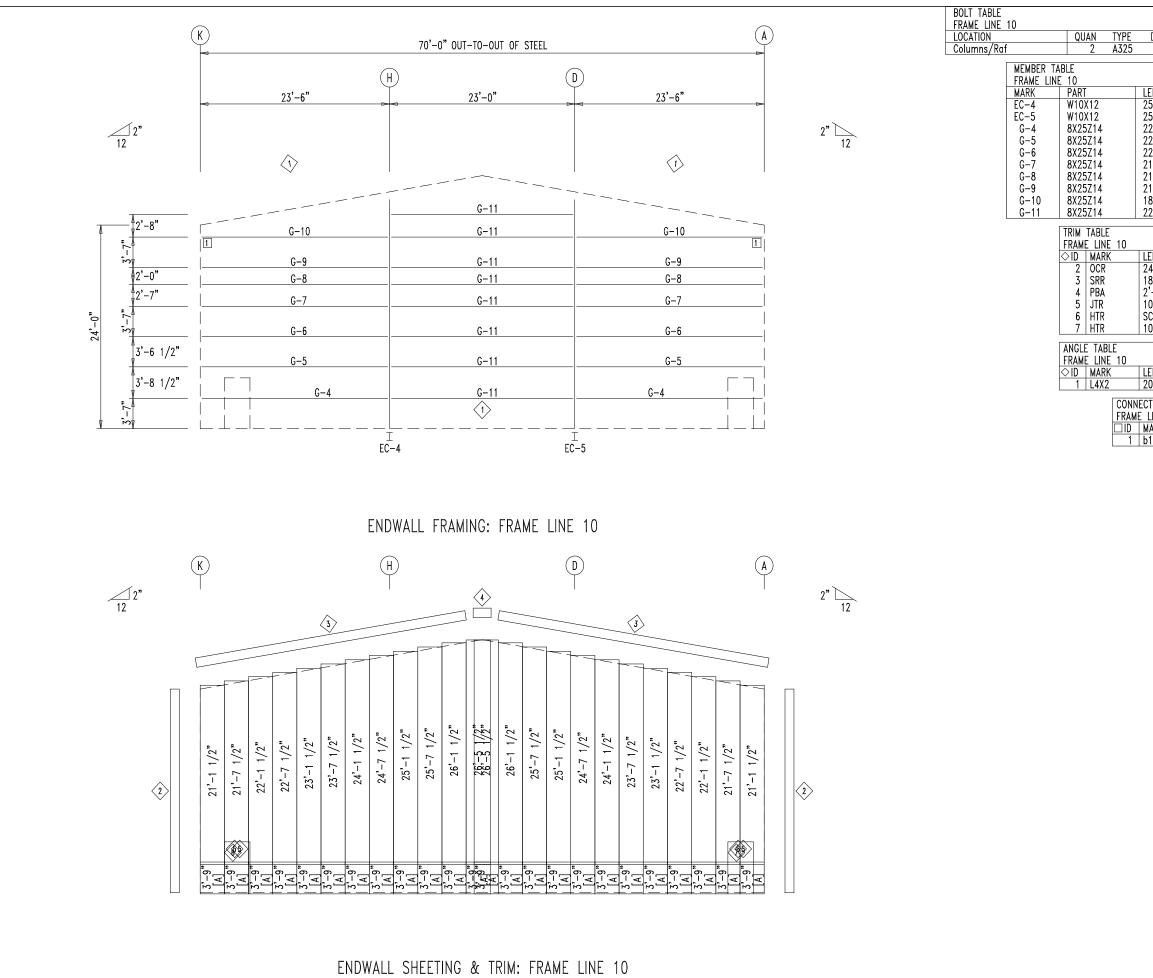


$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	BUILD IT WITH STEEL BUILD IT WITH STEEL COMPONE IS TEEL COMPONE 432-758- 430 US HWY SEMINOLE, T	9900 Y 180 W
A	PROJECT PB50586 HAYLEE DDESSA TEXAS 7 SHEET NAME/TITLE RDDF SHEETING	'9336
	DRAWN BY CHECKED BY ·BN APPROVED BY · SHEET NO. REV. E 2 of 30	DATE 7/26/23 DATE 7/26/23 DATE 7/26/23 SCALE NTS
K		BY DATE 00-00-00



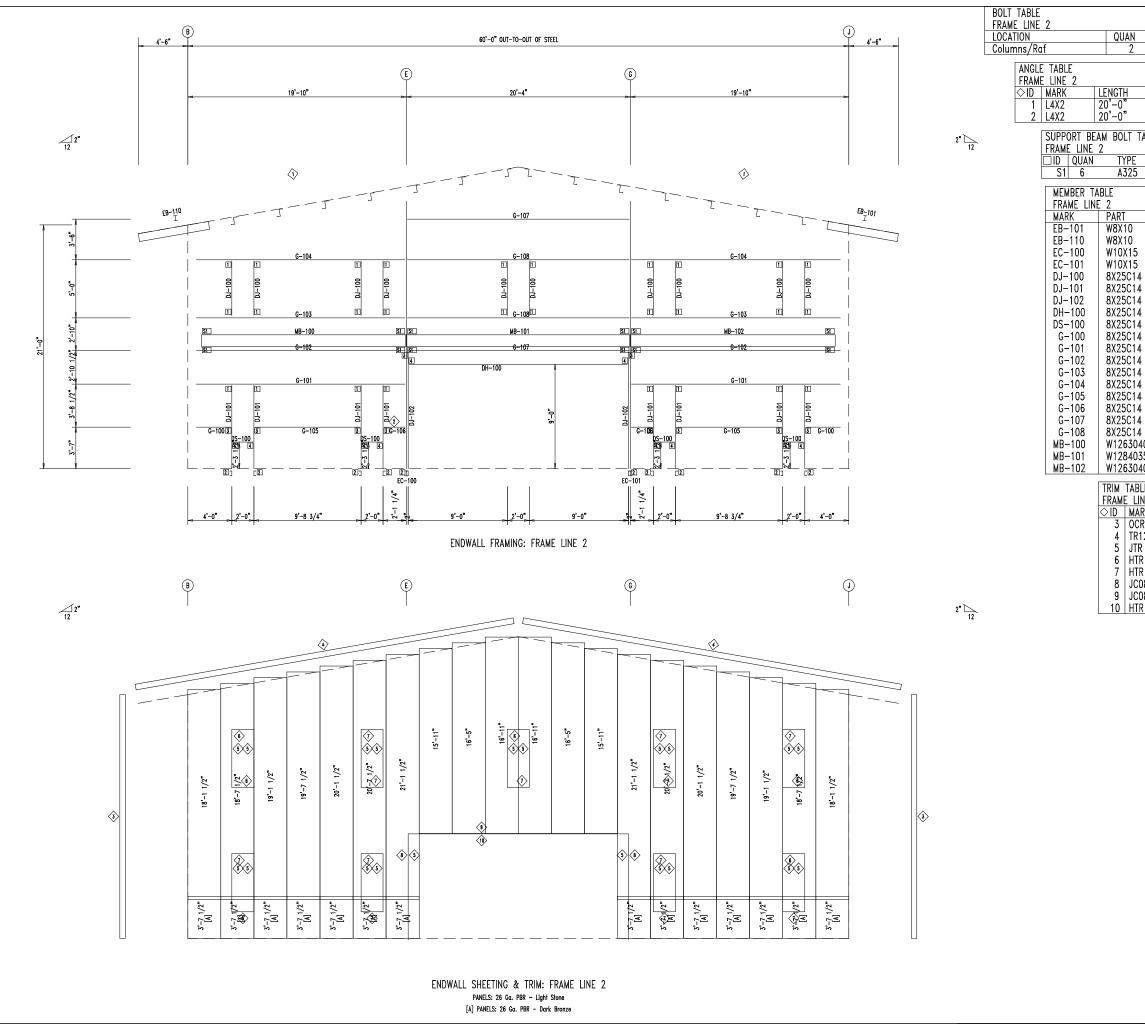
[A] PANELS: 26 Ga. PBR - Dark Bronze (B) PANELS: 26 Ga. PBR - Light Stone

A325	DIA LENGTH 1/2" 1 1/4" ENGTH 5'-11 5/8" 5'-10 5/8" 5'-11 5/8" 7'-2" 6'-10 1/2" 7'-2"		TEEL COMPON 432-758-9 430 US HWY SEMINOLE, T	<u>ENT</u> 9900 180)) W
NE 4 RK LE R 24 R 18 A 22 R 22 R 22 TR 10	ENGTH 4'-6" 8'-6" 1'-0" 1'-0" 6'-6"	CUST	OMER INFO		
(2 20 CONNEC FRAME L	ARK/PART 1	HA` D De	ect 50586 YLEE ESSA TEXAS 7 Et name/title	933	6
		EN DRAW 'BN APPRC	DWALL FRAMIN	DA DA DA DA SC	7/26/23 TE 7/26/23 TE 7/26/23 ALE
		REV	of 30 isions notes 	BY	DATE 00-00-00

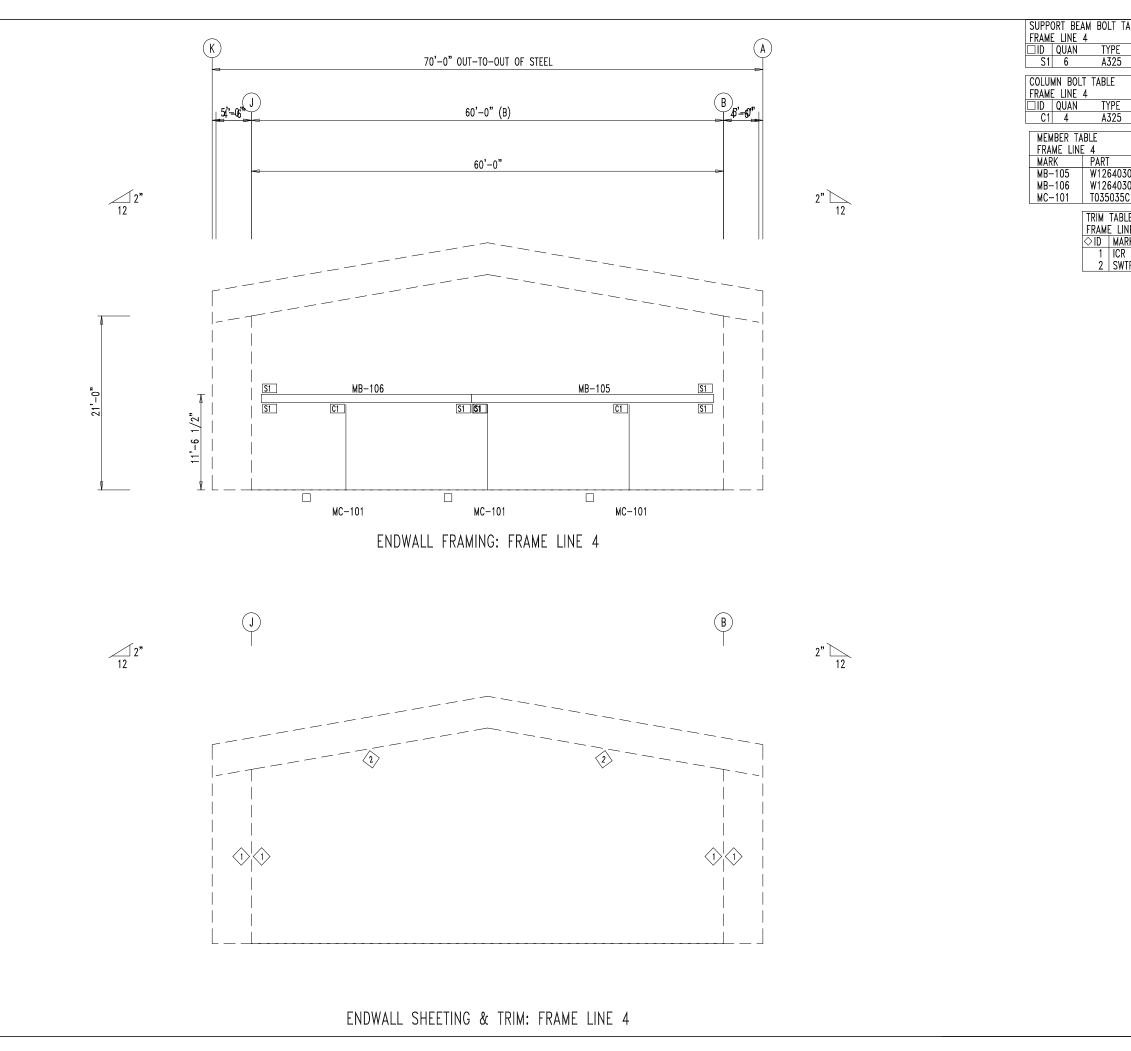


PANELS: 26 Ga. PBR — Light Stone [A] PANELS: 26 Ga. PBR — Dark Bronze

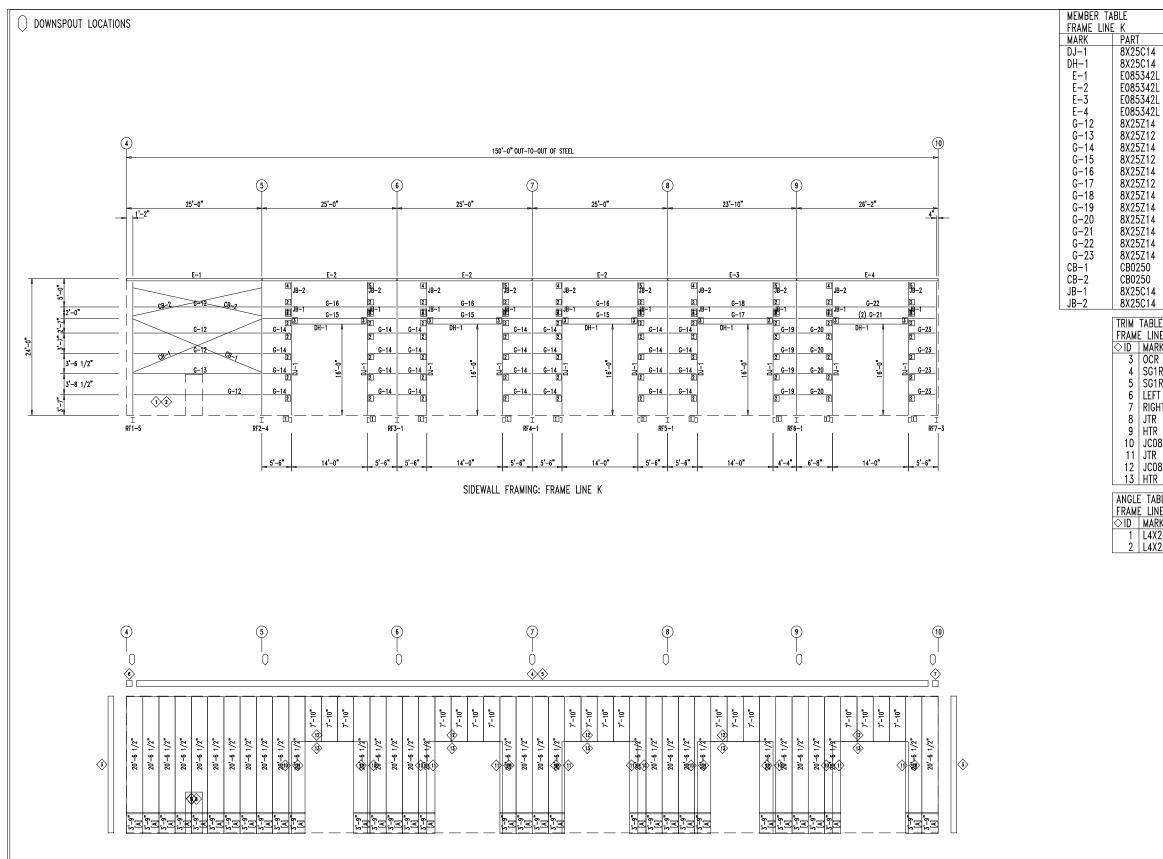
TYPE A325	DIA LENGTH 1/2" 1 1/4" 25'-9 15/16" 22'-3 7/8" 22'-2 1/8" 22'-2 1/8" 22'-0 1/2" 21'-10 7/8" 21'-9 5/8" 21'-9 5/8" 21'-8 3/4" 18'-2 7/8" 22'-4 1/2"	43 430 L	EL BUILD IT OMPONENT 2-758-9900 US HWY 180 NOLE,TX 79	<u>s, in c.</u>
LE NE 10 RK R R R A R	21 -8 3/4 18'-2 7/8" 22'-4 1/2" LENGTH 24'-6" 18'-6" 2'-0" 10'-6"	CUSTOMER INFO PROJECT		
R ABLE NE 10 RK X2 CONN FRAM	IO-6 SCRAP 10'-6" LENGTH 20'-0" ECTION PLATES E LINE 10 MARK/PART	PB50586 HAYLEE DDESSA TE SHEET NAME/TITLE ENDWALL DRAWN BY		
	b1	CHECKED BY BN APPROVED BY	DAT DAT Z EV. SC/	7/26/23 FE 7/26/23 FE 7/26/23
		REVISIONS NO. NOTES	BY	DATE 00-00-00



TYPE A325	DIA LENGTH 1/2" 1 1/4" DETAIL ANGLE_6 DIA LENGTH 5/8" 2"	<u>ISTEEL COMPO</u> 432-758 430 US HW SEMINOLE,	-9900 /Y 180 W
4 4 4	LENGTH 6'-0 9/16" 22'-0 9/16" 22'-0 9/16" 22'-0 9/16" 4'-11 1/2" 7'-2 7/8" 9'-9 7/8" 19'-11 1/2"	CUSTOMER INFO PROJECT PB50586	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$		HAYLEE DDESSA TEXAS SHEET NAME/TITLE ENDWALL FRAMI DRAWN BY	
	20'-2 3/4" 18'-6 15/16" LENGTH 22'-6" 16'-6" 10'-6" SCRAP 10'-6" 21'-0" 21'-0" VECTION PLATES ILINE 2 MARK/PART 0C-3 DC-1 0C-1 0C-2	CHECKED BY 'BN APPROVED BY ' SHEET NO. REV. PN REVISIONS NO. NOTES	DATE 7/26/23 DATE 7/26/23 SCALE NTS BY DATE 00-00-00 0 0 0 0 0 0 0 0 0 0 0 0 0 0

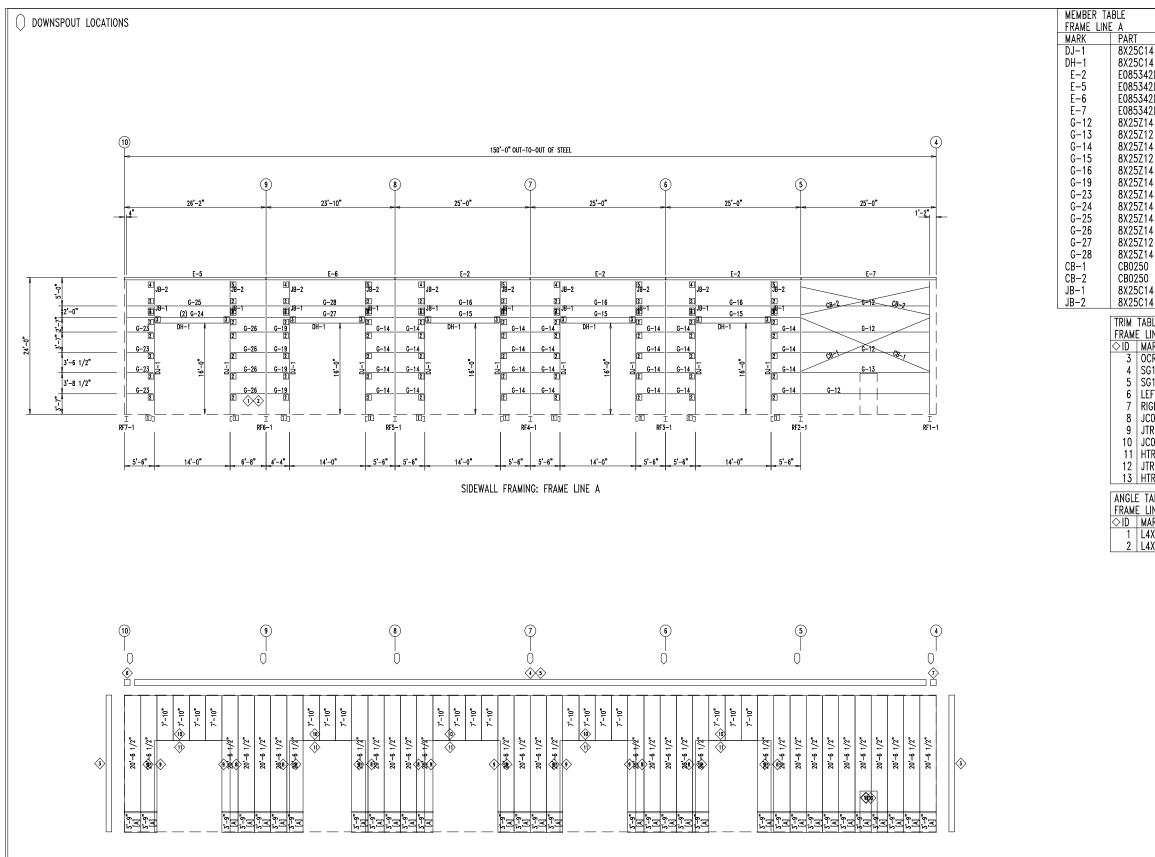


ABLE						
	DIA 3/4"	LENGTH 2 1/2"	BL	JLD IT WITH STEEL	BUILD IT	TOLAST
	DIA 3/4"	LENGTH 2 1/2"	Js	TEEL COM	PONENT	<u>S, INC.</u>
	J/ 4	۷ ۱/۷			58-9900	
	LENGTH		-	430 US H		
30 30	30'-9 26'-9 10'-6	3/8" 3/8"		SEMINOL	E,TX 79	9360
С	10'-6	1/2"				
LE NE 4			CUS	TOMER INFO		
RK	<u>LENGTH</u> 21'-0"		-			
TR	16'-6"					
			PRO. PB	јест 50586		
				YLEE		
					7000	<i>r</i>
				ESSA TEXA	2 \	ь
				i et name/title IDWALL_FR4	AMING	
			DRAW	/N BY	DA ,	те 7/26/23
				KED BY	DA ⁻	те 7/26/23
			·BN	OVED BY	DA	
						7/26/23
				т ю. REV. of 30		ale TS
				/ISIONS NOTES	DY	DATE
					BY	DATE



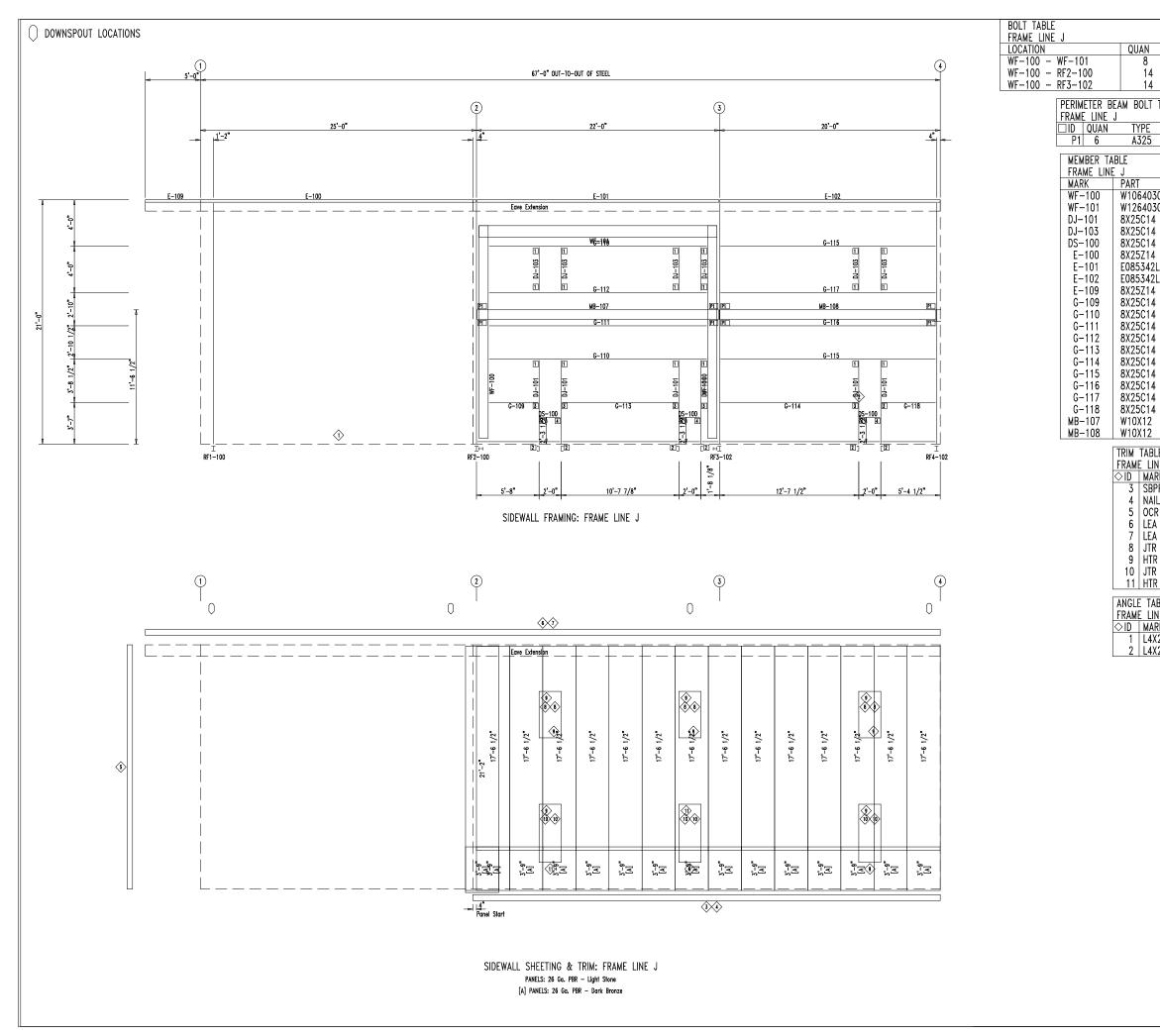
SIDEWALL SHEETING & TRIM: FRAME LINE K PANELS: 26 Ga. PBR – Light Stone [A] PANELS: 26 Ga. PBR – Dark Bronze

4 4 2L 2L 2L 2L 2L 4 2 2 4 4 4 4 4 4 4 4	$\begin{array}{r} \ \ \ \ \ \ \ \ \ \ \ \ \ $		430	<u>сомро</u> 432-758) US HV /INOLE	3-9900 VY 18(<u>s, in</u>)) W	
4 <u>ILE</u> <u>NE K</u> <u>RK</u> 1R 1R 1R 5HT R R R 08 R 08 R 08	24'-10 1/2" 1'-4 1/2" 3'-11 5/8" LENGTH 24'-6" 21'-0" 26'-3" 6" 10'-6" 10'-6" 16'-6" 16'-6" 14'-6"	HA DD She SI Draw	50586 YLEE ESSA et name/ti DEWAL	TEXAS	MING DAT Z	re 7/26 re	
FRAM	LENGTH 20'-0" SCRAP ECTION PLATES E LINE K MARK/PART DC-1 OC-1 OC-2 e1 e2	BN APPRO SHEE PN	OVED BY	REV.	- / DA1	7/26 7/26 ALE	/23

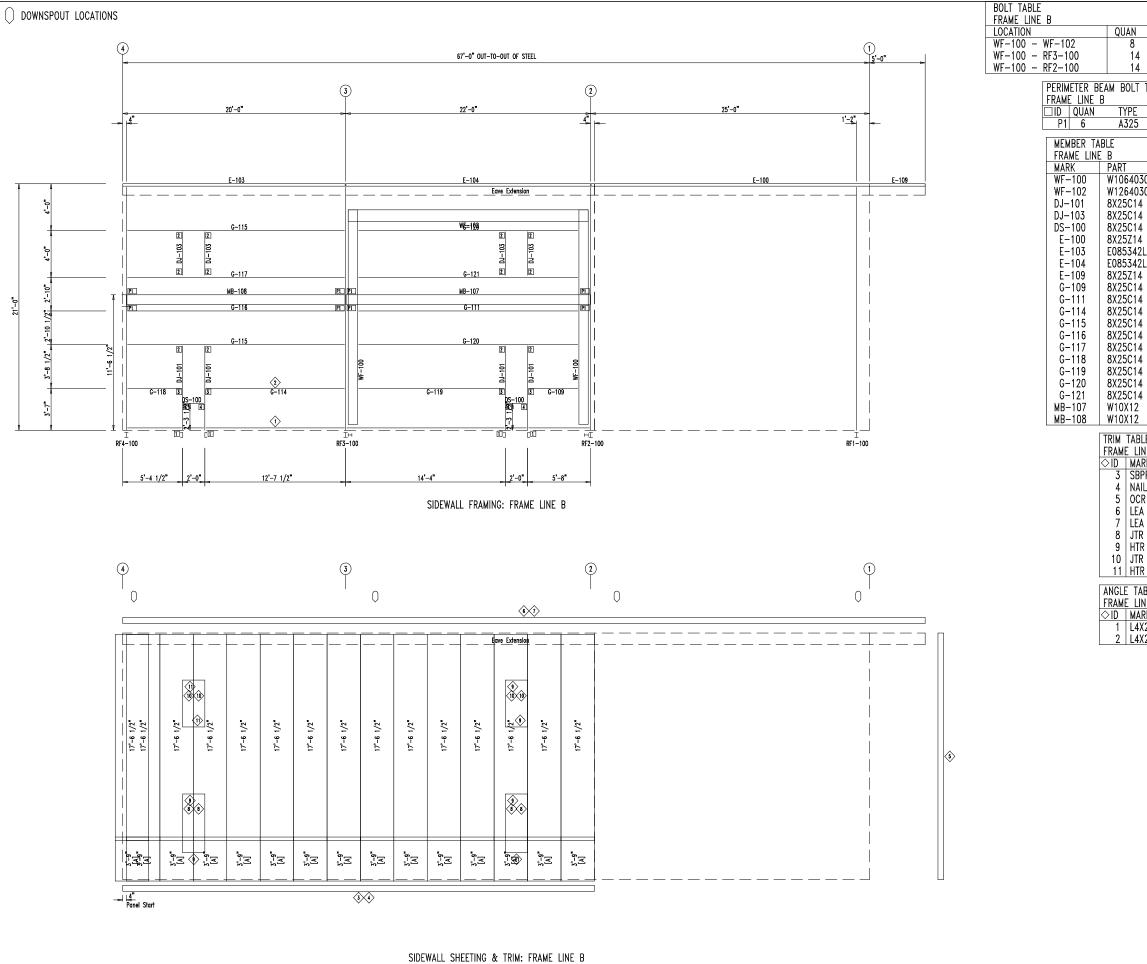


SIDEWALL SHEETING & TRIM: FRAME LINE A PANELS: 26 Ga. PBR – Light Stone [A] PANELS: 26 Ga. PBR – Dark Bronze

4 2L 2L 2L 2L 2L 2L 2L 4 4 4	LENGTH 16'-7 7/8" 13'-11 1/2" 24'-11 1/2" 23'-9 1/2" 24'-11 1/2" 23'-1 1/4" 23'-1 1/4" 23'-1 1/4" 24'-3 1/4" 24'-3 1/4" 24'-5 7/8"	<u>]Ste</u>	EL COMP 432-75 430 US H SEMINOL	58-9900 WY 18) 0 W
4 4 4 2 4 4 4	$\begin{array}{c} 4 - 5 & 7/8 \\ 25' - 1 & 1/4" \\ 25' - 1 & 1/4" \\ 23' - 1 & 1/4" \\ 23' - 1 & 1/4" \\ 23' - 1 & 1/4" \\ 26' - 1 & 1/2" \\ 24' - 10 & 1/2" \\ 1' - 4 & 1/2" \\ 3' - 11 & 5/8" \end{array}$	CUSTOMER PROJECT P B 5 0 5			
BLE INE A IRK TR TR TR FT GHT O8 R 08 R R	LENGTH 24'-6" 21'-0" 26'-3" 6" 16'-6" 16'-6" 16'-6" 14'-6" 14'-6"	HAYLE DDESS Sheet na	ie Sa texas	AMING da	б те 7/26/23
ABLE INE A IRK X2 X2	10'-6" SCRAP LENGTH 20'-0" SCRAP	CHECKED E 'BN APPROVED ' SHEET NO. PN		DA	те 7/26/23 те 7/26/23 аце Т S
FRAM	IECTION PLATES E LINE A MARK/PART DC-1 OC-1 OC-2 e1 e2			BY	DATE 00-00-00

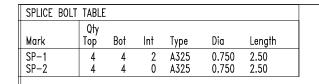


TYPE A325 A325 A325 TABLE	DIA LENGTH 3/4" 2 1/2" 5/8" 2" 5/8" 2" DIA LENGTH 5/8" 2"					
30 30 4 4 4 4 2L 2L 2L 4	LENGTH 18'-6" 20'-3 3/8" 7'-2 7/8" 3'-11 1/2" 1'-11 1/2" 23'-1 1/4" 21'-3 1/4" 18'-11 1/4" 5'-5 5/8"	CUSTOMER INFO				
4 4 4 4 4 4 4 4 4 4 4	4'-5 15/16" 20'-3 3/8" 20'-3 3/8" 20'-3 3/8" 10'-0 3/8" 11'-11 3/8" 18'-11 1/4" 18'-11 1/4" 18'-11 1/4" 4'-4 3/8"	PROJECT PB50586 HAYLEE DDESSA TEXAS 79336 Sheet name/title SIDEWALL FRAMING				
BLE INE J	21'-10' 3/4" 19'-6' 3/4"	DRAWN BY	DATE 7/26/23			
ARK IPR VIL I CR A A R R R R R R	LENGTH 10'-6" 1'-0" 22'-6" 22'-6" 22'-6" SCRAP SCRAP SCRAP 10'-6" 10'-6"	CHECKED BY ·BN APPROVED BY · SHEET NO. REV. PN	DATE 7/26/23 DATE 7/26/23 SCALE NTS			
ABLE INE J ARK X2 X2 CONN FRAM D ID 1 2 3	LENGTH SCRAP 20'-0" IECTION PLATES IE LINE J MARK/PART OC-3 DC-1 OC-1	REVISIONS NO. NOTES	BY DATE 00-00-00			
4	0C-2					



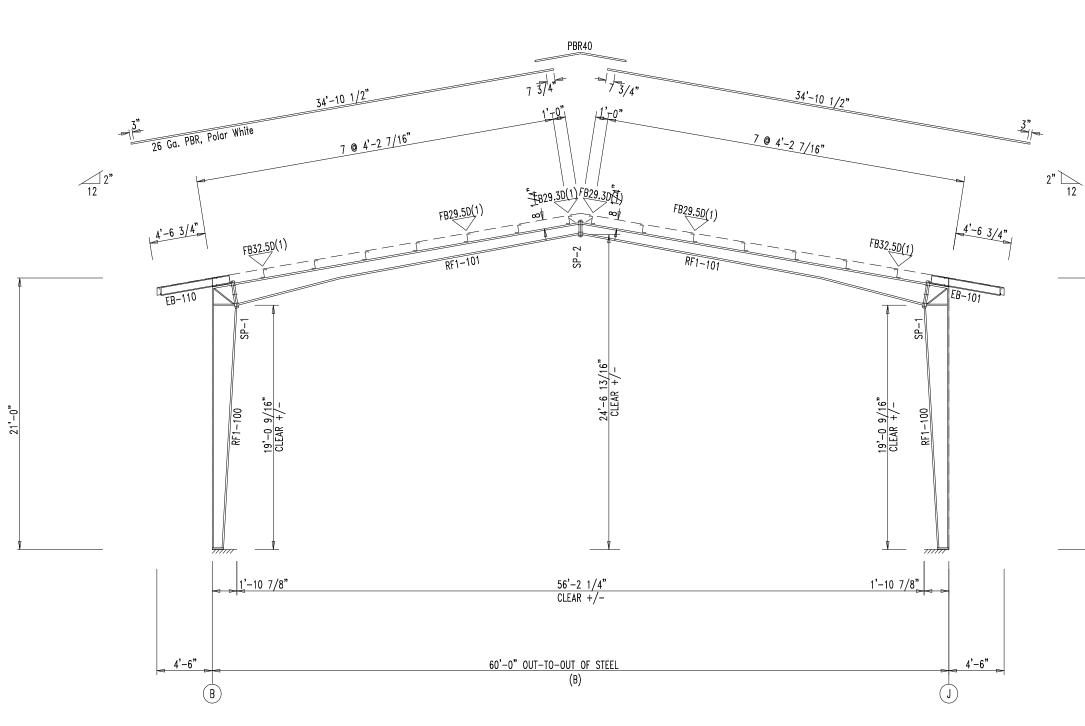
SIDEWALL SHEETING & TRIM: FRAME LINE B PANELS: 26 Gg. PBR – Light Stone [A] PANELS: 26 Gg. PBR – Dark Bronze

		_				
TYPE A325 A325 A325 TABLE	DIA LENGTH 3/4" 2 1/2" 5/8" 2" 5/8" 2" DIA LENGTH		TEEL	н <u>steel</u> <u>сомро</u> 432-750		
	5/8" 2"					
	•		43	0 US H\	NY 18	80 W
			SE	MINOLE	: тх 7	9360
	LENGTH		021		.,.,.,	0000
30	18'-6"					
30	20'-3 $3/8"$	CUST	OMER INF	0		
4	7'-2 7/8" 3'-11 1/2"					
4	1'-11 1/2"					
4	23'-1 1/4"					
2L	18'-11 1/4"					
2L 4	21'-3 1/4" 5'-5 5/8"					
4	4'-5 15/16"					
4	20'-3 3/8"	PRO				
4	11'-11 3/8"	l rb;	50586	>		
4	18'-11 1/4"					
4 4	18'-11 1⁄/4" 18'-11 1/4"	HA'	YLEE			
4	4'-4 3/8"		- ~ ~ ^		7000	
4	13'-1 15/16"	ועם ן	- 2 2 A	TEXAS	1939	56
4	20'-3 3/8"	SHE	ET NAME/1	TITI F		
4	20'-3 3⁄/8" 21'-10 3/4"			ll Fra	MING	
	<u>19'-6 3/4</u>					
		DRAW	'N BY			
BLE INE B						7/26/23
ARK	LENGTH	CHEC	KED BY		DA	ATE
PR	SCRAP	'BN				7/26/23
	SCRAP		0.00			
CR	22'-6" 22'-6"	APPR	OVED BY			ate 7/26/23
A A	22 -6"	· ·				1160/63
R	10'-6"	SHEE	T NO.	REV.		CALE
R	SCRAP	ΡN			Ν	TS
R R	SCRAP 10'-6"					
	10-0		ISIONS			
		NO.	NOTES		BY	
INE B ARK	LENGTH					- 00-00-00
X2	SCRAP					
X2	20'-0"					
CONN	ECTION PLATES					
	E LINE B					
	MARK/PART					
1	DC-1					
23	0C-3 0C-1					
4	0C-2					
<u> </u>						
		1				



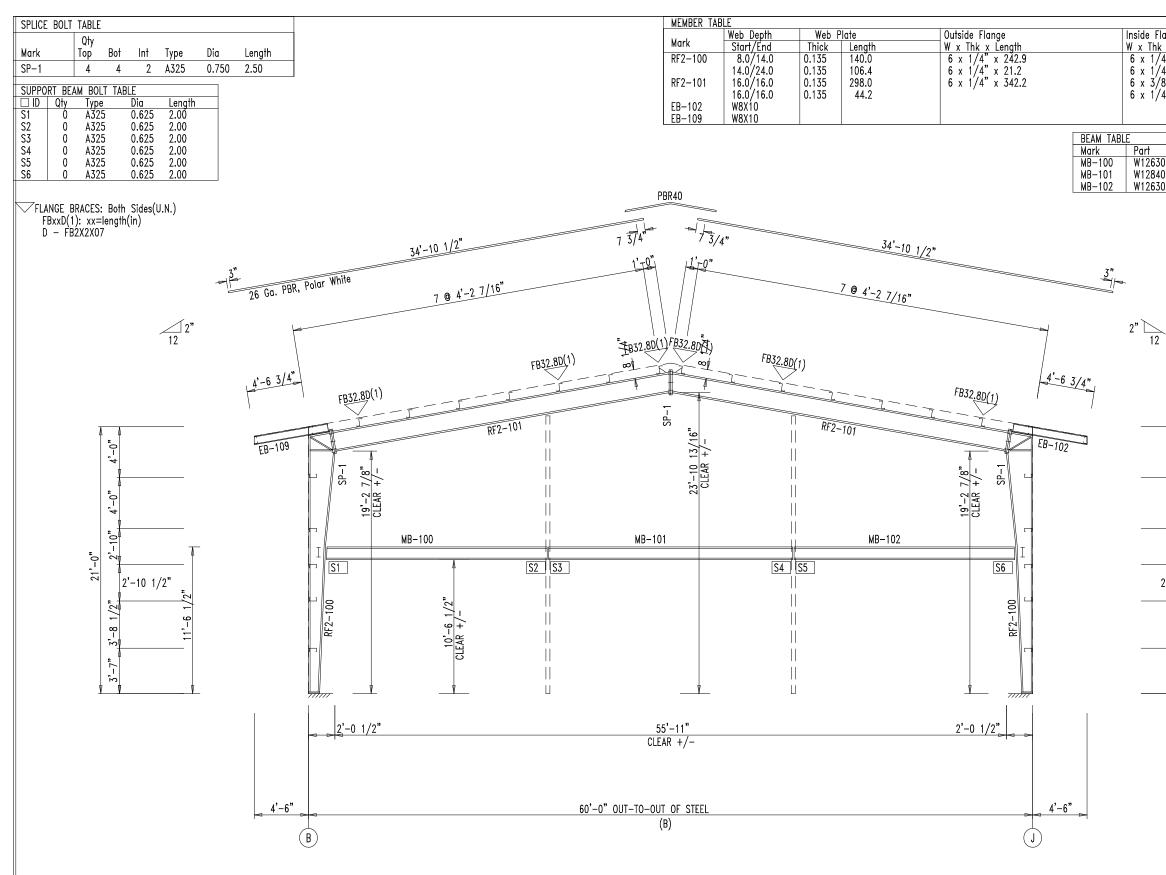
MEMBER TAB	LE				
	Web Depth	Web F	Plate	Outside Flange	Inside Flo
Mark	Start/End	Thick	Length	W x Thk x Ľength	W x Thk
RF1-100	8.0/22.0	0.135	245.9	6 x 3/8" x 242.8	6 x 1/2
				6 x 3/8" x 18.9	,
RF1-101	18.0/10.0	0.135	104.5	6 x 1/4" x 344.5	6 x 1/4
	10.0/ 8.0	0.135	240.0	,	6 x 1/4
EB-101	W8X10				, ,
EB-110	W8X10				

FLANGE BRACES: Both Sides(U.N.) FBxxD(1): xx=length(in) D - FB2X2X07



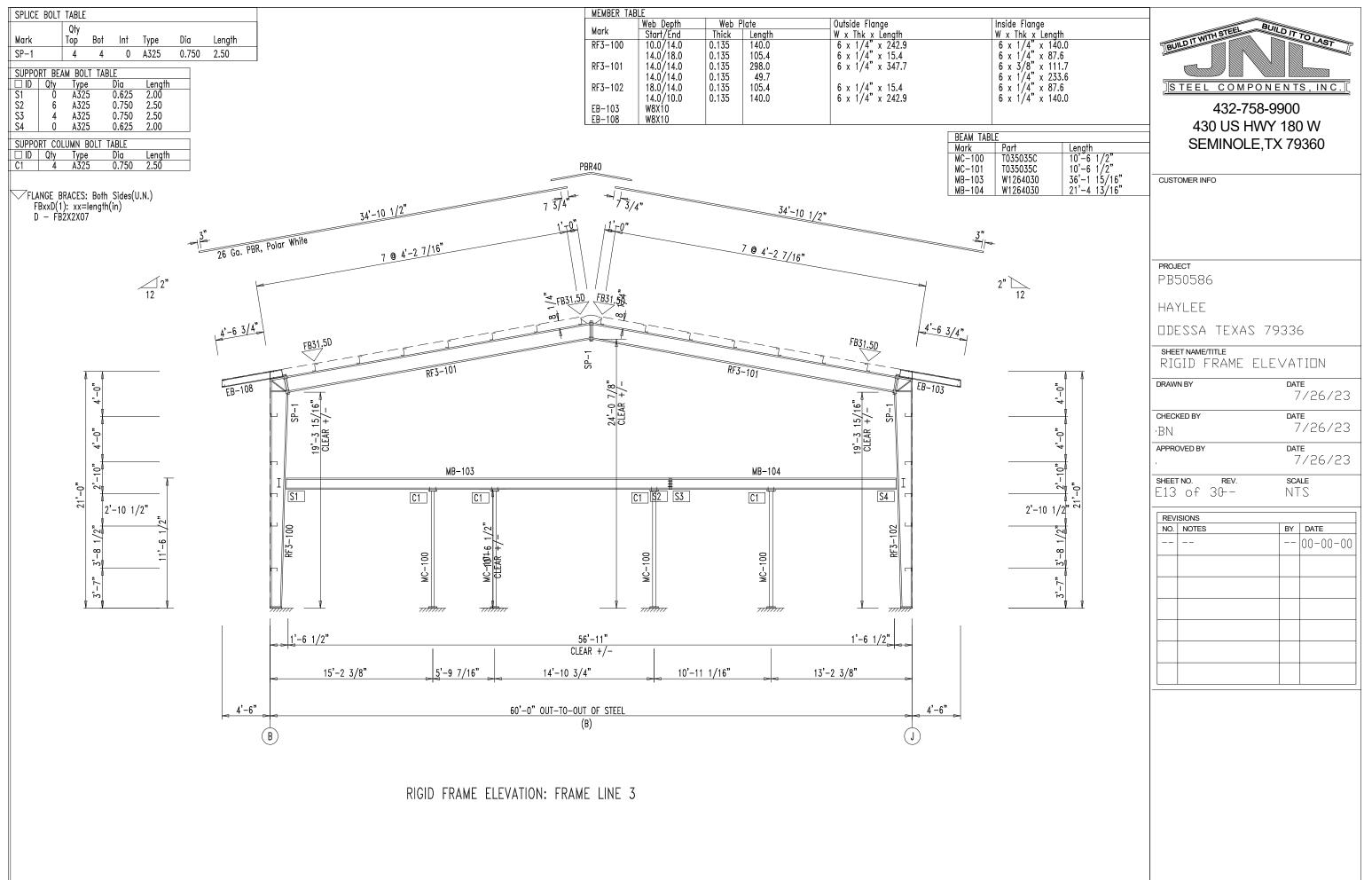
RIGID FRAME ELEVATION: FRAME LINE 1

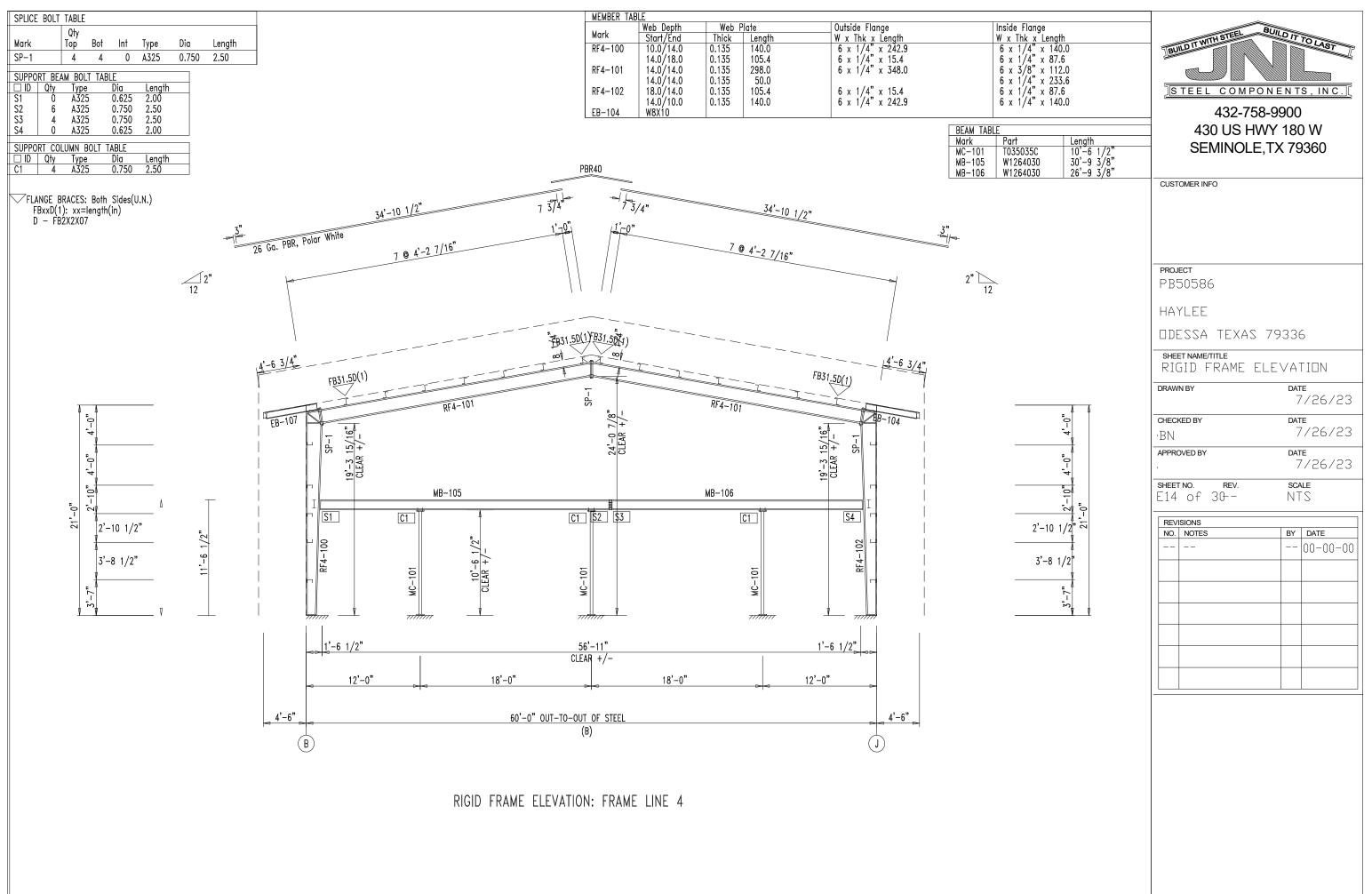
STEEL COMPO 432-758- 430 US HW SEMINOLE,	-9900 Ƴ 180 W
CUSTOMER INFO	
PROJECT PB50586 HAYLEE DDESSA TEXAS	79336
sheet name/title RIGID FRAME EL	EVATION
DRAWN BY	date 7/26/23
снескед ву 'BN	date 7/26/23
APPROVED BY	date 7/26/23
sheet no. rev. E11 of 30	scale NTS
REVISIONS NO. NOTES	BY DATE 00-00-00
	Image: Steel component 432-758- 430 US HW SEMINOLE, CUSTOMER INFO PROJECT PB50586 HAYLEE DESSA TEXAS SHEET NAME/TITLE RIGID FRAME EL DRAWN BY CHECKED BY 'BN APPROVED BY ' SHEET NO. REV. E11 of 30 REVISIONS

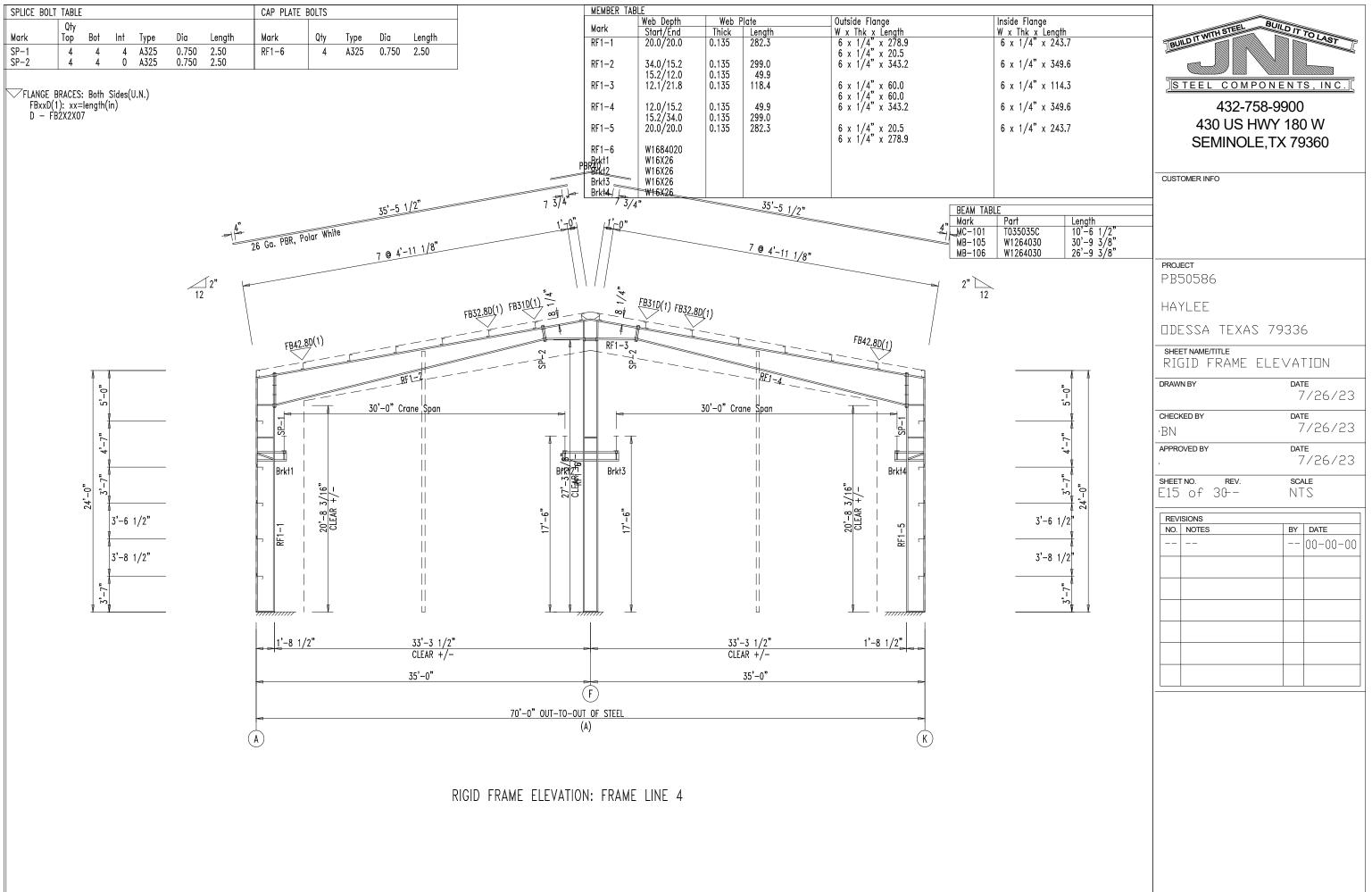


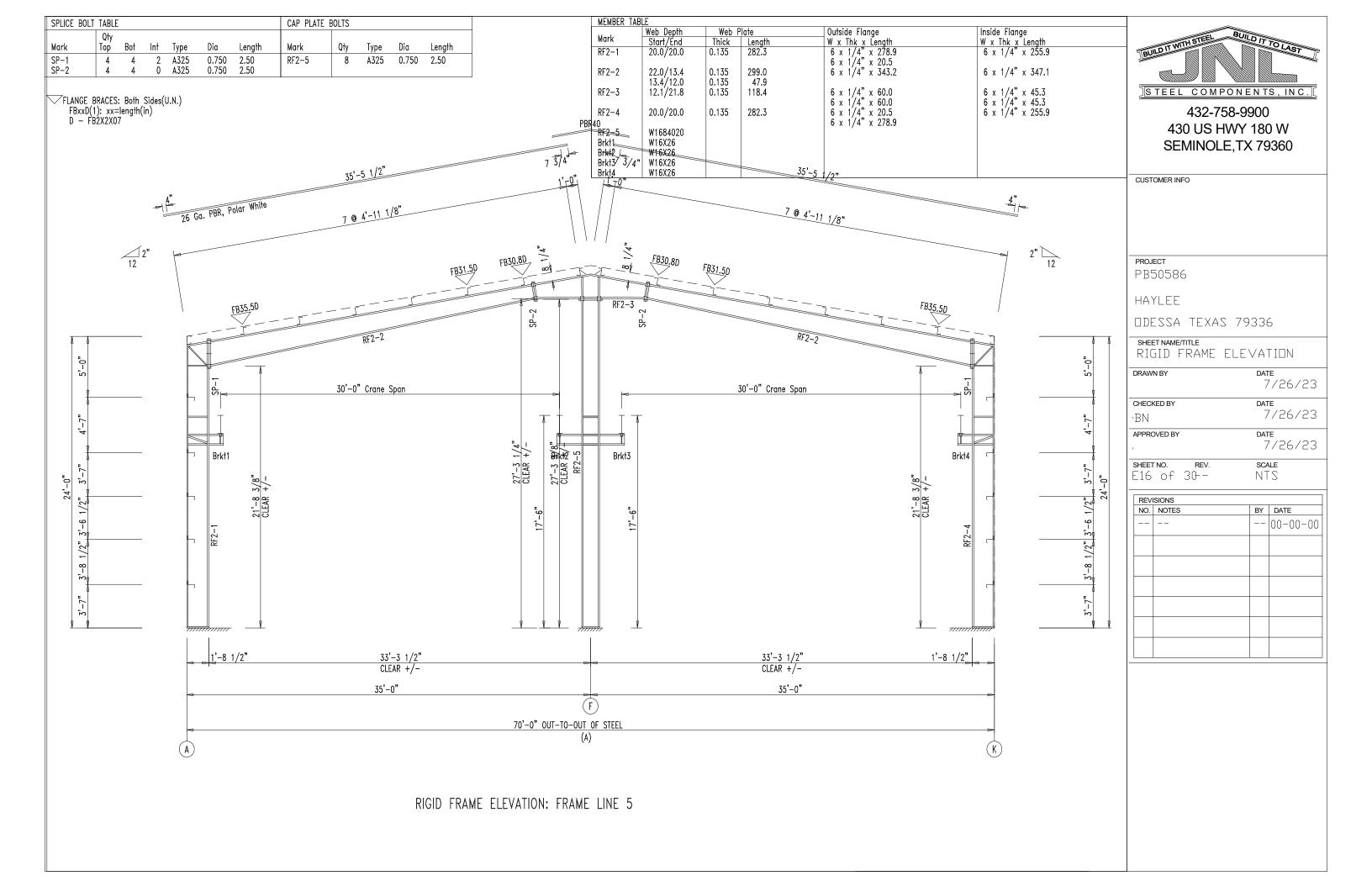
RIGID FRAME ELEVATION: FRAME LINE 2

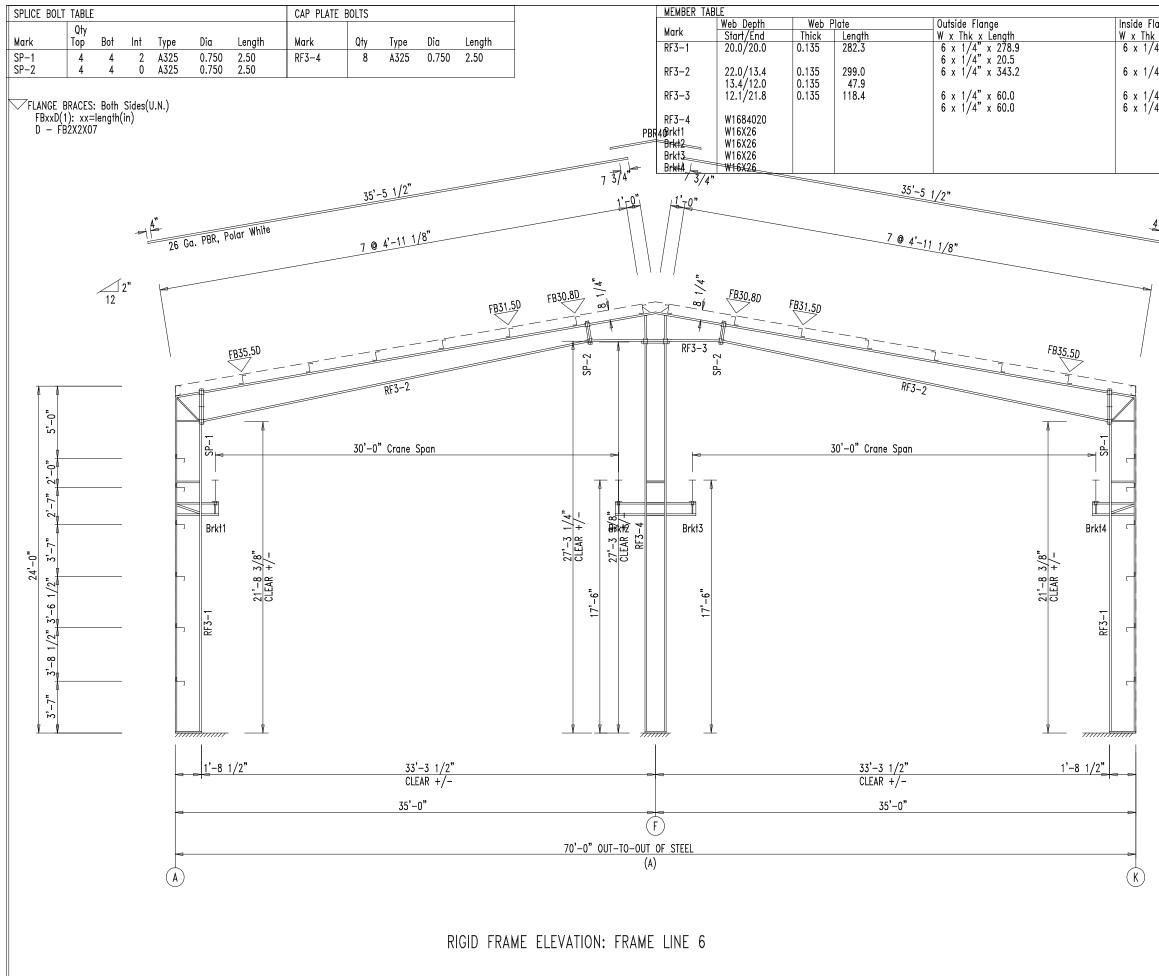
ange x Length 4" x 140.1 4" x 87.0 3" x 106.2 4" x 233.3 Length 040 18'-6 15/16" 040 18'-6 15/16" 040 18'-6 15/16"		Y 180 W
	PROJECT PB50586 HAYLEE DDESSA TEXAS SHEET NAME/TITLE RIGID FRAME EL DRAWN BY CHECKED BY BN APPROVED BY SHEET NO. REV. E12 of 30 REVISIONS NO. NOTES 	



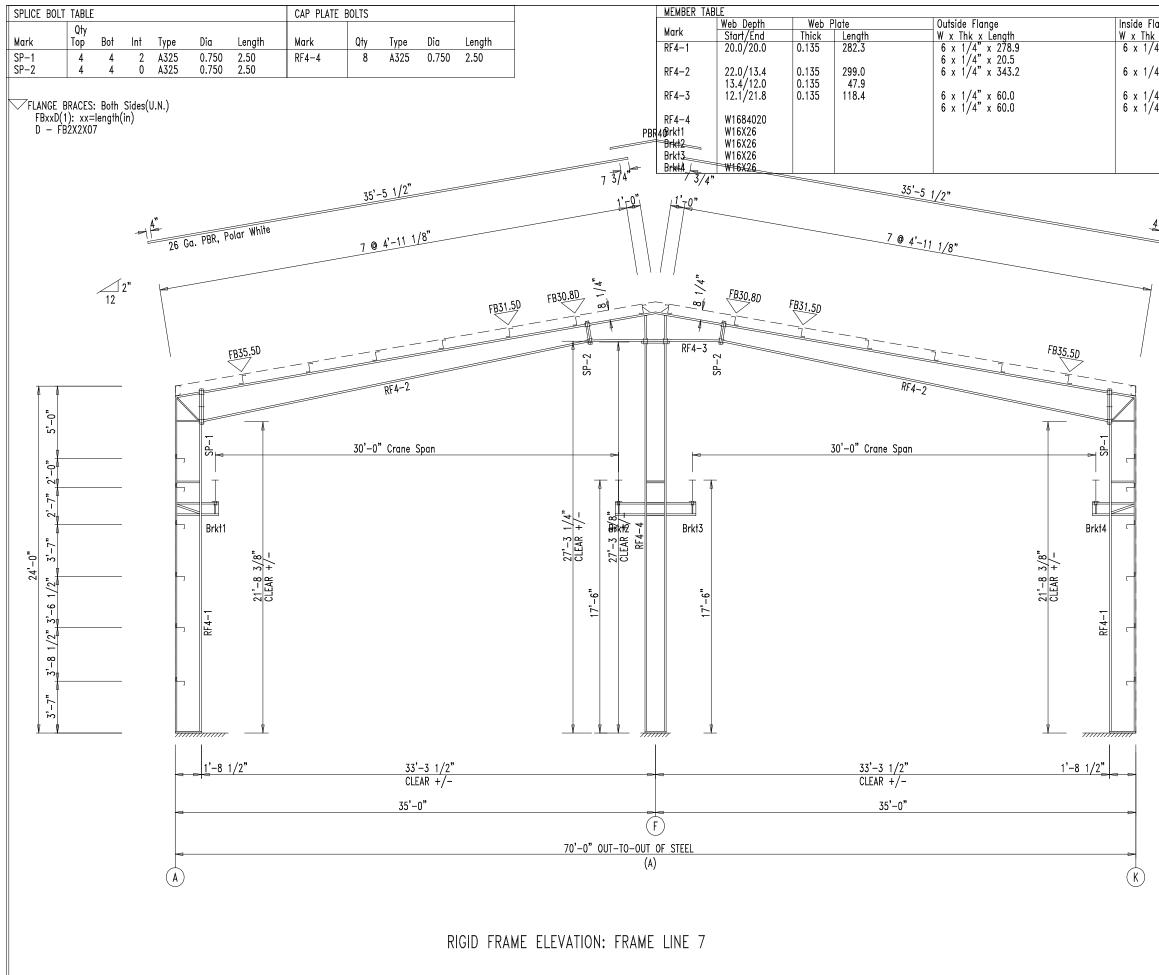




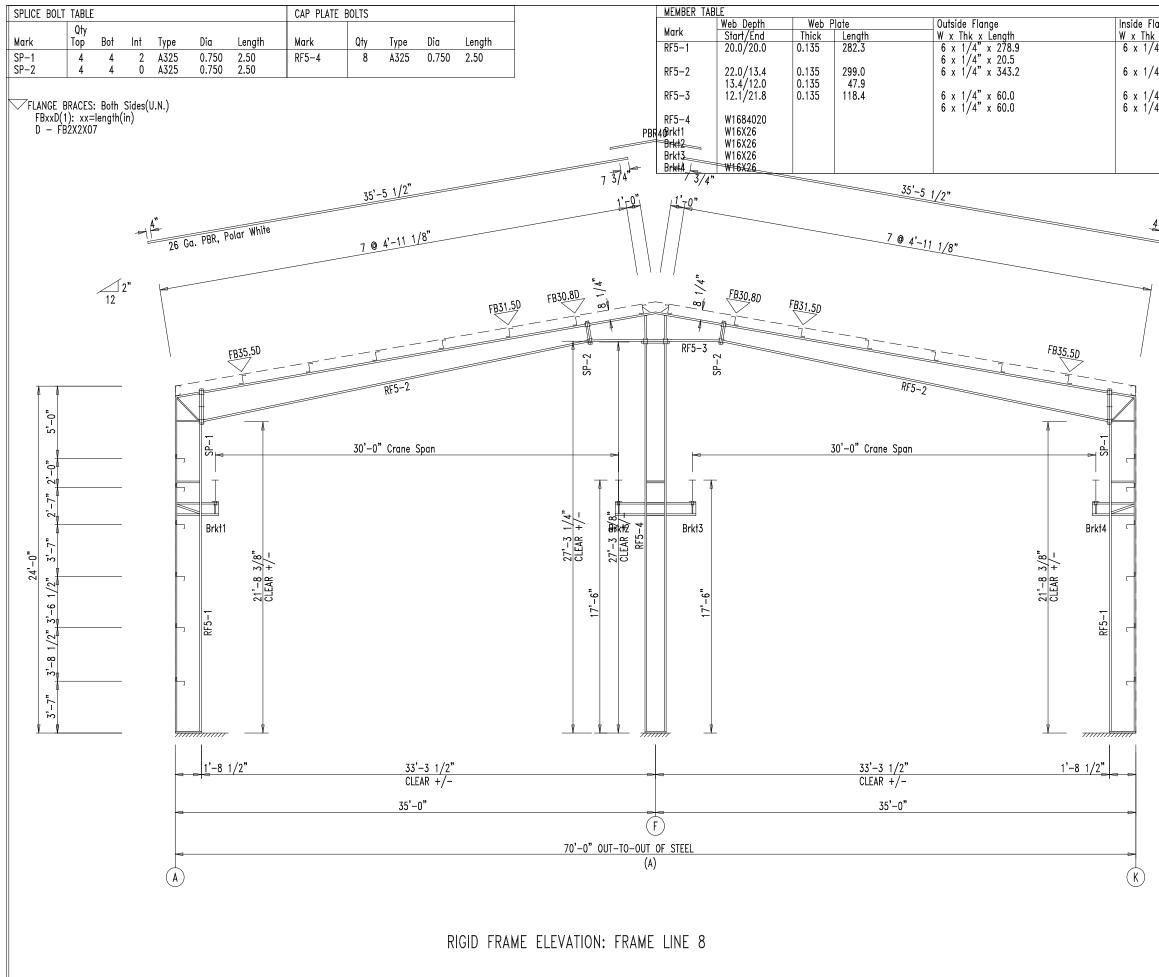




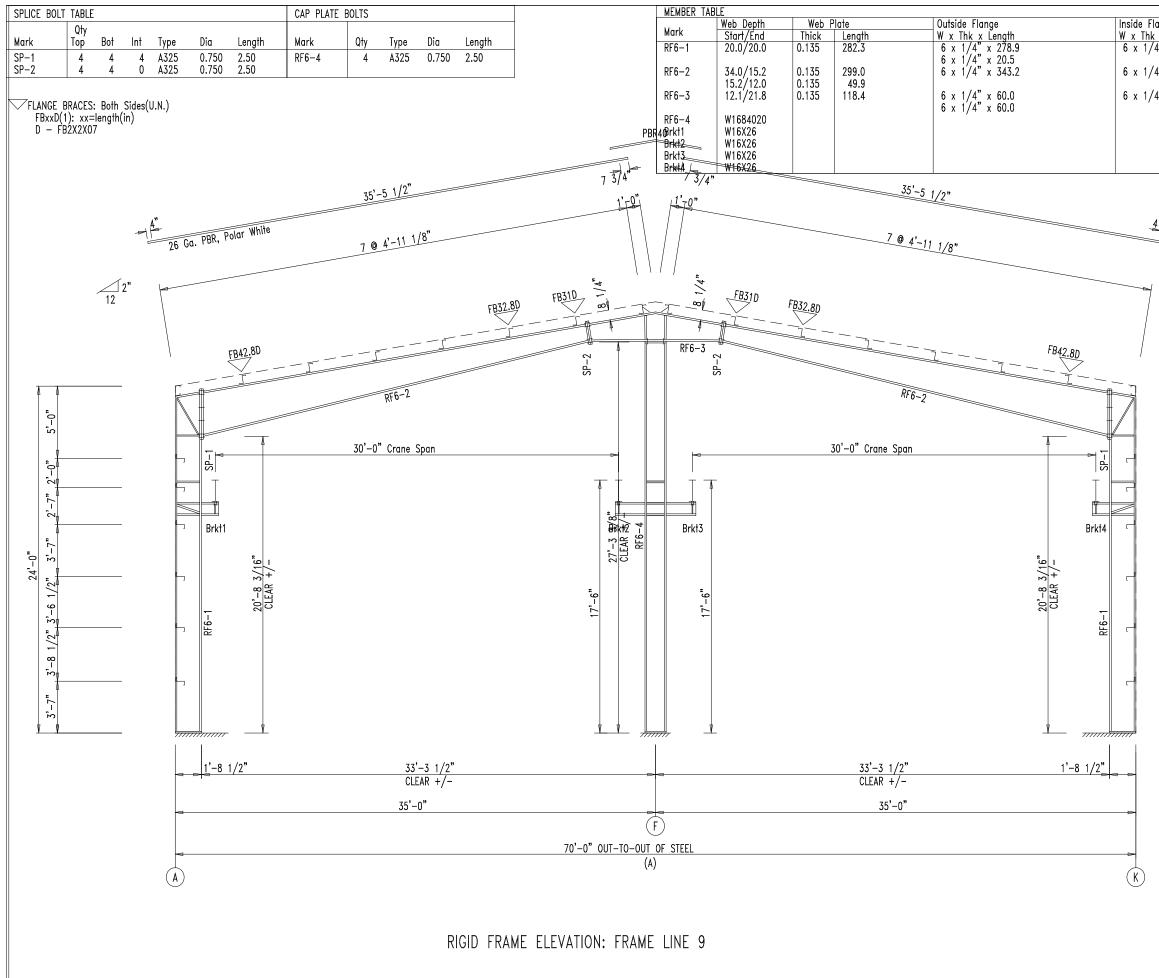
ange x Length 4" x 255.9 4" x 347.1 4" x 45.3 4" x 45.3	BUILD IT WITH STEEL BUILD IT TO LAST BUILD IT WITH STEEL COMPONENTS, IN C. STEEL COMPONENTS, IN C. 432-758-9900 430 US HWY 180 W SEMINOLE, TX 79360
2" 12	CUSTOMER INFO PROJECT PB50586
<u> </u>	HAYLEE DESSA TEXAS 79336 SHEET NAME/TITLE RIGID FRAME ELEVATION DRAWN BY DATE 7/26/23 CHECKED BY DATE ·BN 7/26/23 APPROVED BY SHEET NO. REV. SCALE E17 of 30- NTS
<u>3'-7"</u> <u>3'-8 1/2"</u> <u>3'-6 1/2"</u> <u>3'-</u>	REVISIONS NO. NOTES BY DATE 00-00-00



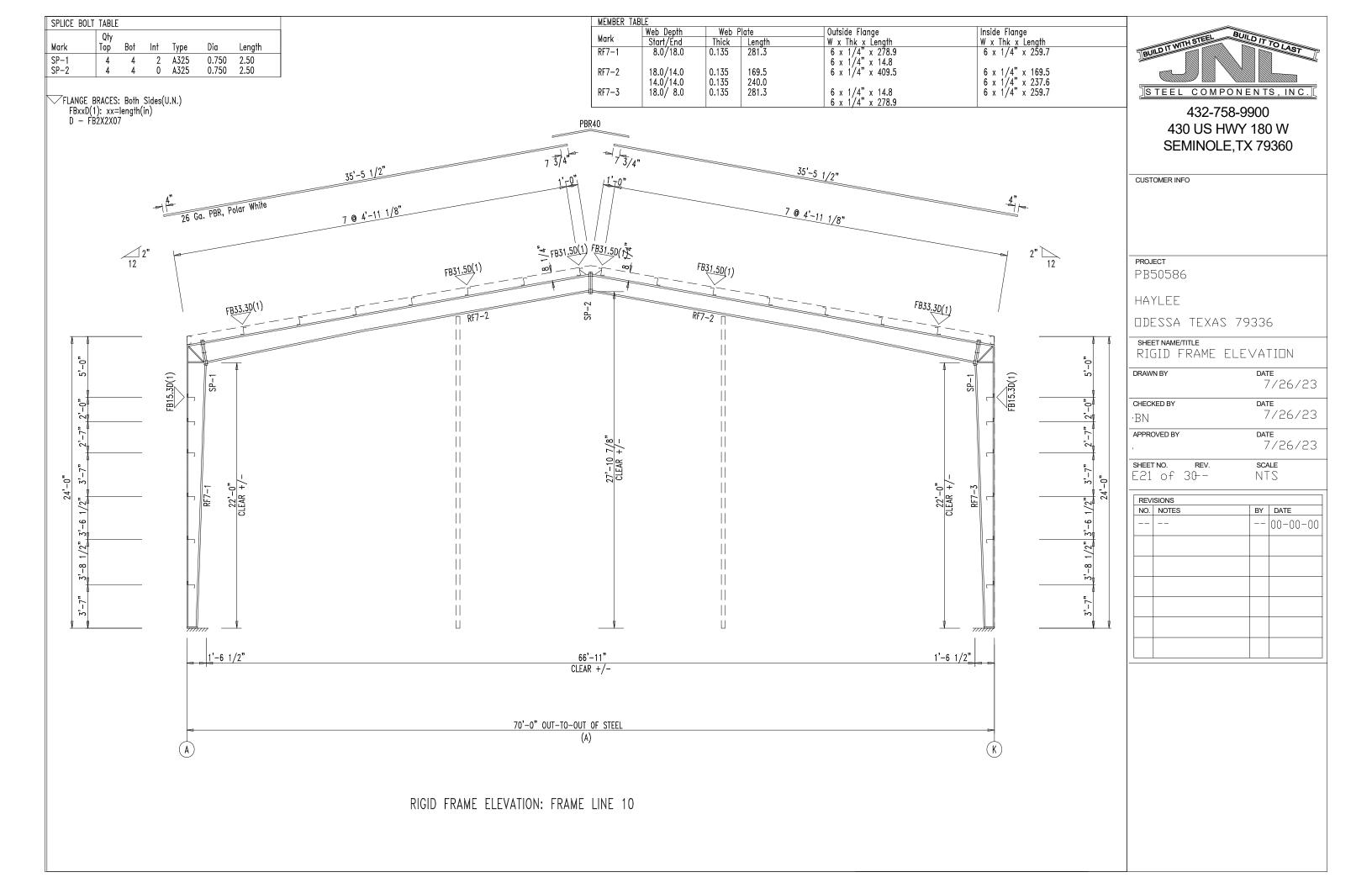
ange x Length 4" x 255.9 4" x 347.1 4" x 45.3 4" x 45.3	BUILD IT WITH STEEL BUILD IT TO LAST IS TEEL COMPONENTS, INC. [432-758-9900 430 US HWY 180 W SEMINOLE, TX 79360			
<u>4</u> " - −	CUSTOMER INFO			
	PROJECT PB50586 HAYLEE DESSA TEXAS SHEET NAME/TITLE RIGID FRAME EL DRAWN BY CHECKED BY ·BN APPROVED BY · SHEET NO. REV. E18 of 30 REVISIONS NO. NOTES -			

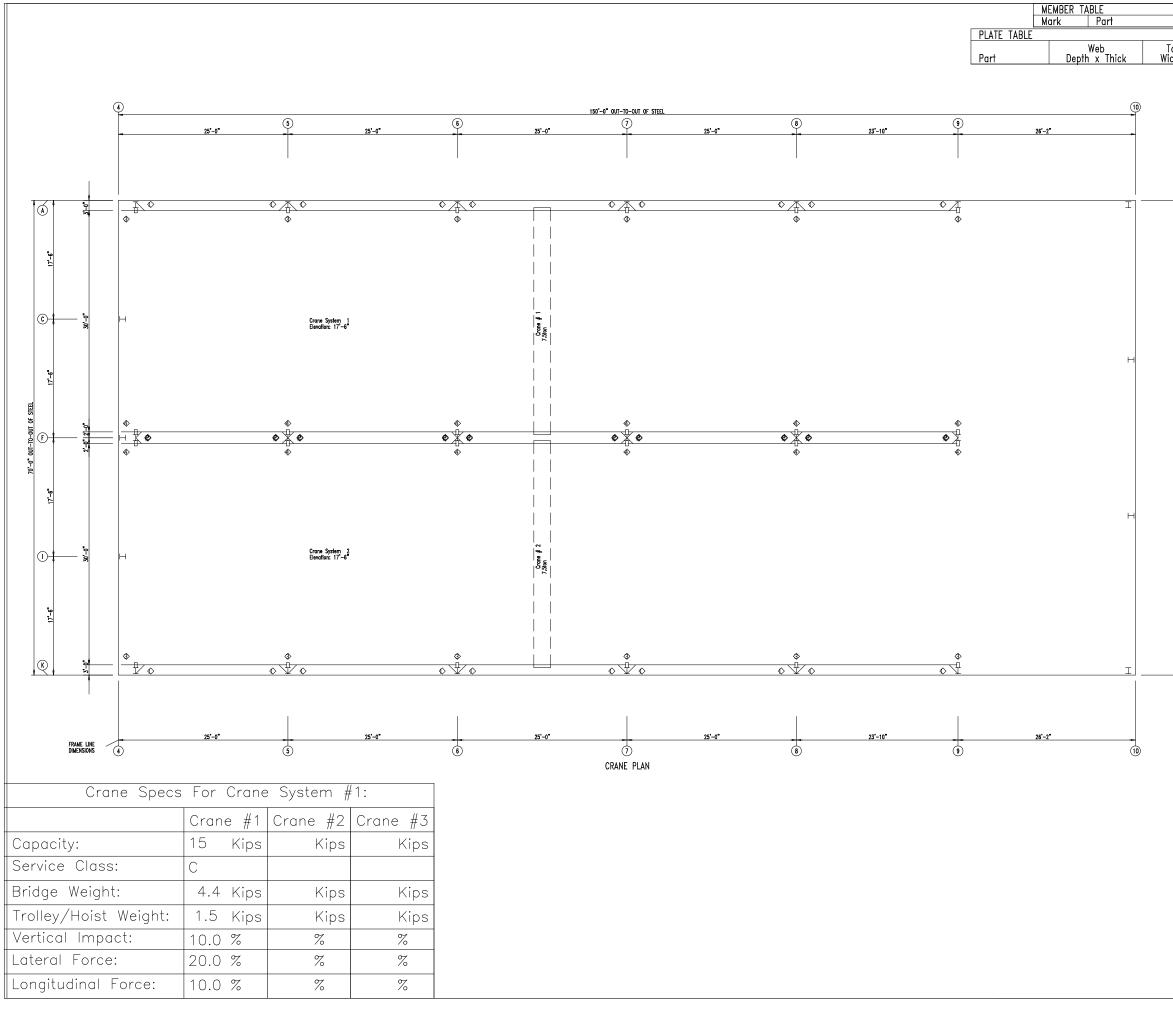


lange (x Length 4" x 255.9 (4" x 347.1 (4" x 45.3 (4" x 45.3 (4" x 45.3	BUILD IT WITH STEEL BUILD IT TO LAST S TEEL COMPONENTS, INC. 432-758-9900 430 US HWY 180 W SEMINOLE, TX 79360			
4" 2" 12 2"-0" 2" 2" -0" 2" -0" -2" -0" -2" -0" -2" -0" -2" -0" -2" -0" -2" -0" -2" -0"	CUSTOMER INFO PROJECT PB50586 HAYLEE DDESSA TEXAS SHEET NAME/TITLE RIGID FRAME EL DRAWN BY CHECKED BY 'BN APPROVED BY 'BN 'BN 'BN APPROVED BY 'BN 'BN 'BN 'BN 'BN 'BN 'BN 'BN	79336		

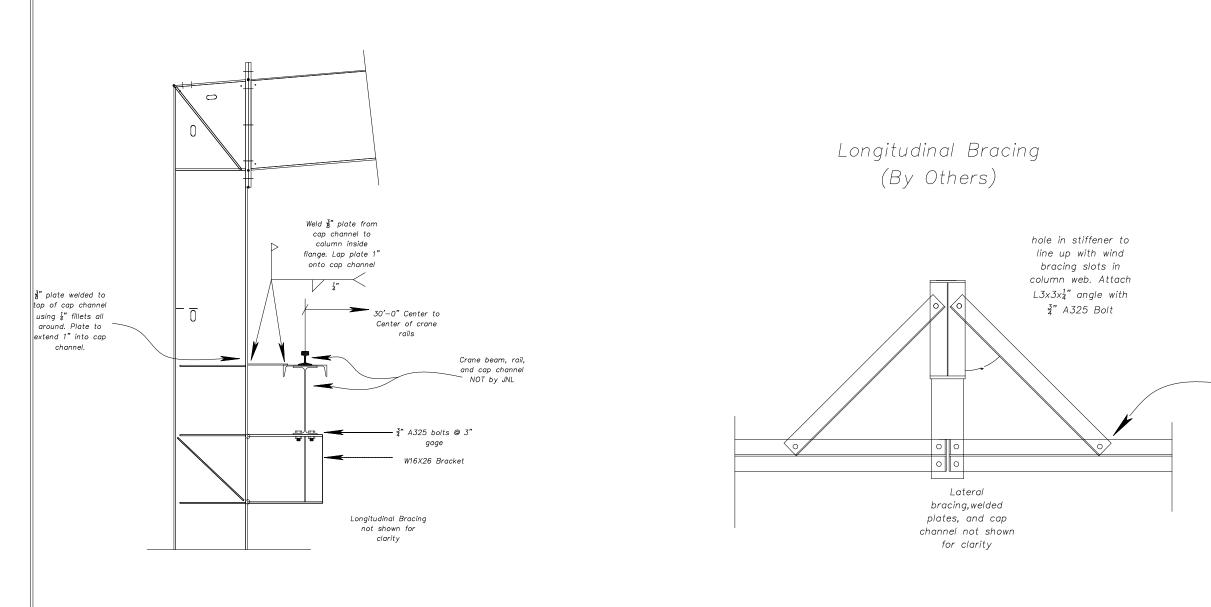


ange <u>x Length</u> 4" x 243.7 4" x 349.6 4" x 114.3	BUILD IT WITH STEEL BUILD IT TO LAST BUILD IT WITH STEEL COMPONENTS, INC. STEEL COMPONENTS, INC. 432-758-9900 430 US HWY 180 W SEMINOLE,TX 79360
t"] /=-	CUSTOMER INFO
2" <u>12</u>	PROJECT PB50586 HAYLEE DDESSA TEXAS 79336 Sheet Name/Title
<u>5'-7" 3'-8 1/2" 3'-6 1/2" 3'-7" 2'-7" 2'-0" 5'-0"</u>	RIGID FRAME ELEVATION DRAWN BY DATE 7/26/23 CHECKED BY DATE ·BN 7/26/23 APPROVED BY DATE . 7/26/23 SHEET NO. REV. E20 of 30- NTS REVISIONS BY NO. NOTES BY DATE 00-00-00 I I I I I I I



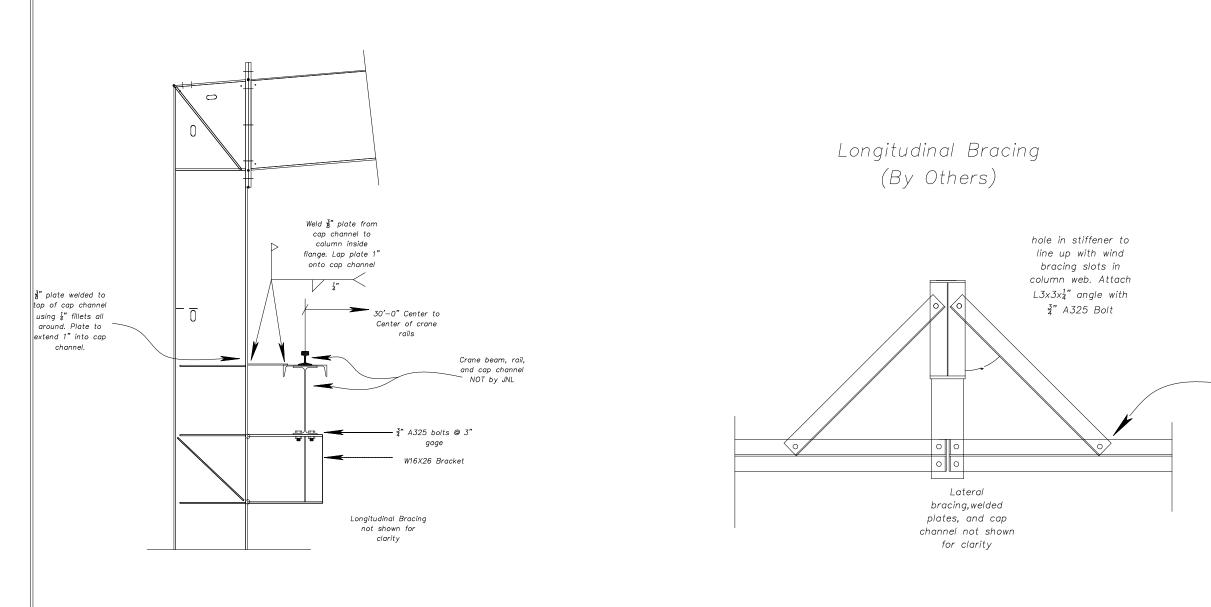


Length			TH STEEL	BUILD IT		
op Flar Ifh x T	nge Ihick	Bottom Flange Width x Thick ANGLE TABLE ◇ ID Part 1 L3X3X316 2 L3X3X316 3 L3X3X316 4 L3X3X316		TEEL COMPO 432-758 430 US HV SEMINOLE	<u>NENT</u> 3-9900 VY 180	<u>s , in c . [</u>) W
•	Â		CUST	OMER INFO		
23'-6"			proj PB	е ст 50586		
			НΑ`	YLEE		
	⊢₪		SHEE	ESSA TEXAS et name/title ANE PLAN	79336	5
11'-6*			DRAW		DAT	r 7/25/23
	-(F)		CHEC	KED BY	DAT	
11'-6"				OVED BY	DAT	
	H		SHEET E22	т но. rev. 2 of 30-	sca N T	\LE
23'-6"			REV NO.	ISIONS NOTES	BY	DATE
	K					



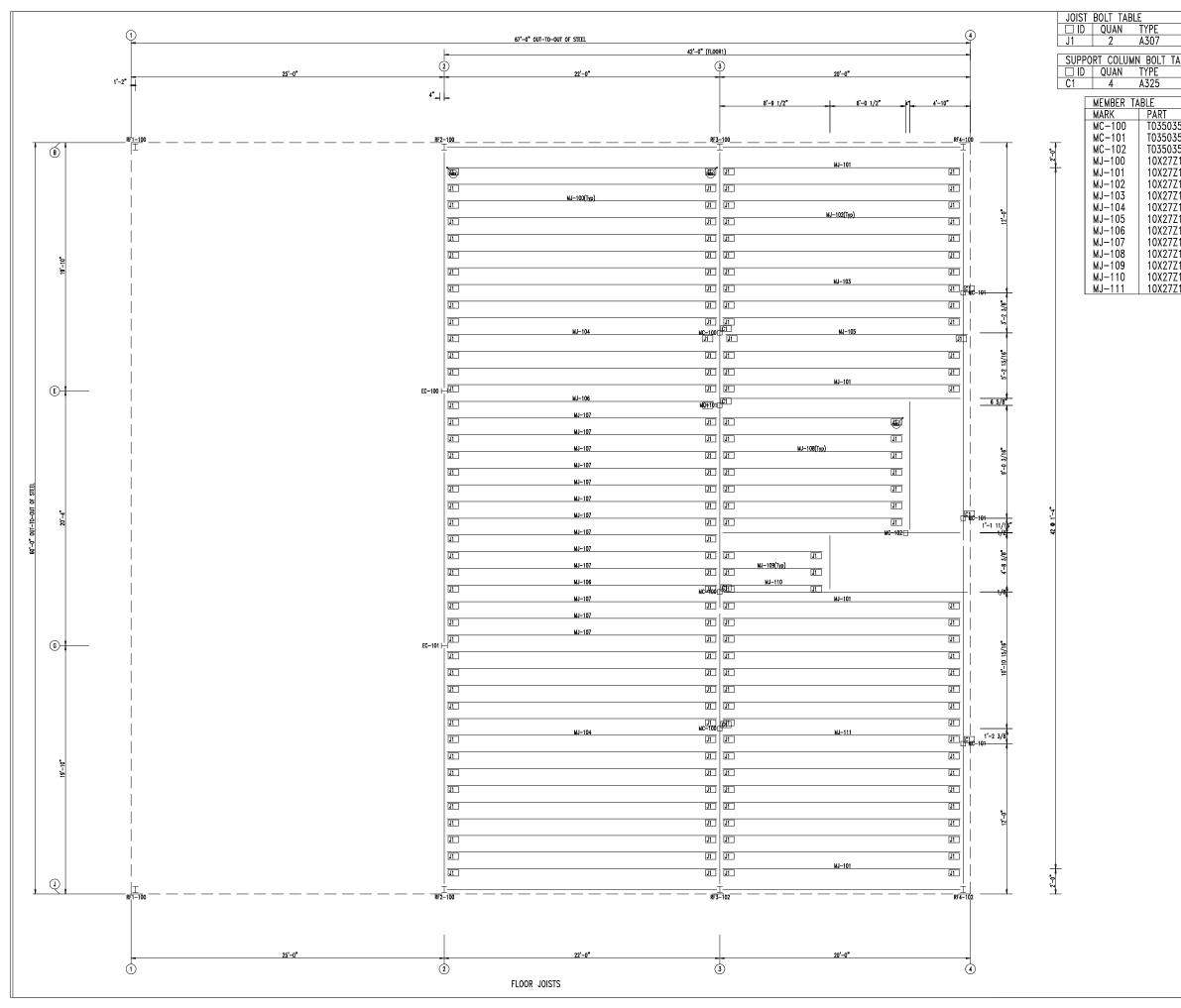
<u>JS TEEL COMPE</u> 432-75 430 US H SEMINOLE,	8-9900 WY 180 W
CUSTOMER INFO	
PREJECT PB50475 EVERLY DDESSA TEXAS Sheet NAME/TITLE	79336
DRAWN BY	DATE 6/13/22
CHECKED BY BN Approved by	DATE 6/13/22 DATE
SHEET NO. REV. PN	6/13/22 scale NTS
REVISIONS ND. NDTES	BY DATE 00-00-00

Longitudinal bracing – to attach to underside of runway beam with (1) ³/₄" A325 bolt

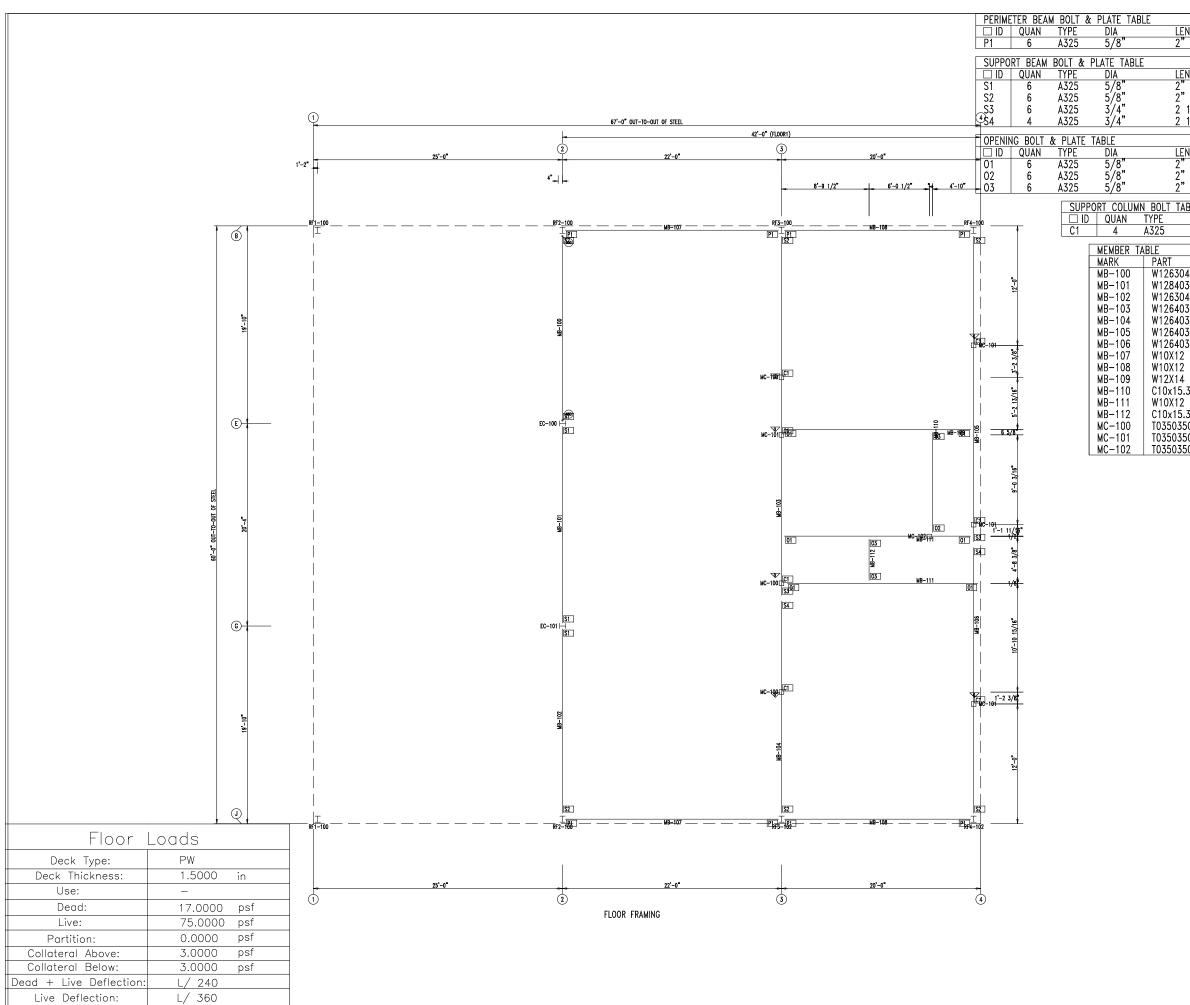


<u>JS TEEL COMPE</u> 432-75 430 US H SEMINOLE,	8-9900 WY 180 W
CUSTOMER INFO	
PREJECT PB50475 EVERLY DDESSA TEXAS Sheet NAME/TITLE	79336
DRAWN BY	DATE 6/13/22
CHECKED BY BN Approved by	DATE 6/13/22 DATE
SHEET NO. REV. PN	6/13/22 scale NTS
REVISIONS ND. NDTES	BY DATE 00-00-00

Longitudinal bracing – to attach to underside of runway beam with (1) ³/₄" A325 bolt

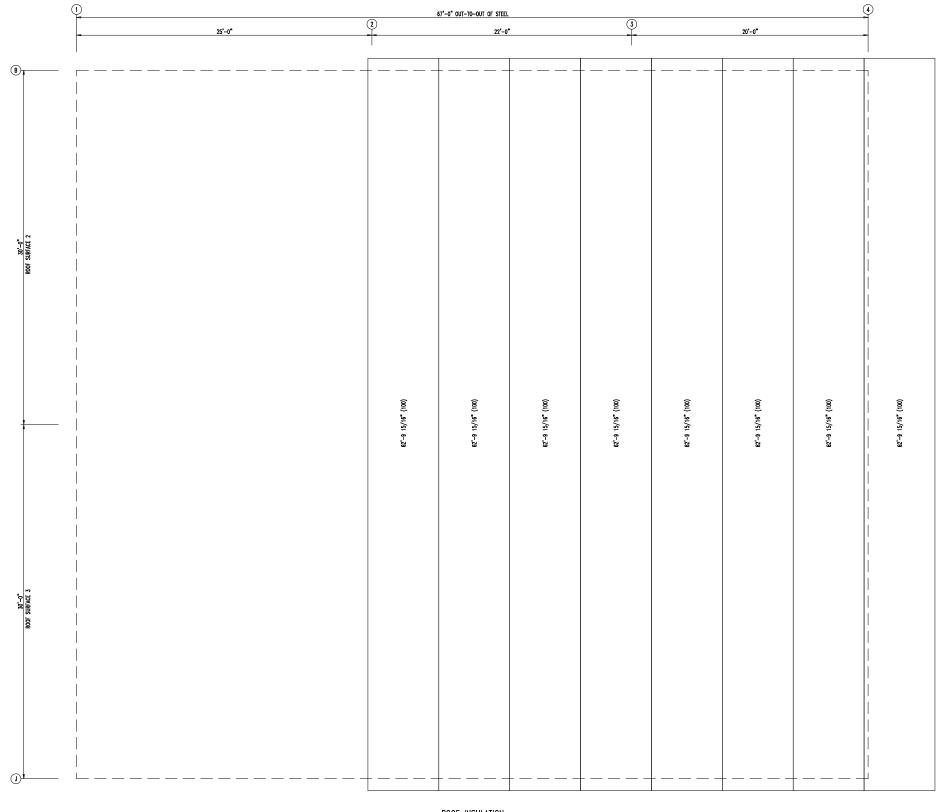


DIA 1/2" TABLE DIA 3/4" 035C 035C 035C 7212 7214	LENGTH 1 1/4" LENGTH 2 1/2" LENGTH 10'-6 1/2" 10'-6 1/2" 11'-6 1/2" 21'-5" 19'-1"		4: 430 (<u>N E N T</u> -9900 'Y 180) W
7212 7212 7212 7212 7212 7212 7212 7214 7214	19'-1" 19'-1" 21'-5" 19'-1" 21'-4" 21'-4" 14'-6 1/2" 8'-2" 8'-2" 19'-1"	- PROJ	OMER INFO			
			YLEE			
		SHEI	ESSA T et name/titli DDR JD	E	79336	5
		DRAW	N BY		DAT 7	е 7/25/23
		снесі 'BN	KED BY		dat 7	е 7/25/23
		1	OVED BY			/25/23
		E25	of 36	REV. } —	sca NT	
		REV NO.	ISIONS NOTES		BY	DATE
						00-00-00



LENGTH 2"	MARK/PART m3	BUILD IT WITH STEEL	BUILD IT TO LAST
LENGTH	MARK/PART	BUILD II .	
LENGTH 2" 2 1/2" 2 1/2"	m2 m1	STEEL COMPC	<u>DNENTS, INC.</u>
		432-758 430 US HV	
LENGTH 2" 2" 2"	MARK/PART m4 m6 m5	SEMINOLE	
TABLE DIA	LENGTH	CUSTOMER INFO	
3/4"	2 1/2"	-	
3040 4035 3040 4030 4030 4030 4030 12 12 12 14 15.3	LENGTH 18'-6 15/16" 20'-2 3/4" 18'-6 15/16" 36'-1 15/16" 21'-4 13/16" 30'-9 3/8" 26'-9 3/8" 21'-10 3/4" 19'-6 3/4" 19'-6 3/4" 19'-4 1/4"	PROJECT B HAYLEE DESSA TEXAS Sheet NAME/TITLE	79336
12 15.3 035C	4'-5 3/16" 10'-6 1/2"		DATE
035C 035C	10'-6 1/2" 11'-6 1/2"		7/25/23
		·BN	date 7/25/23
		APPROVED BY	date 7/25/23
		sheet no. rev. E26 of 30-	scale NTS
		REVISIONS NO. NOTES 	BY DATE 00-00-00

INSULAT ROOF P	TION TABL PLAN	E
ROLL	QUAN	MARK
100	8	RI-100

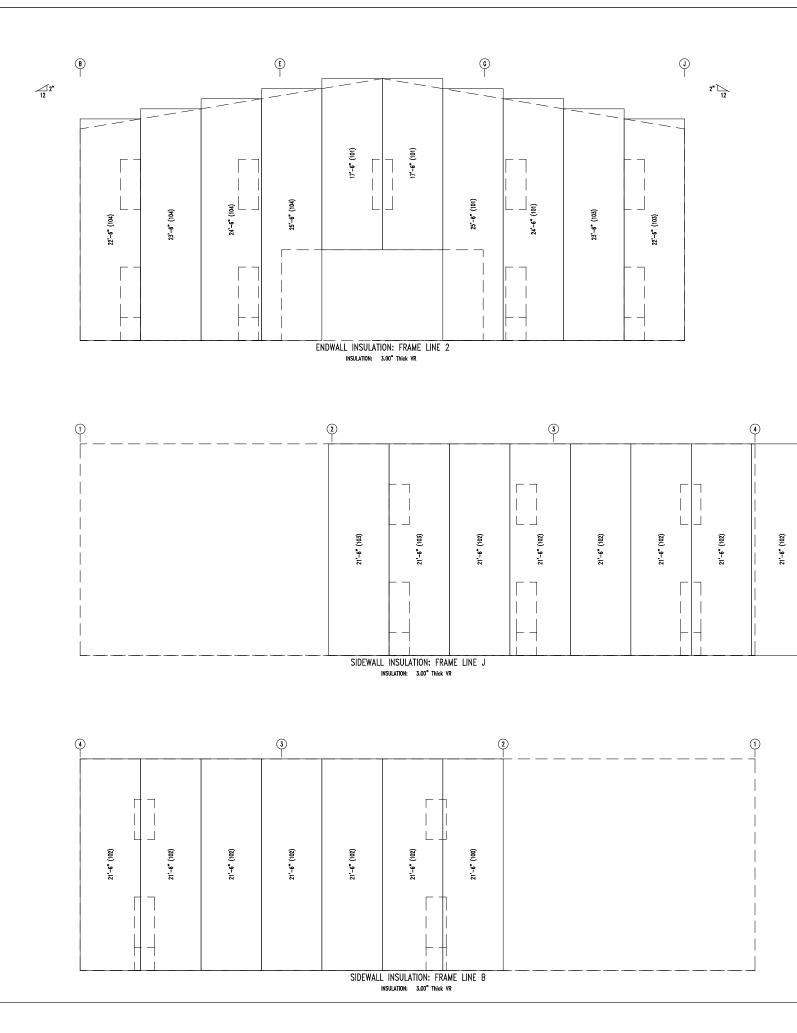


ROOF INSULATION Insulation: 3.00" thick vr

WIDTH	LENGTH
6'-0"	62'-9 15/16"

BUILDIT WITH STEEL B	NENTS, INC.
432-758- 430 US HW SEMINOLE,1	Y 180 W
CUSTOMER INFO	
project PB50586 HAYLEE	
DESSA TEXAS 7 sheet name/title RODF INSULATIO	
DRAWN BY	DATE 7/25/23
снескед ву ·BN	date 7/25/23
APPROVED BY	date 7/25/23
sheet no. rev. E27 of 30-	scale NTS
REVISIONS	
NO. NOTES	BY DATE 00-00-00
<u> </u>	

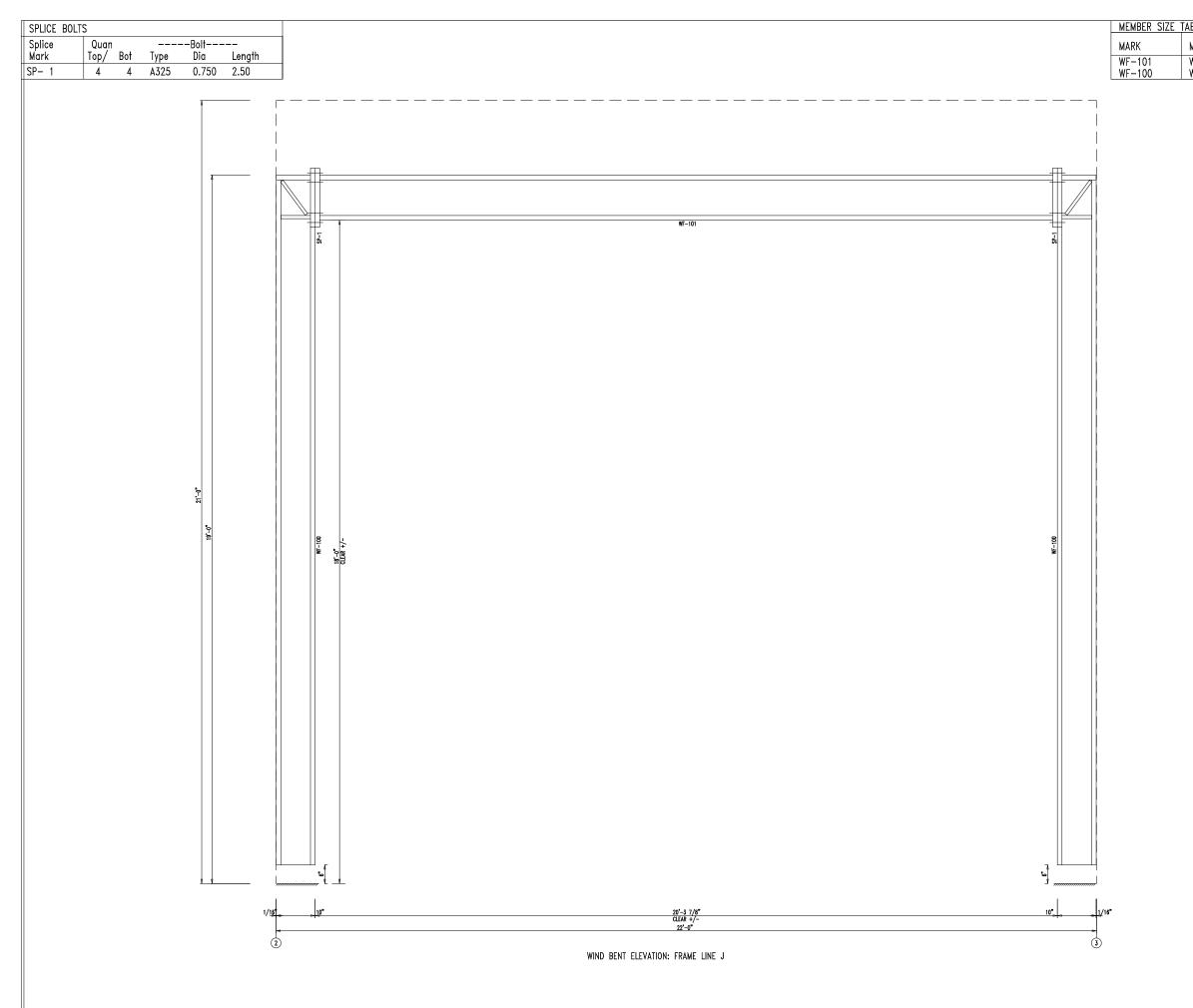
	TION TABL	
ROLL	line 1 J Quan	
100	1	WI-100
101	1	WI-101
102 103	3	WI-102 WI-103
103	1	WI-103 WI-104



WIDTH LENGTH 6'-0" 21'-6" 6'-0" 85'-0" 6'-0" 86'-0" 6'-0" 89'-0" 6'-0" 96'-0"		
6'-0" 85'-0" 6'-0" 86'-0" 6'-0" 89'-0"	WIDTH	
	6'-0" 6'-0" 6'-0"	85'-0" 86'-0" 89'-0"

LEGEND: 21'-6" (1) Length (Roll #)

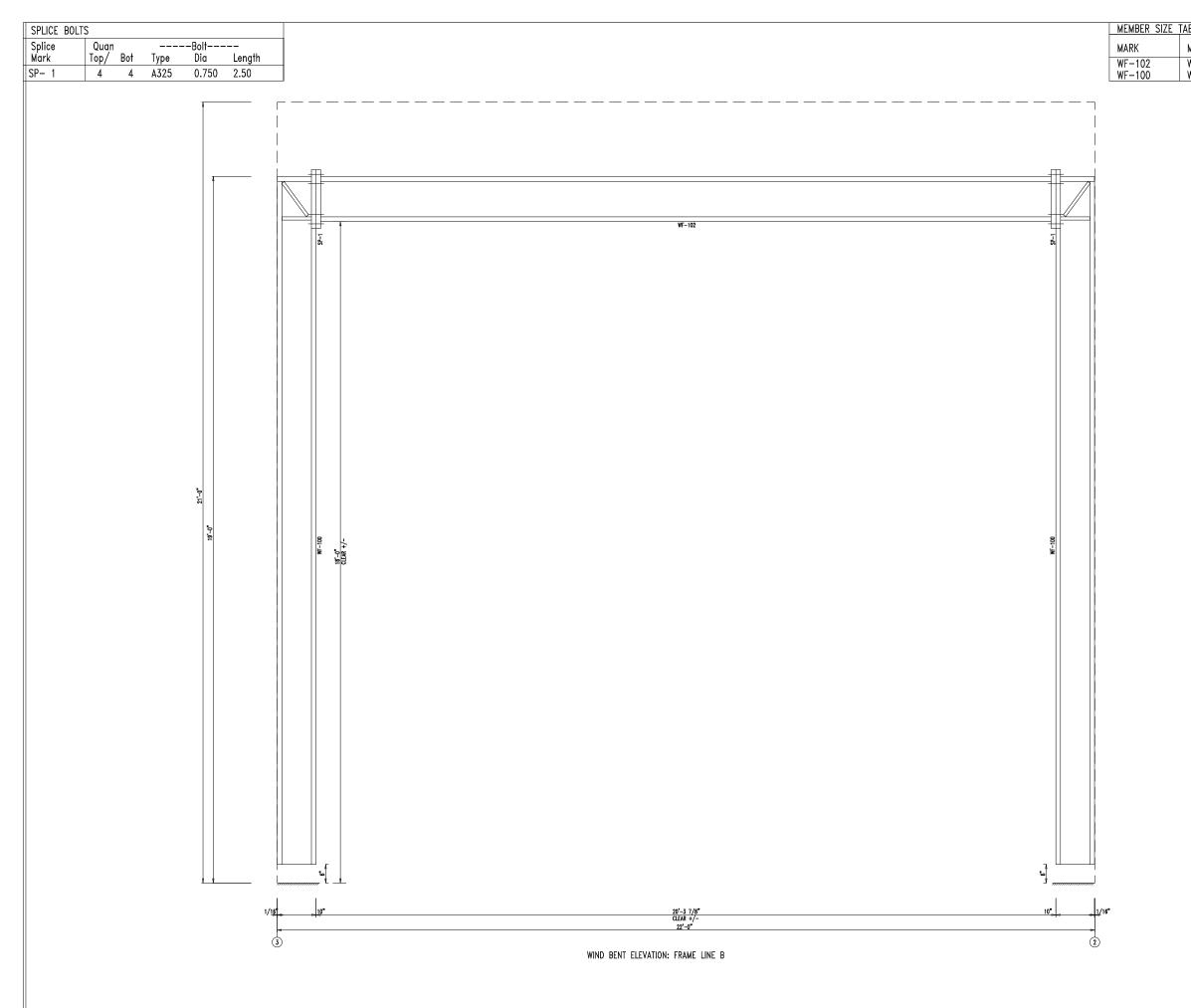
CHECKED BY DATE ·BN 7/25/2 APPROVED BY DATE · 7/25/2 SHEET NO. REV. SCALE SCALE E28 of 30 - NTS REVISIONS NO. NO. NOTES	PB50586 HAYLEE □DESSA TEXAS 79336 SHEET NAME/TITLE WALL INSULATION DRAWN BY DATE 7/25/2 CHECKED BY APPROVED BY		58-9900 HWY 180 W .E,TX 79360
PB50586 HAYLEE DESSA TEXAS 79336 SHEET NAME/TITLE WALL INSULATION DRAWN BY DATE 7/25/2 CHECKED BY -BN 7/25/2 APPROVED BY - SHEET NO. REV. SCALE E28 of 3θ - NTS REVISIONS NO. NOTES BY DATE	PB50586 HAYLEE □DESSA TEXAS 79336 SHEET NAME/TITLE WALL INSULATION DRAWN BY DATE 7/25/2 CHECKED BY APPROVED BY		
□DESSA TEXAS 79336 SHEET NAME/TITLE WALL INSULATION DRAWN BY DATE 7/25/2 CHECKED BY BN 7/25/2 APPROVED BY APPROVED BY CHEET NO. REV. SCALE E28 of 30 - NTS REVISIONS NO. NOTES BY DATE	DESSA TEXAS 79336 SHEET NAME/TITLE WALL INSULATION DRAWN BY DATE 7/25/2 CHECKED BY BN 7/25/2 APPROVED BY APPROVED BY CHECKED		
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SHEET NO. REV. SCALE E 28 of 30 - NTS REVISIONS NO. NOTES BY DATE	SHEET NO. REV. SCALE E 28 of 3θ - NTS REVISIONS NO. NOTES BY DATE		DATE
NO. NOTES BY DATE	NO. NOTES BY DATE		SCALE
			BY DATE
			00-00-0
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B	L	E	

MEMBER	LENGTH
W1264030 W1064030	20'-3_3/8" 18'-6"
W1004030	10-0

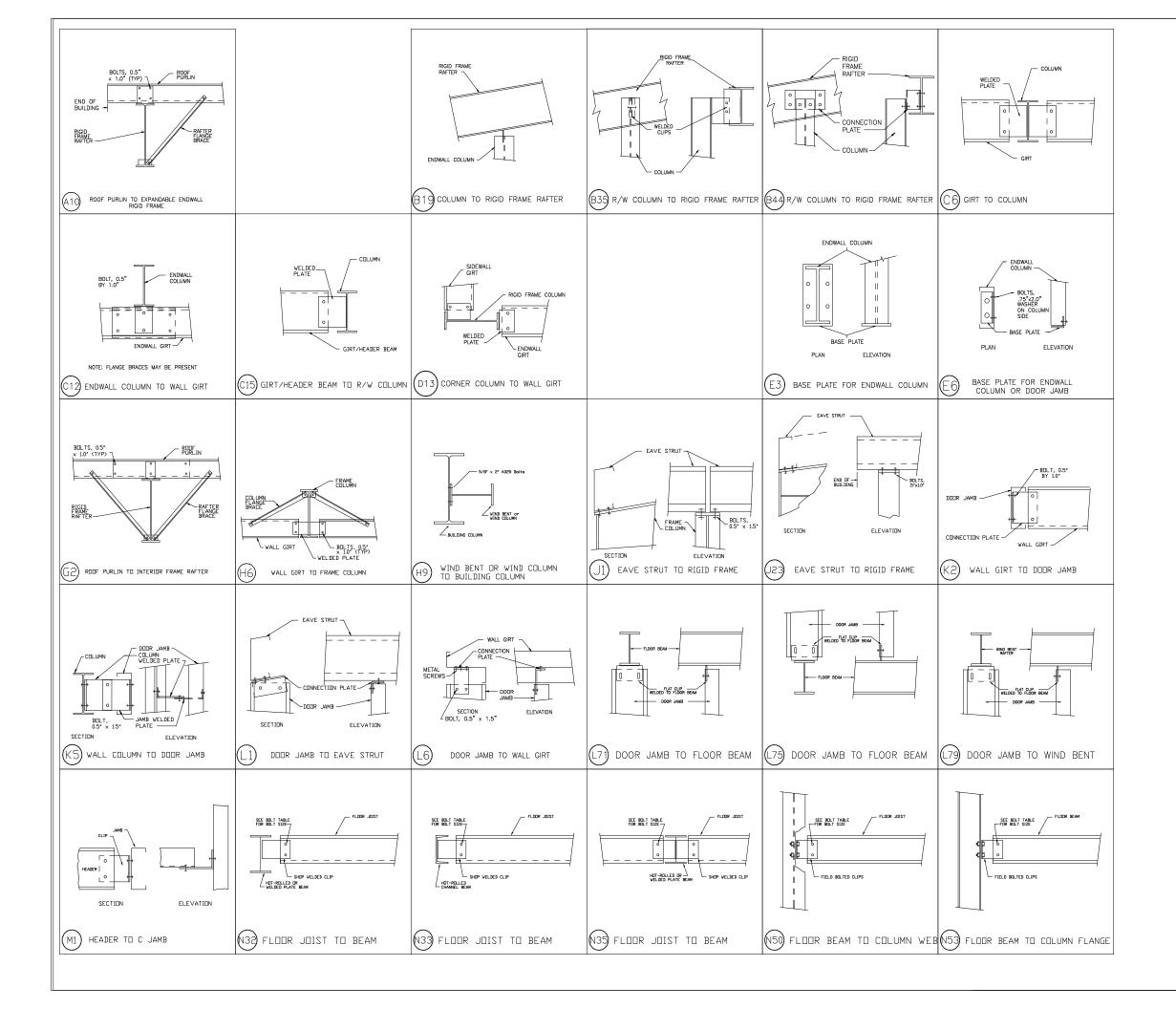
<u>STEEL COMPONENTS, INC.</u> <u>432-758-9900</u> 430 US HWY 180 W SEMINOLE, TX 79360		
PROJECT PB50586 HAYLEE DDESSA TEXAS SHEET NAME/TITLE	79336	
WIND BENT ELI DRAWN BY CHECKED BY	DATE 7/25/23	
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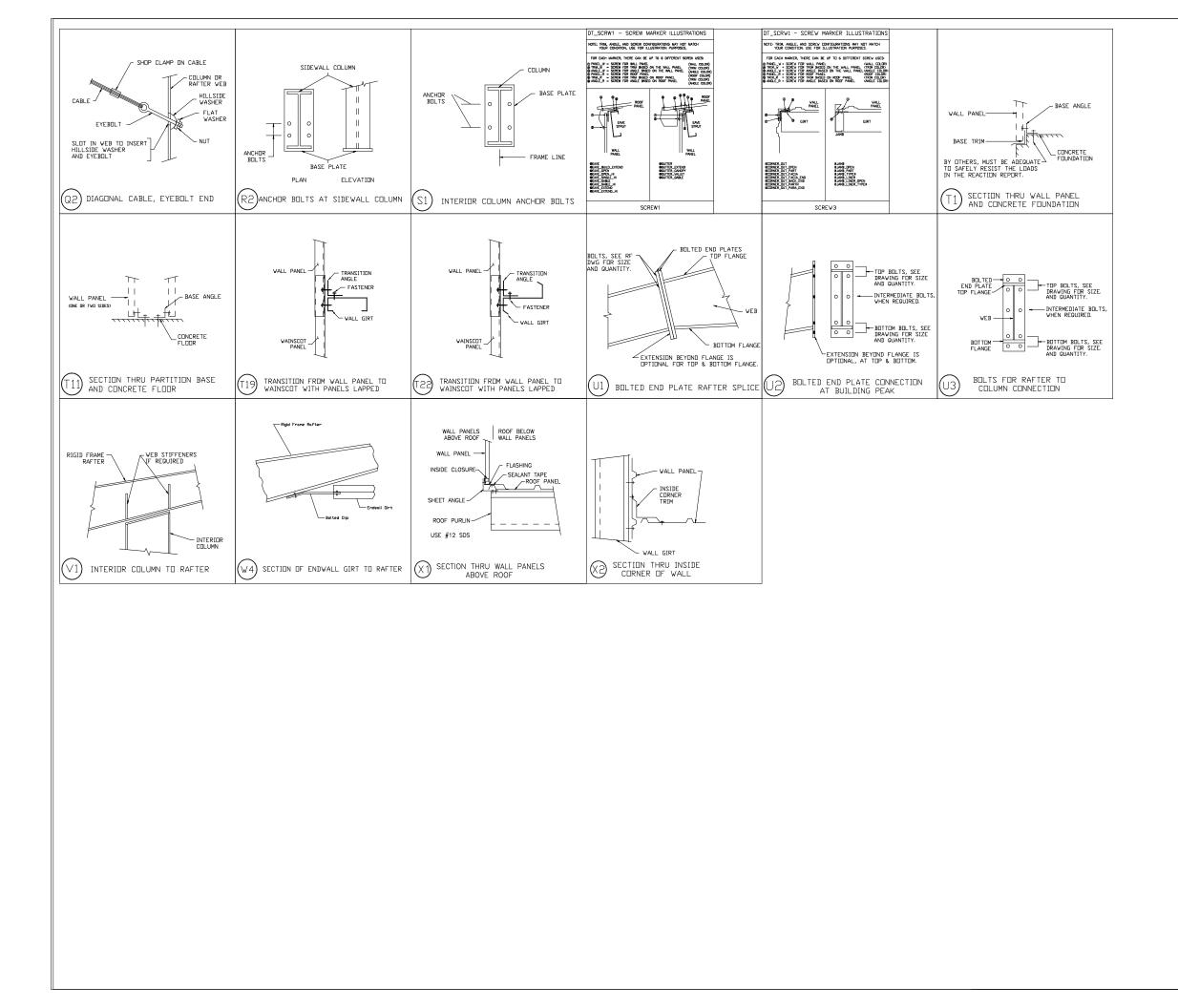
B	L	E

MEMBER	LENGTH
W1264030	20'-3_3/8"
W1064030	18'-6"

 BUILD IT WITH STEEL	BUILD IT TO LAST	
<u>]STEEL COMPONENTS, INC.</u>		
432-758-9900 430 US HWY 180 W		
SEMINOLE,TX 79360		
CUSTOMER INFO		
project PB50586		
HAYLEE		
DDESSA TEXAS	79336	
sheet name/title WIND BENT EL	EVATION	
DRAWN BY	date 7/25/23	
checked by 'BN	date 7/25/23	
APPROVED BY	DATE	
	7/25/23 scale	
E30 of 30-	NTS	
REVISIONS NO. NOTES	BY DATE	
	00-00-00	



	DILD IT TO LAST			
<u>STEEL COMPONENTS, INC.</u> 432-758-9900 430 US HWY 180 W SEMINOLE,TX 79360				
CUSTOMER INFO				
PROJECT PB50586 HAYLEE				
DDESSA TEXAS	79336			
SHEET NAME/TITLE DETAIL DRAWING DRAWN BY	DATE			
CHECKED BY	1/ 4/24 Date 1/ 4/24			
APPROVED BY	date 1/ 4/24			
SHEET NO. REV. PN	scale NTS			
REVISIONS NO. NOTES	BY DATE 00-00-00			



	DILD IT TO LAST			
<u>STEEL COMPONENTS, INC.</u> 432-758-9900 430 US HWY 180 W SEMINOLE,TX 79360				
CUSTOMER INFO				
PROJECT PB50586 HAYLEE				
DDESSA TEXAS	79336			
SHEET NAME/TITLE DETAIL DRAWING DRAWN BY	DATE			
CHECKED BY	1/ 4/24 Date 1/ 4/24			
APPROVED BY	date 1/ 4/24			
SHEET NO. REV. PN	scale NTS			
REVISIONS NO. NOTES	BY DATE 00-00-00			