

SOLID STEEL BUILDINGS, INC.

7630 MEADOW RUN LANE

OWINGS, MD 20736

FO# 27081

Building 1 of 1



Professional Certification; I hereby certify that these documents were prepared or approved by me, and that I am duly licensed professional engineer under the laws of the State of Maryland, License No. 30830, Expiration Date: 07/08/24



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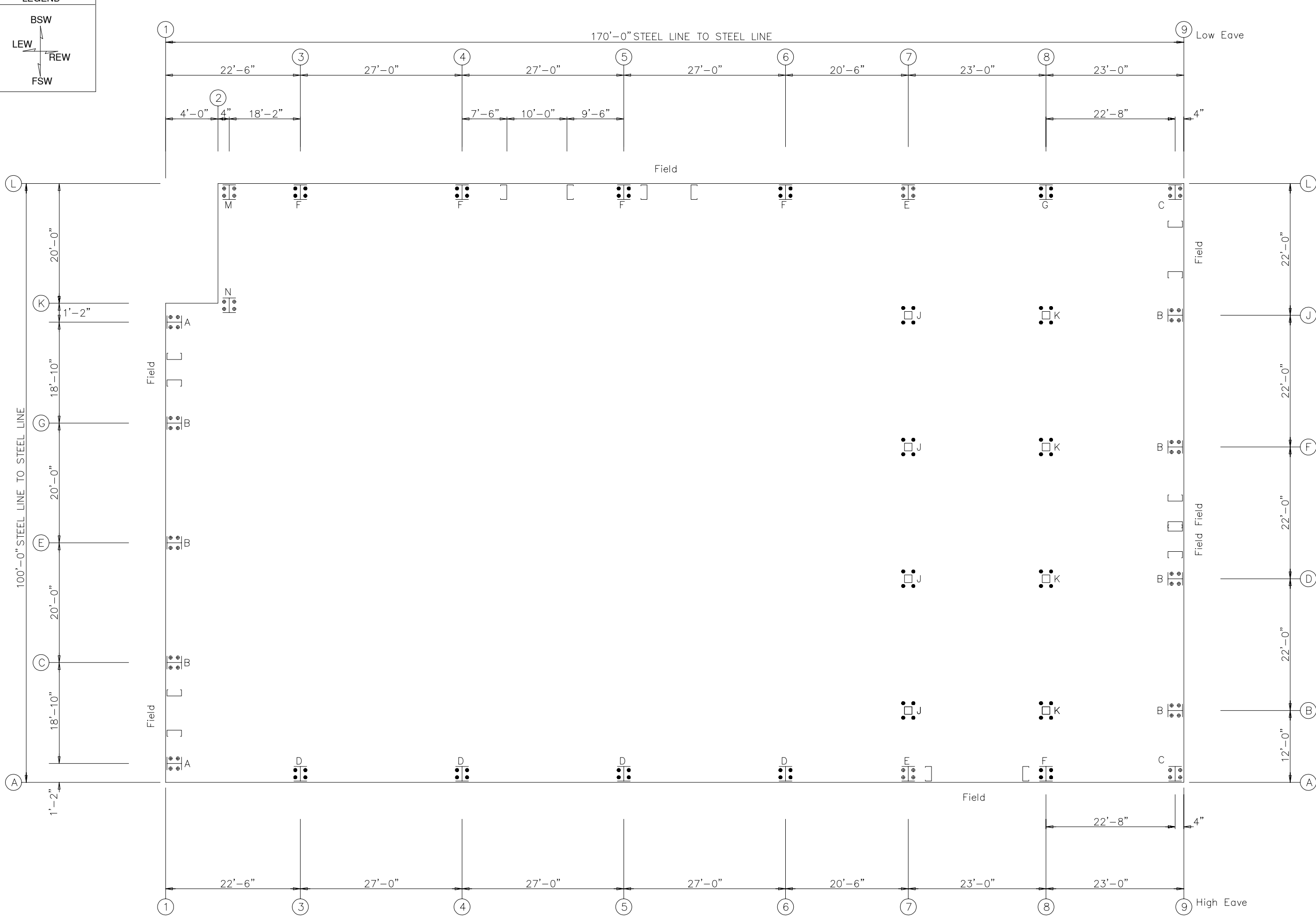
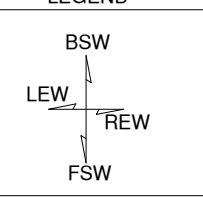
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GENERAL				MATERIALS	ASTM DESIGNATION	MINIMUM YIELD	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD
All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 2.1)				Hot-Rolled Mill Sections	A 36, A 572, A 992	Fy = 36 ksi and/or 50 ksi	Roof and Wall Sheeting	A 792, Gr. 50 Class 1 A 792, Gr. 80	Fy = 50 ksi Fy = 80 ksi
				Structural Steel Plates	A 572, A 1011	Fy = 55 ksi	Mild Steel Bolts	A 307	Fy = 36 ksi
				Structural Steel Bars	A 572 or A 529	Fy = 55 ksi	High Strength Bolts	F3125: A 325–N A 490–N	Fy = 92 or 81 ksi N/A
				Cold Formed Light Gauge Shapes	A 653 Gr. 55	Fy = 55 ksi	Anchor Rods (If supplied)	A 36	Fy = 36 ksi
				Cable Bracing	A 475, EHS	N/A	Pipe and Hollow Structural Sections	A 500 Gr. B	Fy = 42 ksi, 46 ksi
				Rod Bracing	A 36	Fy = 36 ksi			
				CORRECTION OF ERRORS AND REPAIRS					
The manufacturer is not responsible for the design, materials, and workmanship of the foundation. The anchor bolt plans prepared by the manufacturer are intended to show only the anchor bolt location, diameter (based on ASTM A36 bolts), and quantity required to connect the Metal Building System to the foundation. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.2). It is the responsibility of the end customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and / or associated items embedded in the concrete foundation, as well as foundation design based on the loads imposed by the Metal Building System, or other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.2) U.S. –Anchor bolts shall be accurately set to a tolerance of +/- 1/8 in both elevation and location (AISC Code of Standard Practice for Steel Buildings and Bridges). Canada –Anchor bolts shall be accurately set in accordance with CISC Code of Standard Practice, December 2015, Clause 7.8.1				The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping, and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.14; CISC Code of Standard Practice, December 2015, Clause 7.15; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.10).					
				DRAWING DISCREPANCIES					
The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.2.5)				In case of discrepancies between the manufacturers steel plans and plans for other trades, the manufacturers steel plans govern. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 3.3; CISC Code of Standard Practice, December 2015, Clause 3.4; MBMA 2018 Metal Building Systems Manual, Part IV, Section 3.1).					
				DELIVERIES					
All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC–SP2 (Steel Manual, Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud an other contamination of the primer film. Repairs of damage to primed surfaces and or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, December 2015, Clauses 6.8 & 6.9; (MBMA 2018 Metal Building Systems Manual, Part IV, Section 4.2.4).				Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am – 12pm (morning) and 12pm – 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.					
				SHORTAGES					
The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, December 2015, Clause 7.3; (MBMA 2018 Metal Building System Manual, Part IV, Section 6.9).				The purchaser /customer should make an inspection upon arrival of all building components. The purchaser/customer must note on the freight bill any missing item(s) and notify the manufacturers customer service department immediately; otherwise, the manufacturer cannot be held responsible for any shortages. If any item is damaged, note on the bill of lading and file a claim with the freight agent. Concealed shortages must be reported to the manufacturers customer service department within the following time frames (date from receipt of first delivery), based on the project shipment size, i.e., number of truck loads used in delivery.					
				1 to 3 loads....2 weeks 4 loads and over....3 weeks The manufacturers responsibility for shortages expires at the end of these time periods.					
The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (2019), and / or the Seam–Lok Technical – Erection manual (2019) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field–erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection.				FABRICATION ERRORS					
				The purchaser/customer is responsible for contacting the customer service department to advise the manufacturer of fabrication problems and corresponding cost estimates. The manufacturer will be responsible for providing the builder with verbal approval to proceed with appropriate field corrections. This will be done in a timely manner. IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT THE MANUFACTURERS APPROVAL, HE DOES SO AT HIS OWN RISK. The manufacturer shall not be responsible for any claims where the purchaser/customer has not documented the problem, its correction, and reasonable costs for repair, and submitted this documentation for payment within 30 days of the occurrence.					
The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, June 15, 2016, Section 7.10.3; CISC Code of Standard Practices, December 2015, Clause 1.5; MBMA 2018 Metal Buildings System Manual, Part IV, Section 6.2.1.5).				INVOICE PAYMENT					
				By acceptance of the materials of services set forth in the invoice, the purchaser/customer agrees to pay the invoice amount within the time period specified on the invoice. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE.					
U.S. ; Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1:500. (AISC Code of Standard Practice for Steel Buildings and Bridges June 15, 2016 Section 7.13.1; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8) Canada; Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. (CISC Handbook of Steel Construction, Eleventh Edition, Third Revised Printing, Part 1, Clause 29.3; MBMA 2018 Metal Building Systems Manual, Part IV, Section 6.8)				SAFETY PROCEDURES					
				The manufacturer is committed to manufacturing a quality product that can be erected safely. Although good job site practices and a commitment to safety by the erector are beyond the control of the manufacturer, the manufacturer highly recommends the erector provide good, safe working conditions on the job site. The erector should follow all local, state, and federal health and safety regulations at all times. Accident prevention practices should be implemented and each employee should know emergency procedures. The manufacturer also recommends daily meetings to discuss erection safety procedures. For additional information concerning federal health and safety regulations, contact the occupational safety and health administration (osha).					
BOLT TIGHTENING				U.S. Department of Labor Occupational Safety and Health Administration 200 Constitution Avenue, N.W. Washington, DC 20210 www.osha.gov					
				The manufacturer shall not be responsible for personal injury or property damage as a result of failure to follow all applicable safety regulations and material handling and installation recommendations.					

SOLID STEEL BUILDINGS	7630 MEADOW RUN LANE	100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"	DATE: 3/31/23	REVISION: 0	ENG: IRM DWN: BJC APPD: IRM	
F.O.27081						
7630 MEADOW RUN LANE	CITY: OWINGS	ST / PV: MD	REVISION HISTORY			FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.
			REV.	DESCRIPTION	DATE	
DRAWING STATUS	FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.					
	FOR CONSTRUCTION: FINAL DRAWINGS.					

STATE OF MARYLAND Y. JAMES EISENMAN, JR. PROFESSIONAL ENGINEER No. 30830	5/11/23
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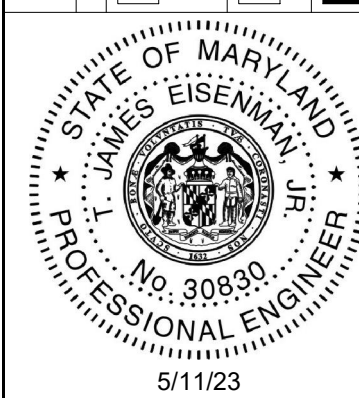
ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)
Finished Floor @ 100'-0"

DRAWING IS NOT TO SCALE

7630 MEADOW RUN LANE

F.O. 27081

DRAWING STATUS		CITY: OWINGS		ST / PV: MD	
REVISION HISTORY		REVISION HISTORY		REVISION HISTORY	
REV.	DESCRIPTION	REV.	DESCRIPTION	REV.	DESCRIPTION
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5/11/23

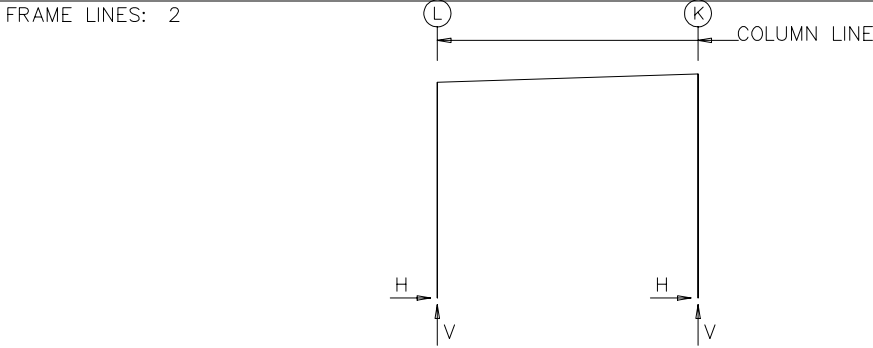
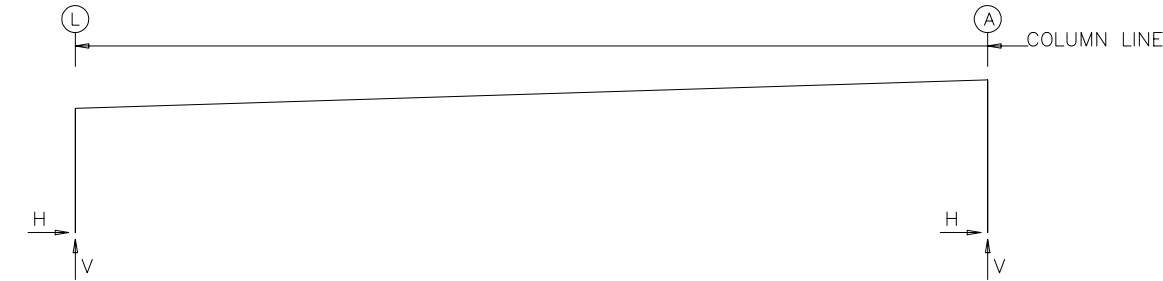
7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23 REVISION: 0

ENG: IRM DWN: BJC APPD: IRM

FRAME LINES: 3 4 5 6 7 8



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate Width	(in) Length	Thick	AFF/BFF (in)
3	L	4	1.000	8.000	11.81	0.625	0.0
3	A	4	1.000	8.000	11.75	0.625	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate Width	(in) Length	Thick	AFF/BFF (in)
4*	L	4	1.000	8.000	11.81	0.625	0.0
4*	A	4	1.000	8.000	11.75	0.625	0.0

4* Frame lines: 4 5

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Qty	Bolt Dia	Base_Plate Width	(in) Length	Thick	AFF/BFF (in)
6	L	4	1.000	8.000	11.81	0.500	0.0
6	A	4	1.000	8.000	11.75	0.500	0.0

FLOOR COLUMN REACTIONS

Frame Line	Col Line	Dead Vert (k)	Coll Vert (k)	Live Vert (k)	Anc. Qty	Bolt Dia	Base Width	Plate (in) Length	Thick	AFF/BFF (in)
7	J	17.4	1.8	44.2	4	1.000	10.00	10.00	1.000	0.0
7	F	17.9	1.8	45.4	4	1.000	10.00	10.00	1.000	0.0
7	D	17.9	1.8	45.4	4	1.000	10.00	10.00	1.000	0.0
7	B	11.9	1.2	30.5	4	1.000	10.00	10.00	1.000	0.0
8	J	23.4	2.4	60.0	4	1.000	12.00	12.00	1.000	0.0
8	F	24.1	2.5	61.6	4	1.000	12.00	12.00	1.000	0.0
8	D	24.1	2.5	61.6	4	1.000	12.00	12.00	1.000	0.0
8	B	15.0	1.5	38.6	4	1.000	12.00	12.00	1.000	0.0

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2	L	0.0	0.8	0.0	0.5	0.1	1.9	0.1	1.9	-3.5	-8.0	3.8	2.7
2	K	0.0	0.8	0.0	0.5	-0.1	2.0	-0.1	2.0	-3.6	2.2	3.6	-8.5
Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2	L	-4.4	-4.9	2.9	5.2	2.1	-2.5	1.1	0.0	-0.1	-0.2	0.1	0.2
2	K	-2.8	5.0	4.4	-5.0	-2.0	-2.5	-1.2	0.2	-0.1	0.2	0.1	-0.2
Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	MIN_SNOW Horiz	MIN_SNOW Vert	MIN_SNOW Horiz	MIN_SNOW Vert	MIN_SNOW Horiz	MIN_SNOW Vert	MIN_SNOW Horiz	MIN_SNOW Vert	MIN_SNOW Horiz	MIN_SNOW Vert
2	L	0.1	1.9	0.1	1.9	0.1	1.9	0.1	1.9	0.1	1.9	0.1	1.9
2	K	-0.1	2.0	-0.1	2.0	-0.1	2.0	-0.1	2.0	-0.1	2.0	-0.1	2.0
Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
3	L	3.1	6.0	5.3	10.1	15.7	24.7	15.7	24.7	-27.9	-36.4	-4.2	-18.4
3	A	-3.1	6.1	-5.3	10.1	-15.7	24.8	-15.7	24.8	14.0	-33.0	21.1	-23.4
Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
3	L	-23.1	-24.3	0.6	-6.2	-13.3	-35.5	-6.8	-24.9	-0.6	-0.3	0.6	0.3
3	A	9.2	-20.9	16.3	-11.3	13.8	-36.9	6.6	-25.9	-0.6	0.3	0.6	-0.3
Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert
3	L	0.0	-2.2	15.7	24.7	0.0	-3.6	15.7	24.7	0.0	-3.6	15.7	24.7
3	A	0.0	-3.6	-15.7	24.8	0.0	-3.6	-15.7	24.8	0.0	-3.6	-15.7	24.8
Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
4*	L	3.3	6.3	5.3	9.5	17.2	27.0	17.2	27.0	-25.4	-33.5	-4.4	-17.9
4*	A	-3.3	6.4	-5.3	9.5	-17.2	27.0	-17.2	27.0	12.9	-30.5	19.6	-22.5
Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
4*	L	-20.1	-20.3	0.8	-4.7	-14.6	-38.2	-7.5	-26.6	-0.7	-0.3	0.7	0.3
4*	A	7.6	-17.2	14.3	-9.3	15.2	-39.6	7.3	-27.6	-0.7	0.3	0.7	-0.3
Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert
4*	L	0.0	-2.2	17.2	26.9	0.0	-3.6	17.2	26.9	0.0	-3.6	17.2	26.9
4*	A	0.0	-3.6	-17.2	27.0	0.0	-3.6	-17.2	27.0	0.0	-3.6	-17.2	27.0
Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
6	L	3.0	5.9	5.0	9.5	15.1	23.7	15.1	23.7	-22.3	-29.5	-3.9	-15.8
6	A	-3.0	5.9	-5.0	9.5	-15.1	23.8	-15.1	23.8	11.3	-26.8	17.2	-19.8
Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
6	L	-17.6	-17.8	0.8	-4.1	-12.8	-28.0	-6.5	-17.8	-0.6	-0.3	0.6	0.3
6	A	6.7	-15.2	12.6	-8.2	13.3	-28.3	6.4	-17.7	-0.6	0.3	0.6	-0.3
Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert
6	L	0.0	-2.2	17.2	26.9	0.0	-3.6	17.2	26.9	0.0	-3.6	17.2	26.9
6	A	0.0	-3.6	-17.2	27.0	0.0	-3.6	-17.2	27.0	0.0	-3.6	-17.2	27.0
Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Floor Horiz	Floor Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert
7	L	2.7	5.4	3.3	5.4	13.1	21.7	0.2	0.1	13.1	21.7	-19.5	-27.0
7	A	-2.7	10.2	-3.3	5.9	-13.1	21.8	-0.2	10.9	-13.1	21.8	9.4	-24.5
Frame Line	Column Line	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert
7	L	-3.2	-14.5	-15.6	-16.3	0.7	-3.8	-10.9	-25.6	-5.5	-16.3	-2.9	-0.9
7	A	15.4	-18.1	5.5	-13.9	11.5	-7.5	11.4	-25.9	5.3	-16.3	-3.2	0.9
Frame Line	Column Line	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Right Horiz	Seismic_Right Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Right Horiz	Seismic_Right Vert
7	L	2.9	0.9	0.0	0.0	13.1	21.7	2.9	0.9	0.0	0.0	13.1	21.7
7	A	3.2	-0.9	0.0	-0.3	-13.2	21.8	3.2	-0.9	0.0	-0.3	-13.2	21.8
Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Floor Horiz	Floor Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert
8	L	3.4	16.9	3.6	6.9	14.3	23.0	1.3	28.8	14.3	23.0	-26.0	-34.5
8	A	-3.4	10.6	-3.6	6.2	-14.3	23.0	-1.3	11.4	-14.3	23.0	12.8	-31.3
Frame Line	Column Line	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert
8	L	-3.7	-17.0	-21.7	-23.2	0.7	-5.7	-12.1	-34.5	-6.1	-24.7	-6.5	-1.2
8	A	19.7	-21.8	8.5	-20.0	15.4	-10.6	12.5	-36.1	5.9	-25.9	-2.8	1.2
Frame Line	Column Line	Seismic_Right Horiz	Seismic_Right Vert	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Right Horiz	Seismic_Right Vert	MIN_SNOW Horiz	MIN_SNOW Vert	Seismic_Right Horiz	Seismic_Right Vert
8	L	6.5	1.2	0.0	-6.4	14.4	23.0	6.5	1.2	0.0	-6.4	14.4	23.0
8	A	2.8	-1.2	0.0	-4.5	-14.4	23.1	2.8	-1.2	0.0	-4.5	-14.4	23.1
4*	Frame lines:		4 5										



7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

DWN: BJC

APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

CITY: OWINGS	ST / PV: MD	REVISION HISTORY	DATE
DESCRIPTION	REV.		
DRAWING STATUS			
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FOR CONSTRUCTION: FINAL DRAWINGS.			



5/11/23

ENDWALL COLUMN:					BASIC COLUMN REACTIONS (k)									
Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	K	0.7	0.5	1.9	1.9	0.0	-3.1	0.0	-1.9	0.0	-2.1	0.0	-0.9	-3.2
1	G	1.2	1.3	5.1	5.1	-3.5	-13.3	0.0	2.0	-3.7	-11.0	0.0	4.3	-6.2
1	E	1.1	1.1	4.3	4.3	0.0	-2.5	5.0	-10.8	0.0	-0.2	4.8	-8.5	-6.4
1	C	1.2	1.3	5.1	5.1	0.0	-8.5	0.0	-4.9	0.0	-6.0	0.0	-2.4	-6.5
1	A	0.7	0.5	1.9	1.9	0.0	-3.1	0.0	-1.9	0.0	-2.2	0.0	-1.0	-3.5

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long2 Horz	Seis_Left Horz	Seis_Right Horz	-MIN_SNOW- Horz	E1PAT_SL_1- Horz
1	K	3.7	0.0	-3.2	0.0	-1.9	0.0	0.0
1	G	6.8	0.0	-7.6	0.0	-4.5	-0.6	-0.8
1	E	7.1	0.7	-8.1	0.3	-4.5	0.0	0.8
1	C	7.1	0.0	-8.5	0.0	-4.9	0.0	0.0
1	A	4.0	0.0	-3.1	0.0	-1.7	0.0	0.0

Frm Line	Col Line	E1PAT_SL_2- Horz	E1PAT_SL_3- Vert	E1PAT_SL_4- Horz	E1PAT_SL_5- Vert
1	K	0.0	0.0	0.0	0.9
1	G	0.0	0.1	0.0	2.7
1	E	0.0	-0.2	0.0	1.1
1	C	0.0	1.4	0.0	-0.1
1	A	0.0	1.0	0.0	0.0

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Floor Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert
9	A	0.9	0.2	0.8	0.0	0.8	0.0	-0.9	0.0	-1.3	0.0	-0.5	0.0	-1.0
9	B	8.5	1.8	4.4	18.2	4.4	0.0	-4.3	0.0	-7.4	0.0	-2.1	0.0	-5.3
9	D	14.3	2.7	5.3	33.2	5.3	0.0	-5.2	0.0	-9.0	0.0	-2.5	0.0	-6.4
9	F	14.2	2.6	4.9	33.2	4.9	-5.3	-11.1	0.0	-4.4	-5.1	-8.4	0.0	-1.7
9	J	14.0	2.7	5.7	32.3	5.7	0.0	0.9	3.4	-13.8	0.0	3.5	3.6	-11.2
9	L	8.6	1.3	2.1	18.9	2.1	0.0	-2.1	0.0	-3.5	0.0	-1.0	0.0	-2.4

Frm Line	Col Line	Wind_Press Horz	Wind_Suct Vert	Wind_Long1 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Right Horz	Seis_Long Horz
9	A	-11.2	-8.7	4.5	8.7	0.0	-1.3	0.0
9	B	-5.5	0.0	6.1	0.0	0.0	-7.4	0.0
9	D	-7.0	0.0	7.8	0.0	0.0	-9.0	0.0
9	F	-6.9	0.0	7.6	0.0	-0.9	-9.4	-0.4
9	J	-6.6	0.0	7.3	0.0	0.0	-8.6	0.0
9	L	-10.5	-7.4	4.0	7.4	0.0	-3.6	0.0

Frm Line	Col Line	-MIN_SNOW- Horz	E2PAT_SL_1- Vert	E2PAT_SL_2- Horz	E2PAT_SL_3- Vert	E2PAT_SL_4- Horz	E2PAT_SL_5- Vert	E2PAT_SL_6- Horz
9	A	0.0	0.8	0.0	0.7	0.0	0.0	0.4
9	B	0.0	4.4	0.0	0.8	0.0	0.0	2.4
9	D	0.0	5.3	0.0	-0.1	0.0	0.1	1.3
9	F	0.0	4.9	0.0	0.0	-0.3	0.0	-0.2
9	J	0.0	5.7	0.0	0.0	0.0	1.6	0.0
9	L	0.0	2.1	0.0	0.0	1.2	0.0	0.0

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Type
⊕	44	Endwall	3/4"	
●	40	Frame	1"	
⊕	16	Frame	3/4"	
●	32	Floor	1"	

BUILDING BRACING REACTIONS

Wall		Col		Reactions in plane of wall ± Reactions(k)				Panel Shear (lb/ft)
Loc	Line	Line	Line	Wind Horz	Seismic Vert	Wind Vert	Seis	
L_EW	1	G,E		Bracing, see EW reactions				
F_SW	A	3,4		7.4	*	3.6	*	
		8,9		7.4	*	4.1	*	
R_EW	9	F,J		Bracing, see EW reactions				
B_SW	L	9,8		7.1	*	10.6	*	
		4,3		7.1	*	2.5	*	

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc._Bolt Qty	Dia	Base_Plate Width	(in) Length	Thick	AFF/BFF (in)
1	K	4	0.750	6.000	12.00	0.375	0.0
1	G	4	0.750	6.000	12.00	0.375	0.0
1	E	4	0.750	6.000	12.00	0.375	0.0
1	C	4	0.750	6.000	12.00	0.375	0.0
1	A	4	0.750	6.000	12.00	0.375	0.0
9	A	4	0.750	8.000	12.25	0.375	0.0
9	B	4	0.750	6.000	12.00	0.375	0.0
9	D	4	0.750	6.000	12.00	0.375	0.0
9	F	4	0.750	6.000	12.00	0.375	0.0
9	J	4	0.750	6.000	12.00	0.375	0.0
9	L	4	0.750	8.000	12.25	0.375	0.0

DESIGN INFORMATION

- All loading conditions are examined and only the maximum / minimum H or V and the corresponding H or V are reported.
- Positive reactions are shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:

DESIGN CRITERIA		SEISMIC CRITERIA		DEFLECTION LIMITS
Width (ft)	=100	Seismic Importance	= 1.00	ENDWALL COLUMN
Length (ft)	=170	Risk Category	=II - Normal	L / 120
Eave Height (ft)	=26.38			ENDWALL RAFTER (Liv
Roof Slope (rise/12)	=0.375:12			L / 180
Building Code	=IBC 18	Mapped Spectral Response Accelerations		ENDWALL RAFTER (Win
Local Code (State/Prov)	=IBC 18	Ss	=0.1230	L / 180
Dead Load (psf)	=2.250	S1	=0.0420	WALL GIRTS
Collateral Load (psf)	=5.00	----Spectral Response Coefficients----		L / 90
Roof Live Load (psf)	=20.00	Sds	=0.1312	PURLIN (LIVE)
Frame Live Load (psf)	=20.00	Sd1	=0.0672	L / 180
		Site Class	=d	PURLIN (WIND)
		Seismic Design Category	=B	L / 150
Snow:				WALL PANEL
Ground Snow Load (psf)	=25.00	-----Base Shear-----		L / 90
Snow Importance	=1.00	Expanded Formula = 0.667*Ie*Fa*Ss*W/R		ROOF PANEL (Live)
Thermal Coefficient	=1.00	Longitudinal Base Shear (k)	=29.33	L / 180
Snow Exposure Factor	=1.0000	Transverse Base Shear (k)	= 8.75	ROOF PANEL (Wind)
Slippery Roof	=N			L / 120
Roof Snow Load, Pf (psf)	=20			Main Frame (Horiz)
Min Roof Snow Load, Pmin (psf)	=20.0000	--Seismic Response Coefficients--		H / 60
Wind:		Frame	=0.044	Main Frame (Vert)
Ultimate Wind Speed (mph)	=115 mph	FSW	=0.044	L / 180
Risk Category	=II - Normal	BSW	=0.044	WIND BRACING
Importance - Wind	= 1.00			H / 60
Wind Exposure	=C			Main Frame (Crane)
Enclosure Classification	=C			H / 100
				Main Frame (Seismi
---Internal Pressure Coefficients---		---Response Modification Factors---		H / 50
Pressure	=0.18	Frame	=3	SEISMIC BRACING
Suction	=-0.18	FSW	=3	H / 50
		BSW	=3	PARTITION COLUMN
-----Components & Cladding-----				L / 120
Design Pressure:				PARTITION GIRT
Pressure (psf)	=29.41			L / 120
Suction (psf)	=-39.22			PARTITION PANEL
				L / 120

Equivalent Lateral Brace Force Procedure.

Steel systems not specifically detailed for seismic resistance.



7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

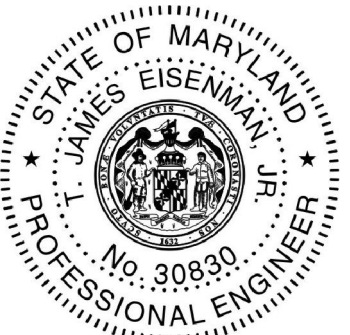
DWN: BJC

APPD: IRM

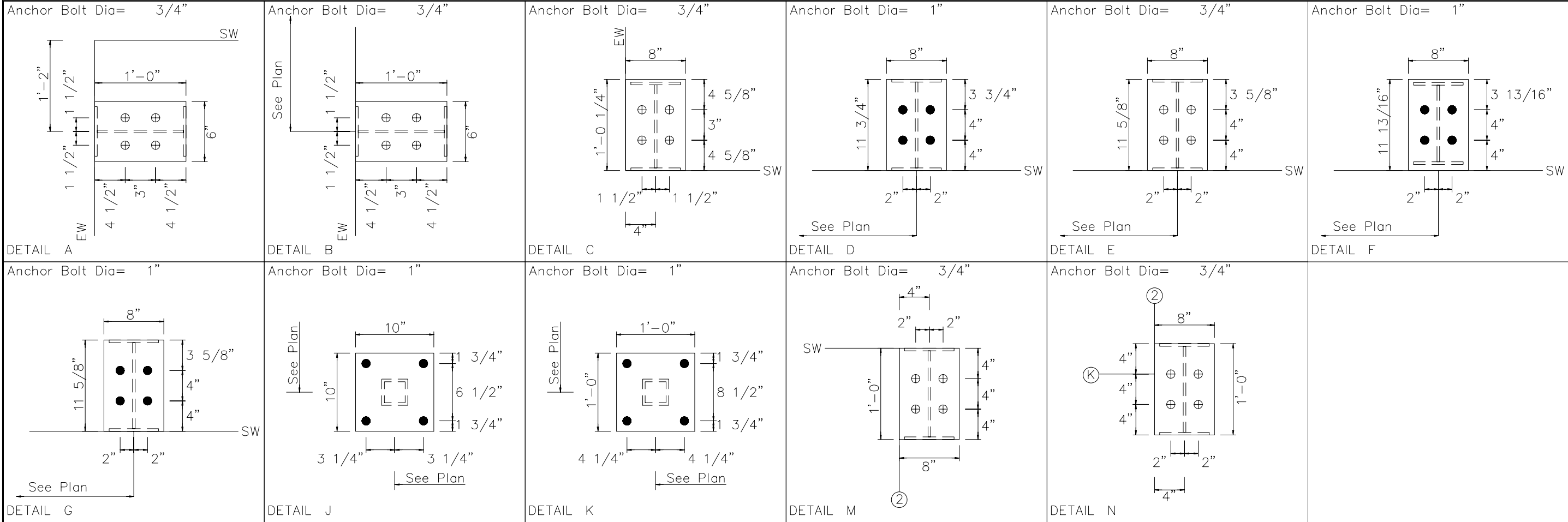
F.O. 27081

7630 MEADOW RUN LANE

DRAWING STATUS		CITY: OWINGS		ST / PV: MD
REVISION HISTORY		REVISION HISTORY		DATE
DESCRIPTION		DESCRIPTION		DATE
REV.		REV.		DATE
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STATE OF MARYLAND

JAMES EISENMAN, JR.

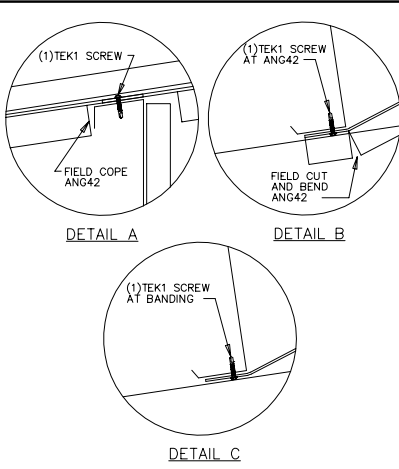
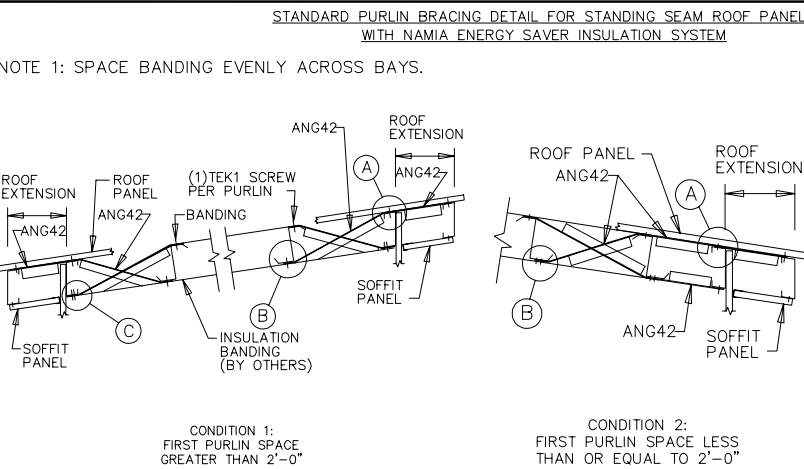
PROFESSIONAL ENGINEER

No. 30830

5/11/23

Additional Loading Information			
Mezzanine Loads		Crane Information and Loading	
<u>Mezzanine #1 Loads:</u> Dead Load <u>46.25</u> PSF Collateral Load <u>5.00</u> PSF Live Load <u>125.00</u> PSF <u>Mezzanine #2 Loads:</u> Dead Load <u>46.25</u> PSF Collateral Load <u>5.00</u> PSF Live Load <u>125.00</u> PSF <u>Mezzanine #3 Loads:</u> Dead Load <u>NA</u> PSF Collateral Load <u>NA</u> PSF Live Load <u>NA</u> PSF		<u>Crane #1 Information:</u> Crane Type <u>NA</u> CMAA Service Class <u>NA</u> Crane capacity = <u>NA</u> Tons Bridge Weight = <u>NA</u> Tons Hoist/Trolley Weight = <u>NA</u> Tons Wheel Spacing = <u>NA</u> Ft. <u>Crane #2 Information:</u> Crane Type <u>NA</u> CMAA Service Class <u>NA</u> Crane capacity = <u>NA</u> Tons Bridge Weight = <u>NA</u> Tons Hoist/Trolley Weight = <u>NA</u> Tons Wheel Spacing = <u>NA</u> Ft. <u>Crane #3 Information:</u> Crane Type <u>NA</u> CMAA Service Class <u>NA</u> Crane capacity = <u>NA</u> Tons Bridge Weight = <u>NA</u> Tons Hoist/Trolley Weight = <u>NA</u> Tons Wheel Spacing = <u>NA</u> Ft. <u>Crane #4 Information:</u> Crane Type <u>NA</u> CMAA Service Class <u>NA</u> Crane capacity = <u>NA</u> Tons Bridge Weight = <u>NA</u> Tons Hoist/Trolley Weight = <u>NA</u> Tons Wheel Spacing = <u>NA</u> Ft.	
Roof Opening Information and Loading			
<u>Roof Opening #1 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons <u>Roof Opening #2 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons		<u>Roof Opening #3 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons <u>Roof Opening #4 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons <u>Roof Opening #5 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons <u>Roof Opening #6 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons	
<u>Roof Opening #7 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons <u>Roof Opening #8 Information:</u> Width <u>NA</u> Ft Length <u>NA</u> Ft Collat <u>NA</u> Tons		<u>Miscellaneous Additional Loads</u> 1. <u>(4) B-ball Hoops @ 2500# Each.</u> 2. _____ 3. _____ 4. _____ 5. _____ 6. _____	

DRAWING IS NOT TO SCALE



BOLT TABLE			
ROOF PLAN			
LOCATION	QUAN	TYPE	DIA
SB/RF	2	A325	1/2"

MEMBER TABLE			
ROOF PLAN			
QUAN	MARK	PART	LENGTH
2	RO-1	08X35C16	4'-0 9/16"
2	RO-2	08X35C16	4'-0 9/16"
8	RO-3	08X35C16	4'-0 9/16"
1	P-1	08X25Z13	25'-7 1/2"
2	P-2	08X25Z13	25'-7 1/2"
14	P-3	08X25Z13	25'-7 1/2"
63	P-4	08X25Z14	33'-3 1/2"
21	P-5	08X25Z16	26'-9 1/2"
21	P-6	08X25Z14	29'-3 1/2"
2	P-7	08X25Z13	26'-1 1/2"
4	P-8	08X25Z13	26'-1 1/2"
15	P-9	08X25Z13	26'-1 1/2"
1	P-10	08X25Z16	16'-0"
2	P-11	08X25Z16	16'-0"
2	P-12	08X25Z16	6'-0"
4	P-13	08X25Z16	6'-0"
1	P-14	08X25Z13	21'-7 1/2"
2	P-15	08X25Z13	21'-7 1/2"
1	P-16	08X25Z13	21'-7 1/2"
2	P-17	08X25Z16	6'-0"
1	P-18	08X25Z16	16'-0"
1	E-1	08X35E10	22'-5 1/2"
3	E-2	08X35E10	26'-11 1/2"
1	E-3	08X35E10	20'-5 1/2"
1	E-4	08X35E10	22'-11 1/2"
1	E-5	08X35E10	22'-11 1/2"
1	E-6	08X35E10	22'-11 1/2"
1	E-7	08X35E10	22'-11 1/2"
1	E-8	08X35E10	20'-5 1/2"
3	E-9	08X35E10	26'-11 1/2"
1	E-10	08X35E10	18'-5 1/2"
1	E-11	08X35E10	4'-6 3/4"
4	CB-16	CABLE375	30'-0 3/16"
4	CB-17	CABLE250	32'-11 1/16"
2	CB-18	CABLE250	30'-3 3/16"
2	CB-19	CABLE375	26'-5 3/8"
1	CB-20	CABLE375	26'-5 9/16"
4	CB-21	CABLE250	29'-8"
2	CB-22	CABLE250	26'-8 1/2"
1	CB-23	CABLE375	26'-5 1/2"
4	SB-1	W8X24	26'-11 5/16"
4	SB-2	W8X24	26'-11 1/4"

SOLIDSTEEL BUILDINGS

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

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F.O. 27081

DRAWING STATUS

CITY: OWINGS

ST / PV: MD

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FINAL DRAWINGS.

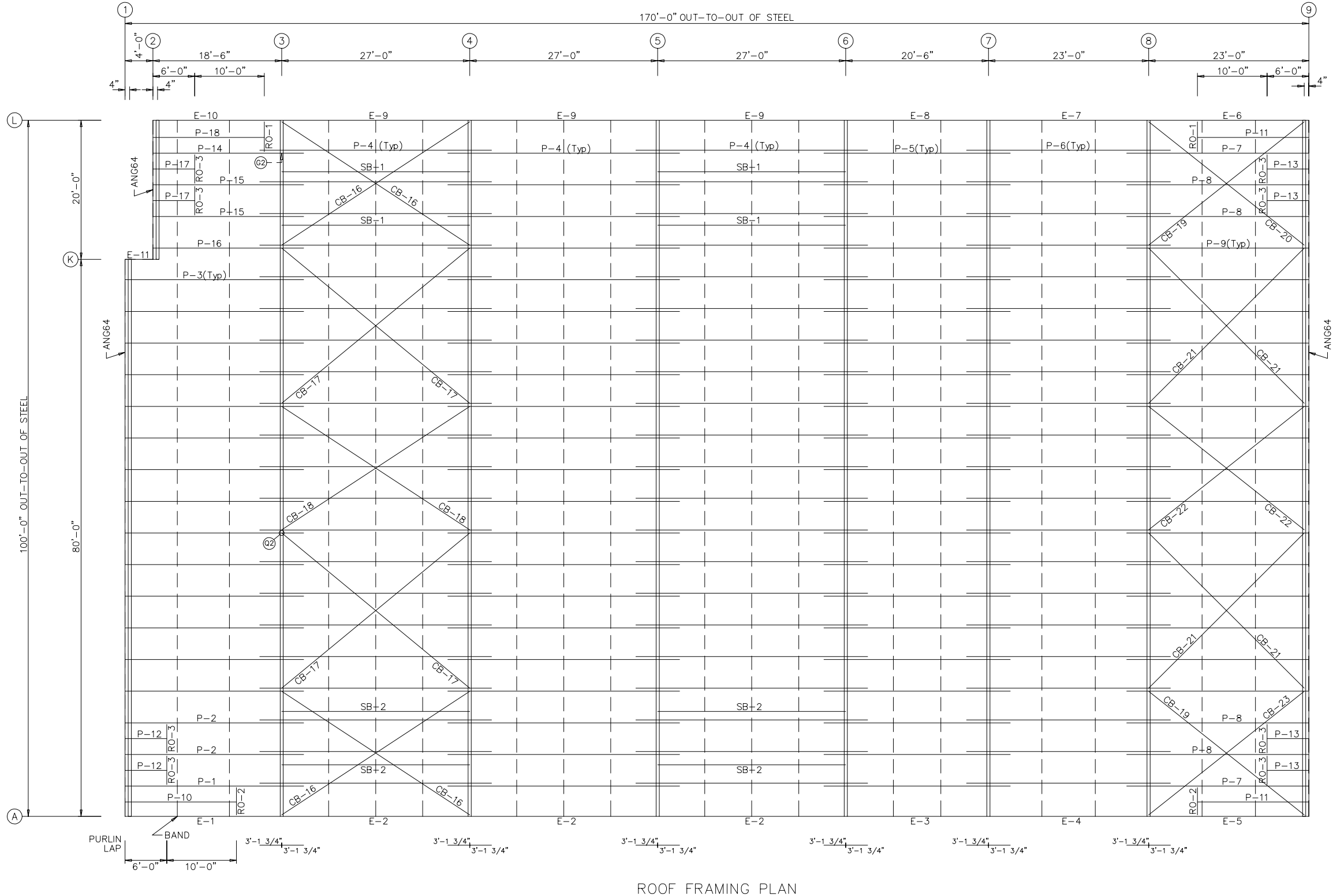
STATE OF MARYLAND

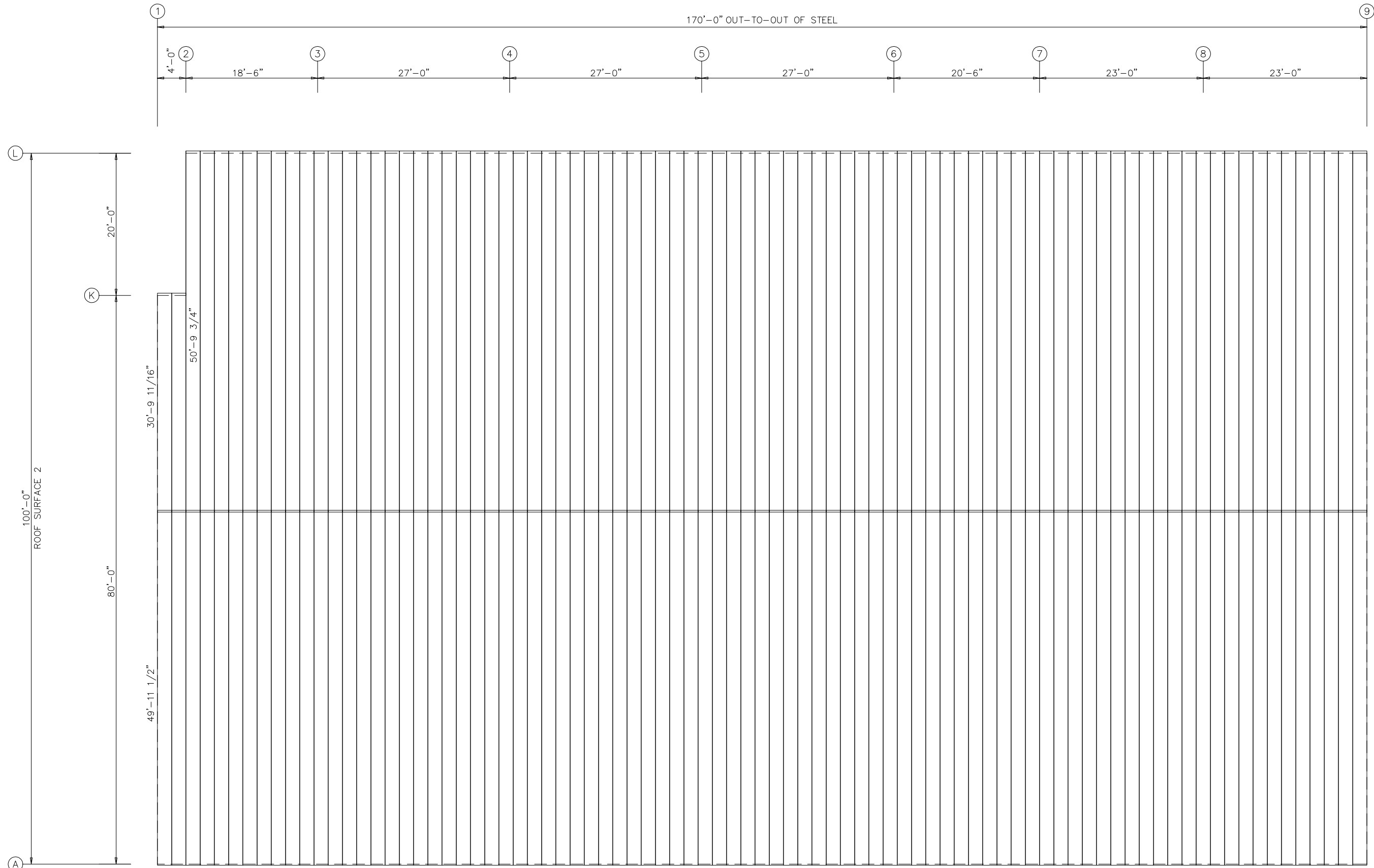
T. JAMES EISENMAN, JR.

PROFESSIONAL ENGINEER

No. 30830

5/11/23





ROOF SHEETING PLAN

PANELS: 24 Ga. L4 - Galvalume

GENERAL NOTES:

Panel "Start" and "End" dimensions must be followed for the proper installation of the gable trim(s) provided.

DRAWING IS NOT TO SCALE

STATE OF MARYLAND
T. JAMES EISENMAN, JR.
PROFESSIONAL ENGINEER
No. 30830
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7630 MEADOW RUN LANE

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7630 MEADOW RUN LANE

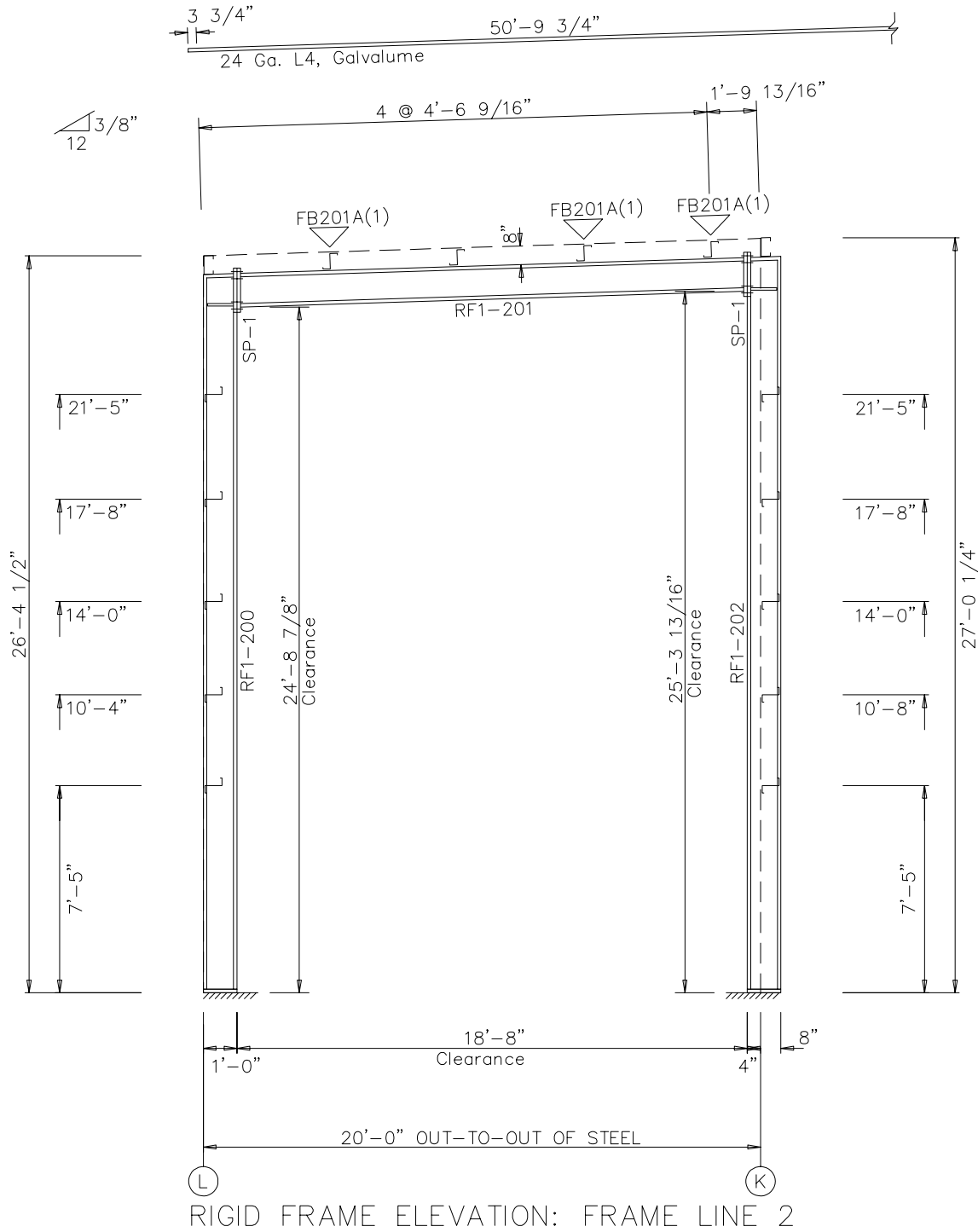
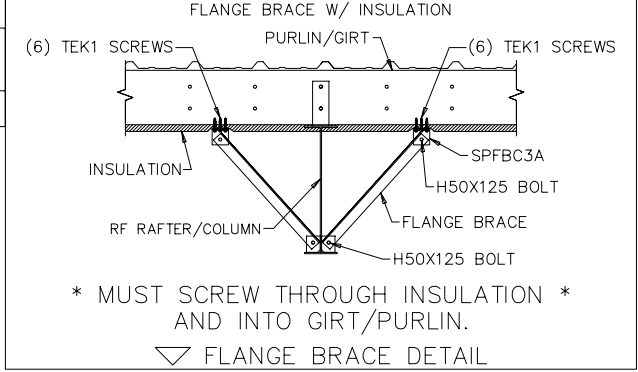
100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23	REVISION: 0
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ENG: IRM	DWN: BJC	APPD: IRM
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SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
SP-1	4	4	0	A325	0.500	2.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1)
A - L15X1/8



GENERAL NOTES:

- 1. See Detail Sheets for Connection Information.
- 2. See Shipping List for Flange Brace Lengths.

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-200	W12X16	25'-8 1/2"
RF1-201	W12X16	18'-7 1/8"
RF1-202	W12X16	26'-4 1/4"



7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/29/23 REVISION: 0

ENG: IRM DWN: BJC APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

CITY: OWINGS

ST / PV: MD

DRAWING STATUS

REVISION HISTORY

REV.

DESCRIPTION

DATE

FOR APPROVAL:
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FOR PERMIT:
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FOR CONSTRUCTION:
FINAL DRAWINGS.

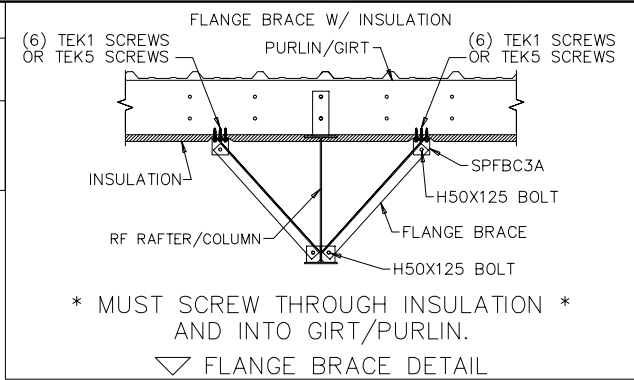


5/11/23

SPLICE BOLT TABLE

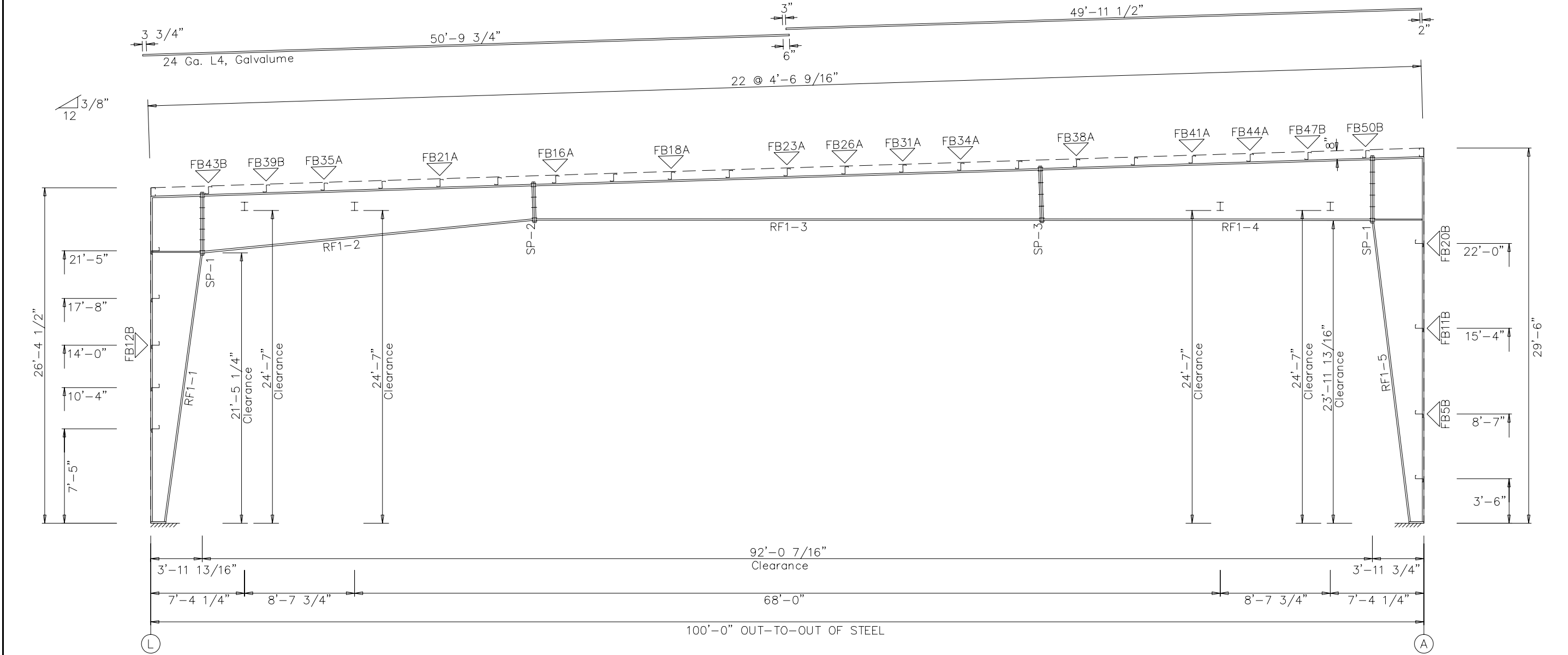
Mark	Qty	Top	Bot	Int	Type	Dia	Length
SP-1	4	4	8	A325	1.000	2.75	
SP-2	4	4	4	A325	0.750	2.25	
SP-3	4	4	6	A325	0.750	2.00	

▽FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
B - L20X3/16
A - L15X1/8



MEMBER TABLE

Mark	Web Depth		Web Plate		Outside Flange			Inside Flange		
	Start	End	Thick	Length	W	Thk	x Length	W	Thk	x Length
RF1-1	11.0	36.6	0.149	180.0	8 x 1/4"		x 120.0	8 x 1/2"		x 255.5
		36.6/47.0	0.313	129.1	8 x 5/16"		x 60.0			
					8 x 1/4"		x 127.6			
RF1-2		52.0/39.1	0.219	193.0	6 x 1/4"		x 47.3			
		39.1/31.0	0.188	120.0	6 x 1/4"		x 191.4	6 x 1/2"		x 73.2
					6 x 5/16"		x 120.0	6 x 1/4"		x 240.5
RF1-3		31.0/38.6	0.188	240.0	6 x 1/2"		x 240.0	6 x 1/4"		x 240.1
		38.6/46.0	0.219	236.9	6 x 3/8"		x 120.0	6 x 5/16"		x 120.1
					6 x 5/16"		x 116.9	6 x 1/4"		x 116.9
RF1-4		46.0/49.9	0.219	120.0	6 x 1/4"		x 312.5	6 x 1/4"		x 240.1
		49.9/56.0	0.250	192.5				6 x 3/8"		x 70.8
								8 x 1/2"		x 285.8
RF1-5		47.0/41.5	0.250	105.1	6 x 1/4"		x 47.3			
		41.5/11.0	0.188	240.0	8 x 1/4"		x 345.1			



RIGID FRAME ELEVATION: FRAME LINE 3

- GENERAL NOTES:
1. See Detail Sheets for Connection Information.

2. See Shipping List for Flange Brace Lengths.

DRAWING IS NOT TO SCALE

SOLID

STEEL BUILDINGS

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

DWN: BJC

APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

CITY: OWINGS

ST / PV: MD

DRAWING STATUS

REVISION HISTORY

DATE

DESCRIPTION

REV.

DATE

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FOR CONSTRUCTION:
FINAL DRAWINGS.

STATE OF MARYLAND

T. JAMES EISENMAN, JR.

PROFESSIONAL ENGINEER

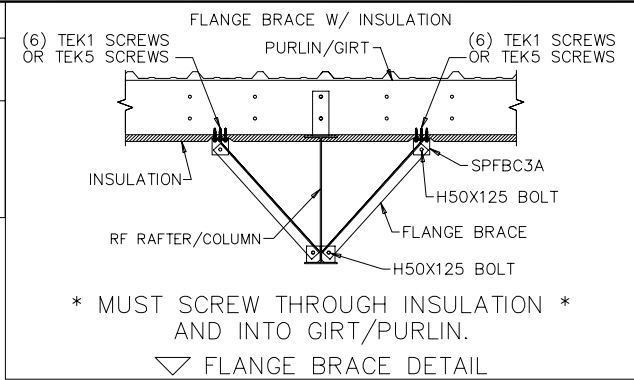
No. 30830

5/11/23

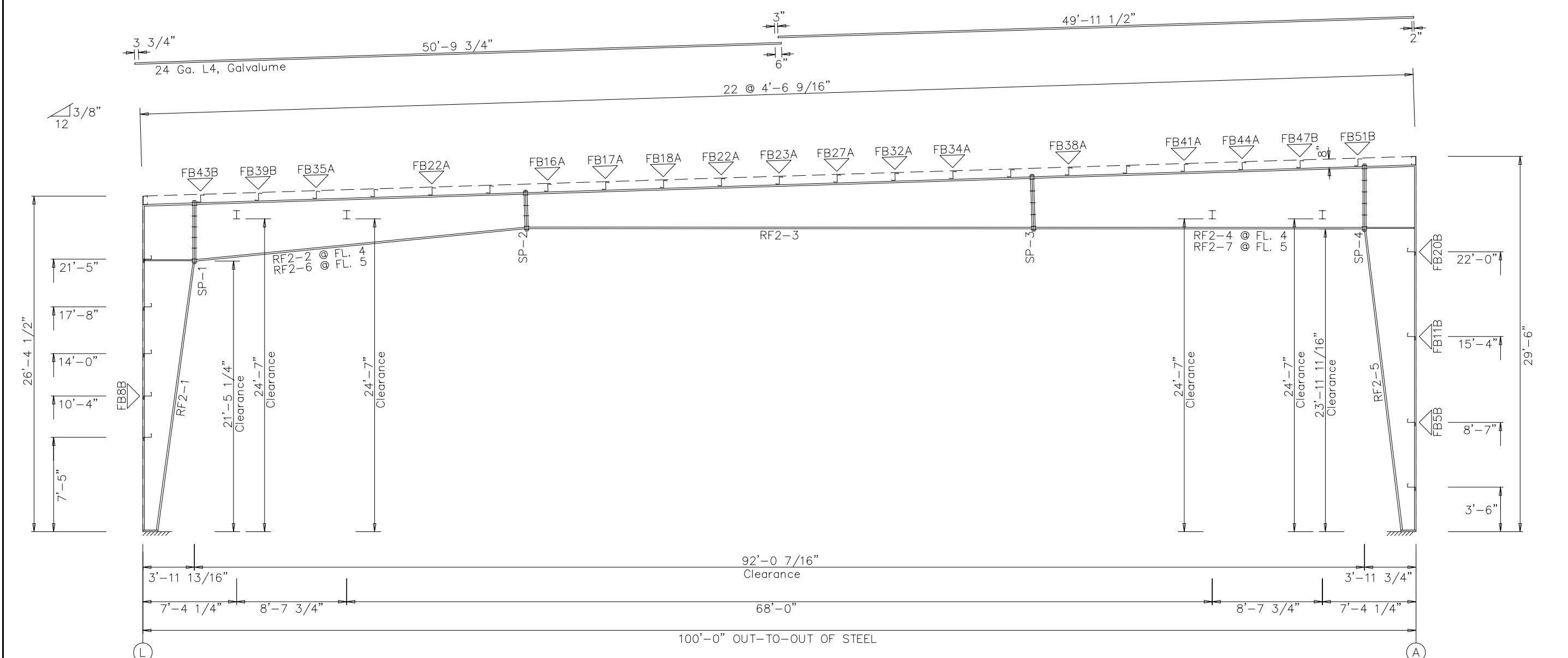
PAGE 9 OF 24

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
SP-1	4	4	8	A325	1.000	2.75
SP-2	4	4	4	A325	0.750	2.25
SP-3	4	4	6	A325	0.750	2.00
SP-4	4	4	8	A325	1.000	3.25

▽FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
B = L20X3/16
A = L15X1/8



MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start/End	Thick	Length	W x Thk x Length	W x Thk x Length	
RF2-1	11.0/36.6	0.149	180.0	8 x 1/4" x 120.0	8 x 1/2" x 255.5	
	36.6/47.0	0.313	129.1	8 x 5/16" x 60.0		
				8 x 1/4" x 127.6		
				6 x 1/4" x 47.3		
RF2-2	52.0/39.1	0.219	193.0	6 x 1/4" x 191.4	6 x 1/2" x 73.2	
	39.1/31.0	0.188	120.0	6 x 3/8" x 120.0	6 x 5/16" x 120.3	
					6 x 1/4" x 120.3	
RF2-3	31.0/38.6	0.188	240.0	6 x 1/2" x 360.0	6 x 1/4" x 476.8	
	38.6/46.0	0.219	236.5	6 x 5/16" x 116.5		
RF2-4	46.0/49.9	0.219	120.0	6 x 1/4" x 312.3	6 x 1/4" x 240.1	
	49.9/56.0	0.250	192.3		6 x 1/2" x 70.6	
RF2-5	47.0/26.2	0.250	225.1	6 x 1/4" x 47.3	8 x 1/2" x 285.8	
	26.2/11.0	0.188	120.0	8 x 1/4" x 345.1		
RF2-6	52.0/39.1	0.219	193.0	6 x 1/4" x 191.4	6 x 1/2" x 73.2	
	39.1/31.0	0.188	120.0	6 x 3/8" x 120.0	6 x 5/16" x 120.3	
RF2-7					6 x 1/4" x 120.3	
	46.0/49.9	0.219	120.0	6 x 1/4" x 312.3	6 x 1/4" x 240.1	
	49.9/56.0	0.250	192.3		6 x 1/2" x 70.6	



RIGID FRAME ELEVATION: FRAME LINE 4 5

GENERAL NOTES:

1. See Detail Sheets for Connection Information.

2. See Shipping List for Flange Brace Lengths.

DRAWING IS NOT TO SCALE

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

DWN: BJC

APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

DRAWING STATUS

CITY: OWINGS

ST / PV: MD

REVISION HISTORY

DATE

DESCRIPTION

REV.

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STATE OF MARYLAND

T. JAMES EISENMAN, JR.

PROFESSIONAL ENGINEER

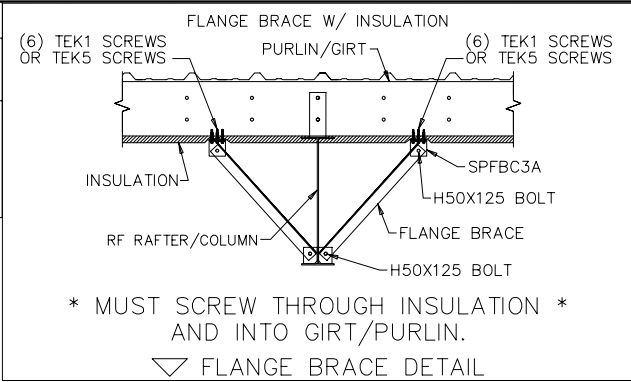
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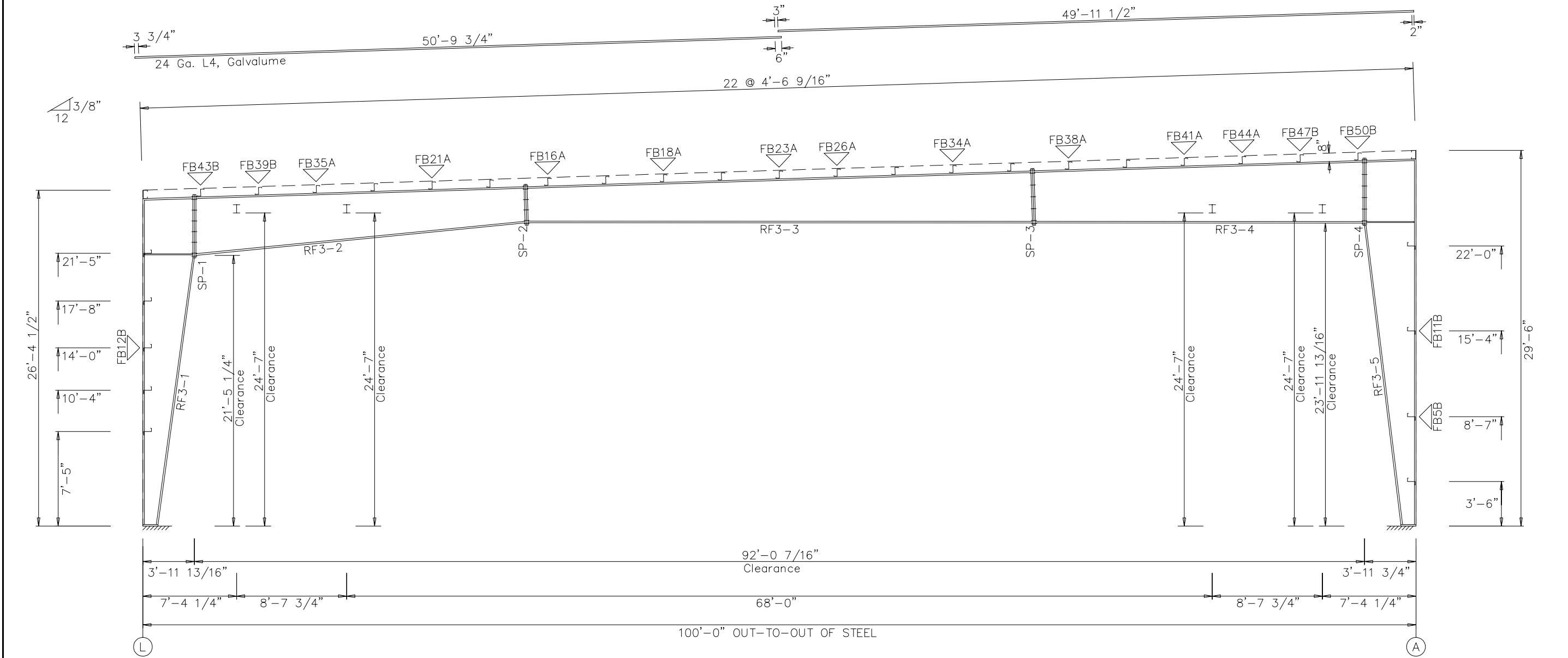
PAGE 10 OF 24

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
SP-1	4	4	8	A325	1.000	2.50
SP-2	4	4	4	A325	0.750	2.00
SP-3	4	4	6	A325	0.750	2.00
SP-4	4	4	8	A325	1.000	2.75

▽ FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
B - L20X3/16
A - L15X1/8



MEMBER	TABLE		Outside Flange			Inside Flange		
	Mark	Web	Depth	Thick	Length	W	x Thk	x Length
		Start/End						
RF3	RF3-1	11.0/36.6	36.6/47.0	0.149	180.0	8 x 1/4"	x 120.0	8 x 1/2" x 255.6
				0.313	129.2	8 x 5/16"	x 60.0	
						8 x 1/4"	x 127.7	
						6 x 1/4"	x 47.3	
						6 x 1/4"	x 191.7	
RF3-2		52.0/39.1		0.219	193.4	6 x 5/16"	x 120.0	6 x 1/2" x 73.5
RF3	RF3-3	31.0/38.5	38.5/46.0	0.188	240.0	6 x 1/4"	x 240.5	6 x 1/4" x 240.1
				0.219	237.0	6 x 1/2"	x 240.0	6 x 5/16" x 120.1
						6 x 3/8"	x 120.0	6 x 1/4" x 117.1
						6 x 5/16"	x 117.0	6 x 1/4" x 240.1
						6 x 1/4"	x 312.5	6 x 3/8" x 70.8
RF3-4		46.0/49.9		0.219	120.0	8 x 1/4"	x 47.3	8 x 1/2" x 286.0
RF3	RF3-5	49.9/56.0		0.250	192.5			
		47.0/41.4		0.250	105.2			
		41.4/11.0		0.188	240.0			



GENERAL NOTES:

- See Detail Sheets for Connection Information.
- See Shipping List for Flange Brace Lengths.

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

DWN: BJC

APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

DRAWING STATUS

CITY: OWINGS

ST/PV: MD

REVISION HISTORY

DESCRIPTION

DATE

FOR APPROVAL:

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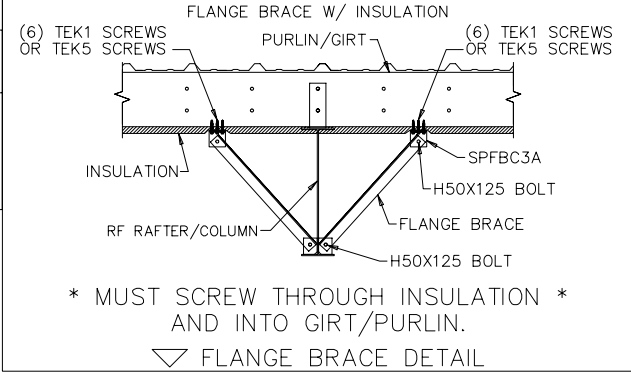
FINAL DRAWINGS.

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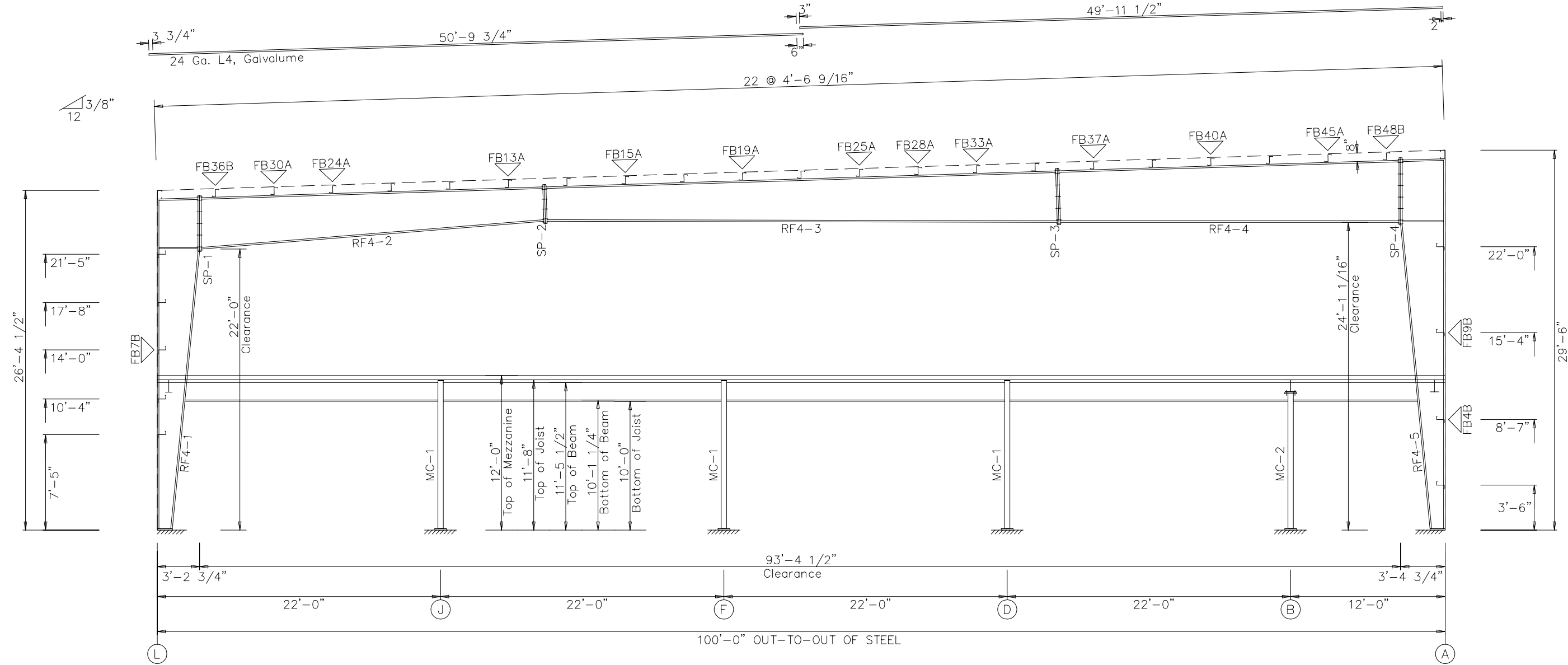
PAGE 11 OF 24

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
SP-1	4	4	6	A325	1.000	2.75
SP-2	4	4	4	A325	0.750	2.25
SP-3	4	4	6	A325	0.750	2.00
SP-4	4	4	8	A325	1.000	2.50

▽ FLANGE BRACES: Both Sides(U.N.)
FBxXB(1)
B - L20X3/16
A - L15X1/8



MEMBER	Web Depth		Web Plate		Outside Flange			Inside Flange		
	Start	End	Thick	Length	W	Thk	x Length	W	Thk	x Length
RF4-1	11.0	36.9	0.250	248.9	8 x 1/4"		x 307.7	8 x 3/8"		x 120.6
RF4-2	36.9	38.0	0.250	60.0	6 x 5/16"		x 38.3	8 x 1/2"		x 140.7
	45.0	40.9	0.219	81.9	6 x 5/16"		x 80.4	6 x 3/8"		x 82.0
	40.9	29.0	0.188	240.0	6 x 1/4"		x 120.0	6 x 1/4"		x 240.3
RF4-3	29.0	38.1	0.188	270.0	6 x 5/16"		x 120.0			
	38.1	45.0	0.188	207.1	6 x 1/2"		x 360.0	6 x 1/4"		x 477.3
	45.0	52.6	0.219	240.0	6 x 5/16"		x 117.1	6 x 1/4"		x 240.1
RF4-4	52.6	55.0	0.250	79.5	6 x 1/4"		x 319.5	6 x 5/16"		x 77.8
	40.0	38.5	0.250	75.2	8 x 1/2"		x 165.8	8 x 1/2"		x 165.8
	38.5	11.0	0.250	270.0	8 x 1/4"		x 345.2	8 x 3/8"		x 120.6



RIGID FRAME ELEVATION: FRAME LINE 7

GENERAL NOTES:

- See Detail Sheets for Connection Information.
- See Shipping List for Flange Brace Lengths.



7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

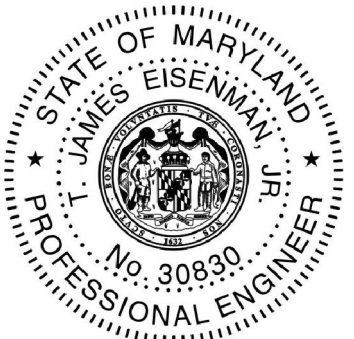
DWN: BJC

APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

DRAWING STATUS		REVISION HISTORY	
CITY: OWINGS	ST/ PV: MD	DESCRIPTION	DATE
REV.			
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FOR CONSTRUCTION: FINAL DRAWINGS.			

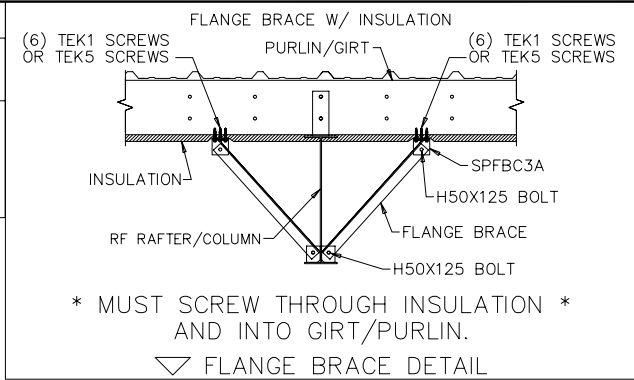


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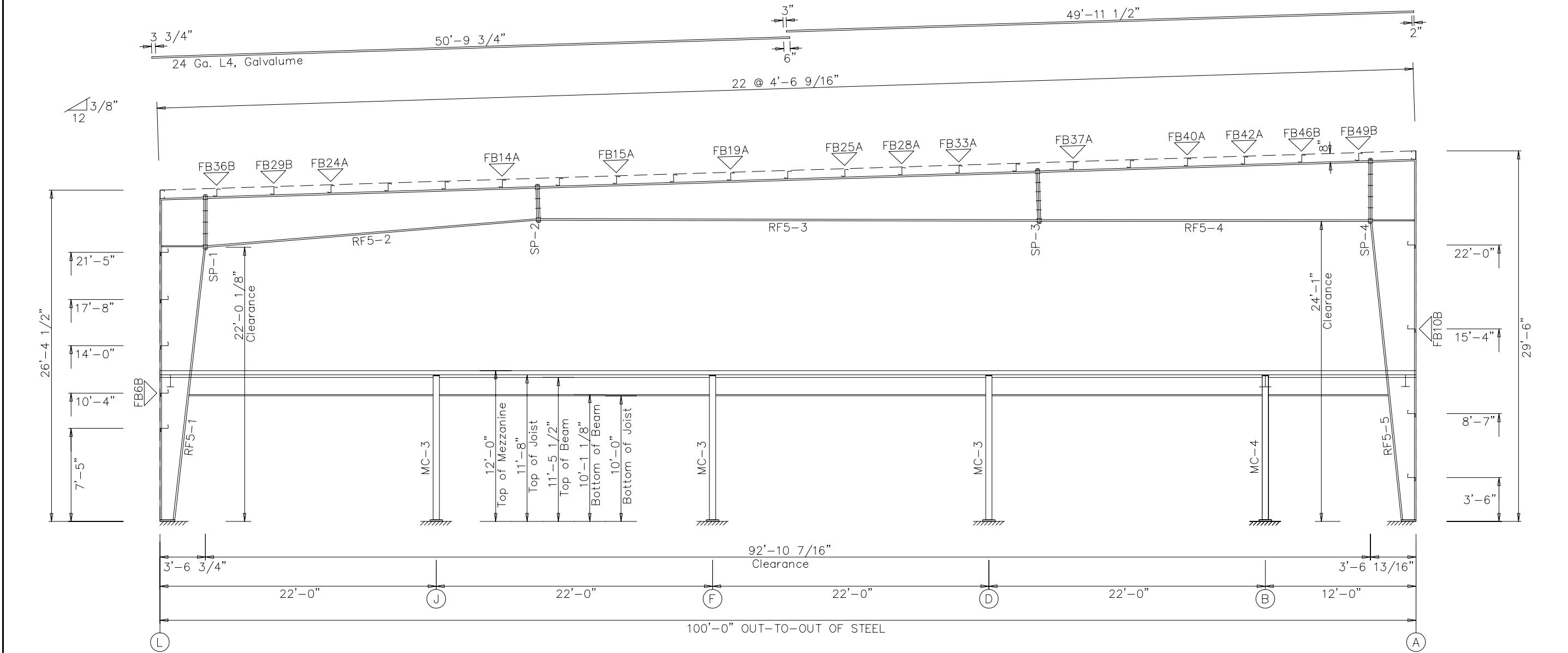
DRAWING IS NOT TO SCALE

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
SP-1	4	4	6	A325	1.000	2.75
SP-2	4	4	4	A325	0.750	2.25
SP-3	4	4	8	A325	0.500	1.50
SP-4	4	4	8	A325	1.000	2.75

▽ FLANGE BRACES: Both Sides(U.N.)
FBxxB(1)
B - L20X3/16
A - L15X1/8



MEMBER TABLE								
Mark	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W	x Thk x Length	W	x Thk x Length
RF5-1	11.0/32.5	0.250	180.0		8	x 1/4" x 307.7	8	x 3/8" x 120.8
	32.5/42.0	0.313	129.1		6	x 1/4" x 42.3	8	x 1/2" x 141.1
RF5-2	45.0/35.1	0.219	197.9		6	x 1/4" x 196.4	6	x 1/2" x 78.0
	35.1/29.0	0.149	120.0		6	x 3/8" x 120.0	6	x 1/4" x 240.3
RF5-3	29.0/38.1	0.188	270.0		6	x 1/2" x 360.0	6	x 1/4" x 477.4
	38.1/45.0	0.188	207.1		6	x 5/16" x 117.1		
RF5-4	45.0/52.6	0.219	240.0		6	x 1/4" x 317.4	6	x 1/4" x 240.1
	52.6/55.0	0.250	77.4				6	x 3/8" x 75.8
RF5-5	42.0/40.4	0.250	75.2		6	x 1/4" x 42.3	8	x 1/2" x 286.5
	40.4/11.0	0.250	270.0		8	x 1/4" x 105.2		
					8	x 5/16" x 120.0		
					8	x 1/4" x 120.0		



RIGID FRAME ELEVATION: FRAME LINE 8

GENERAL NOTES:

- See Detail Sheets for Connection Information.
- See Shipping List for Flange Brace Lengths.

DRAWING IS NOT TO SCALE

SOLID

STEEL BUILDINGS

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM

DWN: BJC

APPD: IRM

F.O. 27081

CITY: OWINGS

ST/ PV: MD

DRAWING STATUS

REVISION HISTORY

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STATE OF MARYLAND

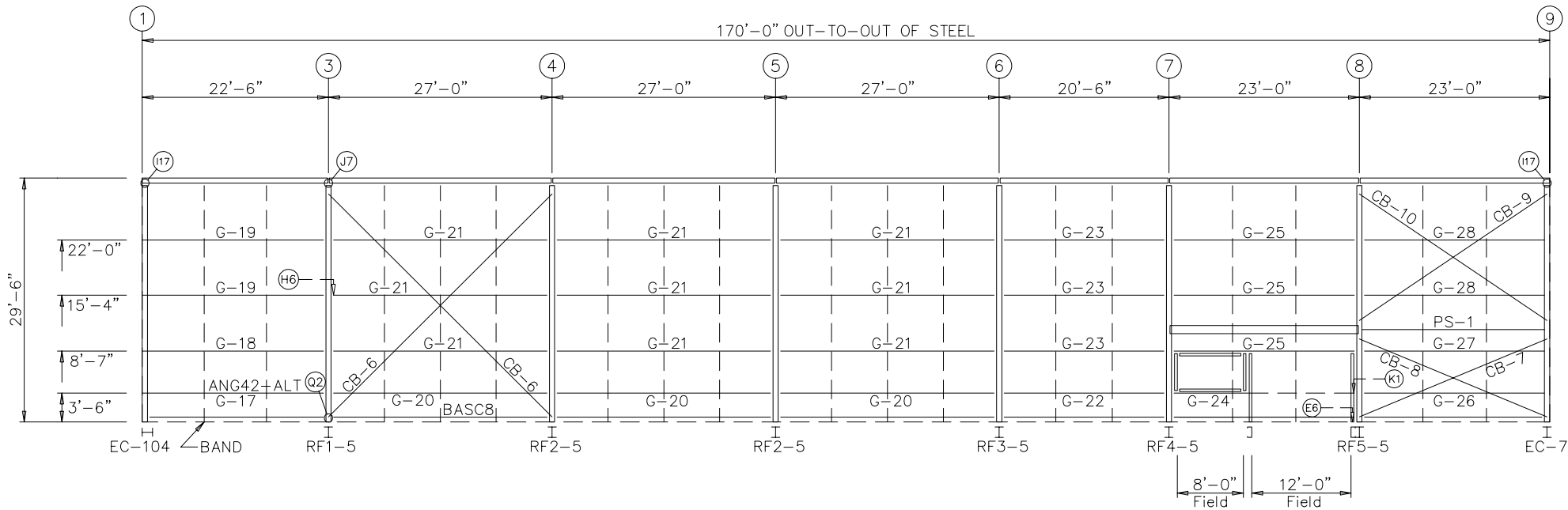
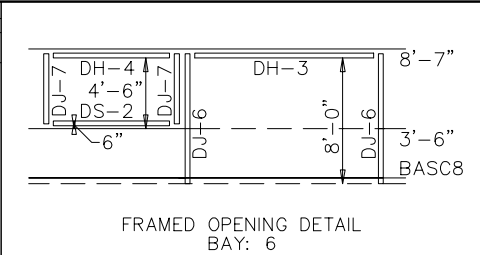
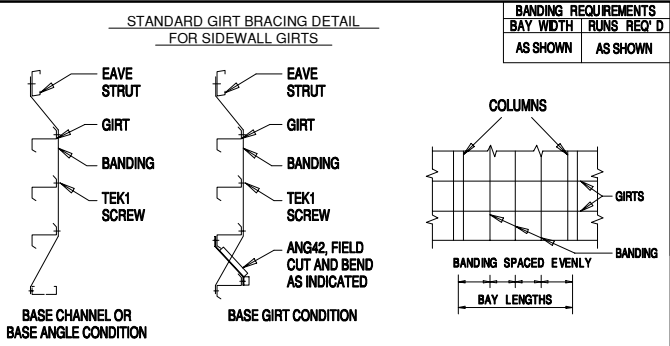
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PROFESSIONAL ENGINEER

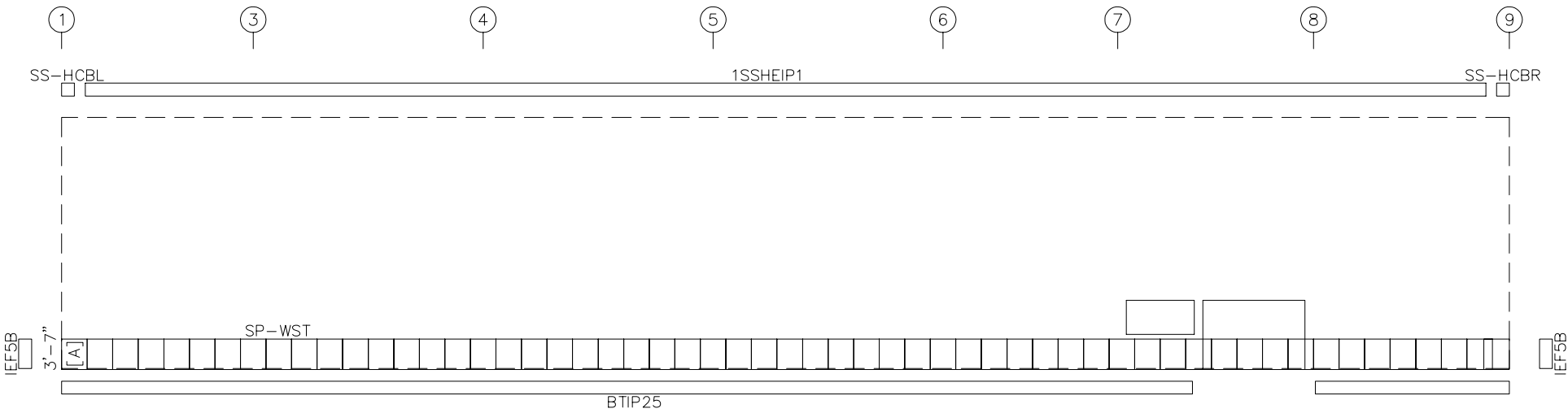
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5/11/23

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SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 2 1/2" MBCI TUFFCAST IMP - By Others
[A] PANELS: 24 Ga. RR - Galvalume

BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
PS-1/RF,EC	2	A325	1/2"	1 1/4"

MEMBER TABLE			
FRAME LINE A			
QUAN	MARK	PART	LENGTH
2	DJ-6	08X35C14	8'-6 3/4"
2	DJ-7	08X35C14	5'-0 1/2"
1	DH-3	08X30C14	12'-0"
1	DH-4	08X30C14	8'-0"
1	DS-2	08X30C14	8'-0"
1	G-17	08X25Z13	22'-1 1/2"
1	G-18	08X25Z12	22'-1 1/2"
2	G-19	08X25Z10	22'-1 1/2"
3	G-20	08X25Z13	26'-3 1/2"
9	G-21	08X25Z10	26'-3 1/2"
1	G-22	08X25Z14	19'-9 1/2"
3	G-23	08X25Z13	19'-9 1/2"
1	G-24	08X25Z14	22'-3 1/2"
3	G-25	08X25Z10	22'-3 1/2"
1	G-26	08X25Z13	21'-11 1/2"
1	G-27	08X25Z12	21'-11 1/2"
2	G-28	08X25Z10	21'-11 1/2"
1	PS-1	P4X11	22'-7 3/4"
2	CB-6	CABLE500	36'-0 1/2"
1	CB-7	CABLE500	22'-2 5/16"
1	CB-8	CABLE500	22'-2 5/16"
1	CB-9	CABLE500	24'-9 15/16"
1	CB-10	CABLE500	25'-3 13/16"

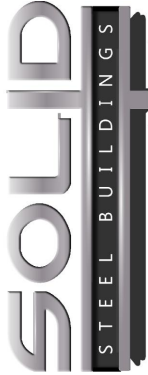
NOTE(S): (1.) FIELD DRILL GIRTS TO ALLOW CABLES TO PASS.

DRAWING IS NOT TO SCALE

TRIM COLORS

EAVE TRIM = Light Stone	CORNER TRIM = Light Stone
BASE TRIM = Light Stone	GUTTER = Light Stone
DOOR TRIM = Light Stone	DOWNSPOUTS = Light Stone
RAKE TRIM = Light Stone	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	

* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.



7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23 REVISION: 0

ENG: IRM DWN: BJC APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

CITY: OWINGS ST / PV: MD

REVISION HISTORY

DATE

DESCRIPTION

REV.

DRAWING STATUS

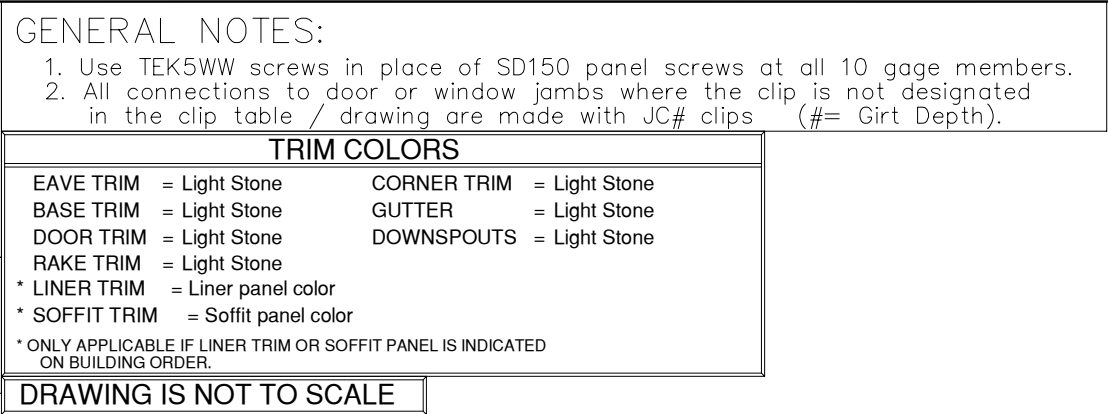
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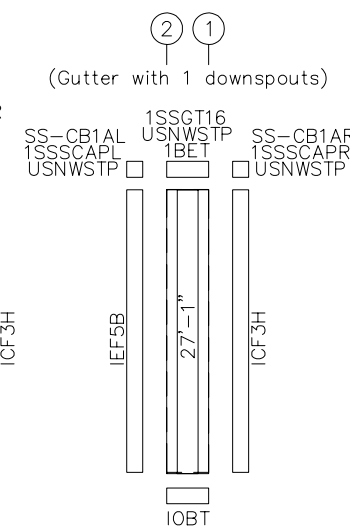
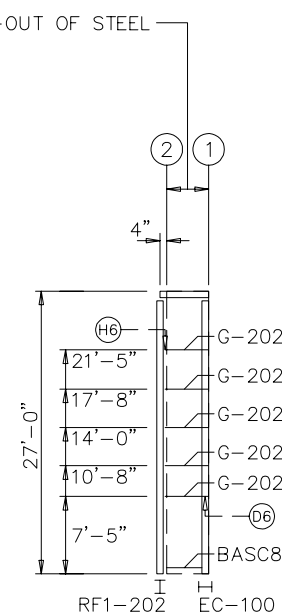


5/11/23



CONNECTION PLATES FRAME LINE L		
ID	QUAN	MARK/PART
5	6	JC

NOTE(S): (1.) FIELD DRILL GIRTS TO
ALLOW CABLES TO
PASS.



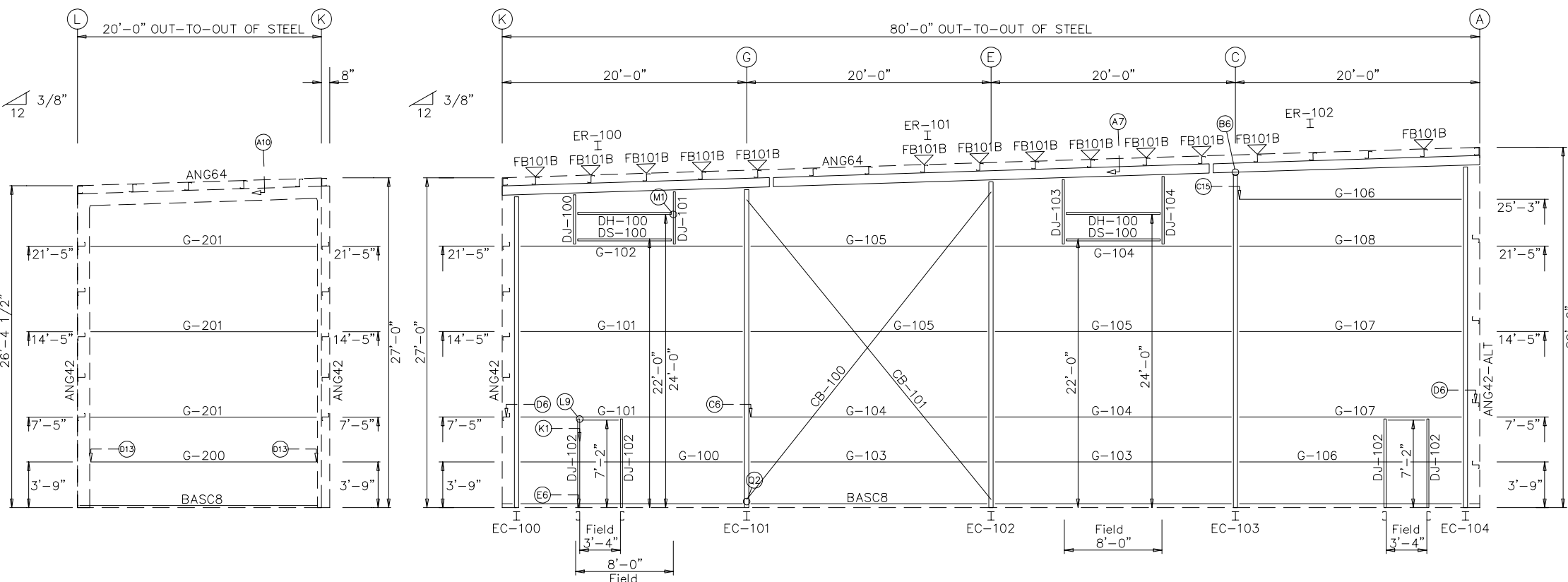
	
7630 MEADOW RUN LANE	
100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"	
DATE: 3/31/23	REVISION: 0
ENG: IRM	DWN: BJC APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE		CITY: OWINGS		ST / PV: MD
DRAWING STATUS		REVISION HISTORY		
		REV.	DESCRIPTION	DATE
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FOR CONSTRUCTION: FINAL DRAWINGS.				

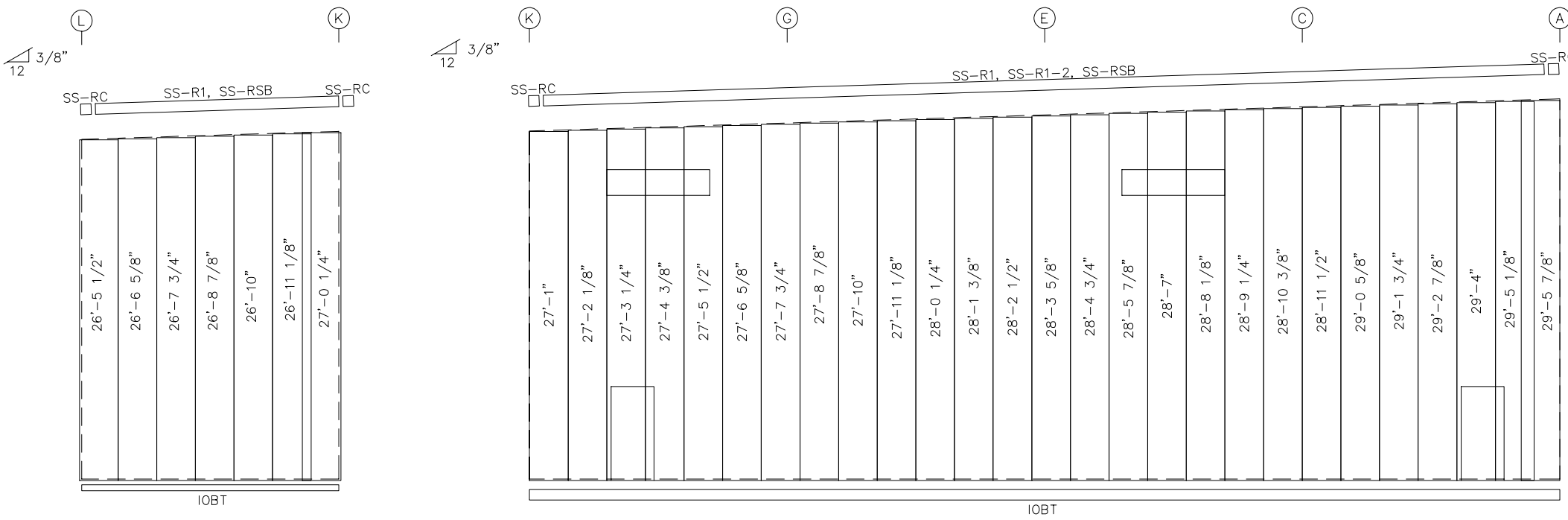


5/11/23



ENDWALL FRAMING: FRAME LINE 2

ENDWALL FRAMING: FRAME LINE 1

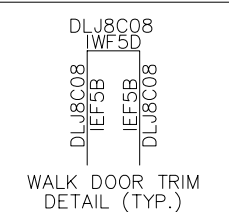
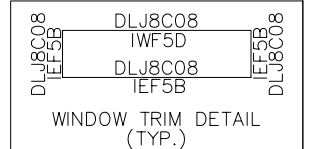


ENDWALL SHEETING & TRIM: FRAME LINE 2
PANELS: 26 Ga. R - Light Stone

ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. R - Light Stone

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. See detail C7A for field coping of coldform endwall column flange braces.
3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

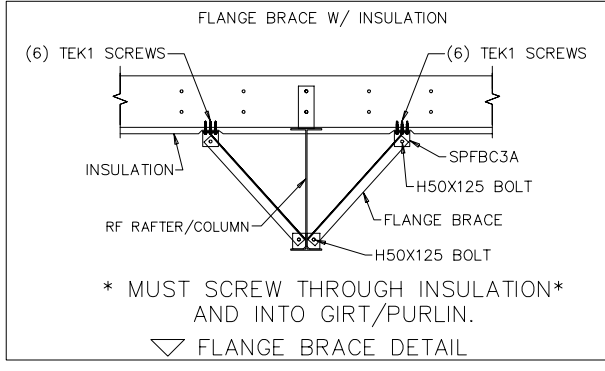


BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-100/ER-101	8	A325	1/2"	1 1/2"
ER-101/ER-102	8	A325	1/2"	1 1/2"
Columns/Raf	2	A325	3/4"	1 3/4"
Jamb	2	A325	1/2"	1 1/2"

MEMBER TABLE			
FRAME LINE 1			
QUAN	MARK	PART	LENGTH
1	EC-100	W12X16	25'-7 13/16"
1	EC-101	W12X16	26'-2 7/8"
1	EC-102	W12X16	26'-10 3/8"
1	EC-103	W12X16	27'-5 7/8"
1	EC-104	W12X16	28'-0 15/16"
1	ER-100	W8X10	22'-0 5/8"
1	ER-101	W8X10	36'-0 1/4"
1	ER-102	W8X10	21'-11 7/8"
1	DJ-100	08X35C16	4'-1 1/8"
1	DJ-101	08X35C16	4'-4 1/8"
4	DJ-102	08X35C16	7'-4 3/4"
1	DJ-103	08X35C16	5'-4 1/8"
1	DJ-104	08X35C16	5'-7 1/8"
2	DH-100	08X35C16	8'-0"
2	DS-100	08X35C16	8'-0"
1	G-100	08X25Z16	18'-5 1/2"
2	G-101	08X25Z12	18'-5 1/2"
1	G-102	08X25Z13	18'-5 1/2"
2	G-103	08X25Z16	19'-7 1/2"
3	G-104	08X25Z13	19'-7 1/2"
3	G-105	08X25Z12	19'-7 1/2"
2	G-106	08X25Z16	18'-5 1/2"
2	G-107	08X25Z12	18'-5 1/2"
1	G-108	08X25Z13	18'-5 1/2"
1	CB-100	CABLE375	30'-4 1/16"
1	CB-101	CABLE375	29'-10 1/4"

FLANGE BRACE TABLE		
FRAME LINE 1		
VID	MARK	LENGTH
1	FB101B	1'-2 3/8"

MEMBER TABLE			
FRAME LINE 2			
QUAN	MARK	PART	LENGTH
1	G-200	08X25Z16	18'-7 1/2"
3	G-201	08X25Z13	18'-7 1/2"



NOTE(S): (1.) FIELD DRILL GIRTS TO ALLOW CABLES TO PASS.

DRAWING IS NOT TO SCALE

TRIM COLORS			
EAVE TRIM	= Light Stone	CORNER TRIM	= Light Stone
BASE TRIM	= Light Stone	GUTTER	= Light Stone
DOOR TRIM	= Light Stone	DOWNSPOUTS	= Light Stone
RAKE TRIM	= Light Stone		
* LINER TRIM	= Liner panel color		
* SOFFIT TRIM	= Soffit panel color		
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.			

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/29/23

REVISION: 0

ENG: IRM

DWN: BJC

APPD: IRM

F.O. 27081

DRAWING STATUS

CITY: OWINGS

ST / PV: MD

REVISION HISTORY

DESCRIPTION

DATE

FOR APPROVAL:

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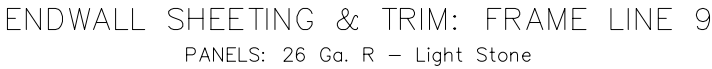
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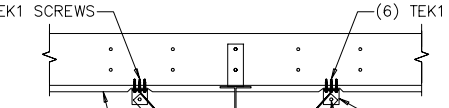
FINAL DRAWINGS.

5/11/23

PAGE 16 OF 24



DLJ8C08
IWF5D
DLJ8C08
IEF5B
IEF5B
DLJ8C08
WALK DOOR TRIM
DETAIL (TYP.)



FLANGE BRACE W/ INSULATION

(6) TEK1 SCREWS

INSULATION

RF RAFTER/COLUMN

SPFBC3A

H50X125 BOLT

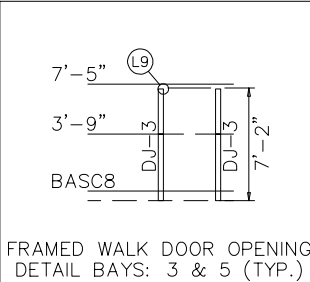
FLANGE BRACE

H50X125 BOLT

* MUST SCREW THROUGH INSULATION*
AND INTO GIRT/PURLIN.

▽ FLANGE BRACE DETAIL

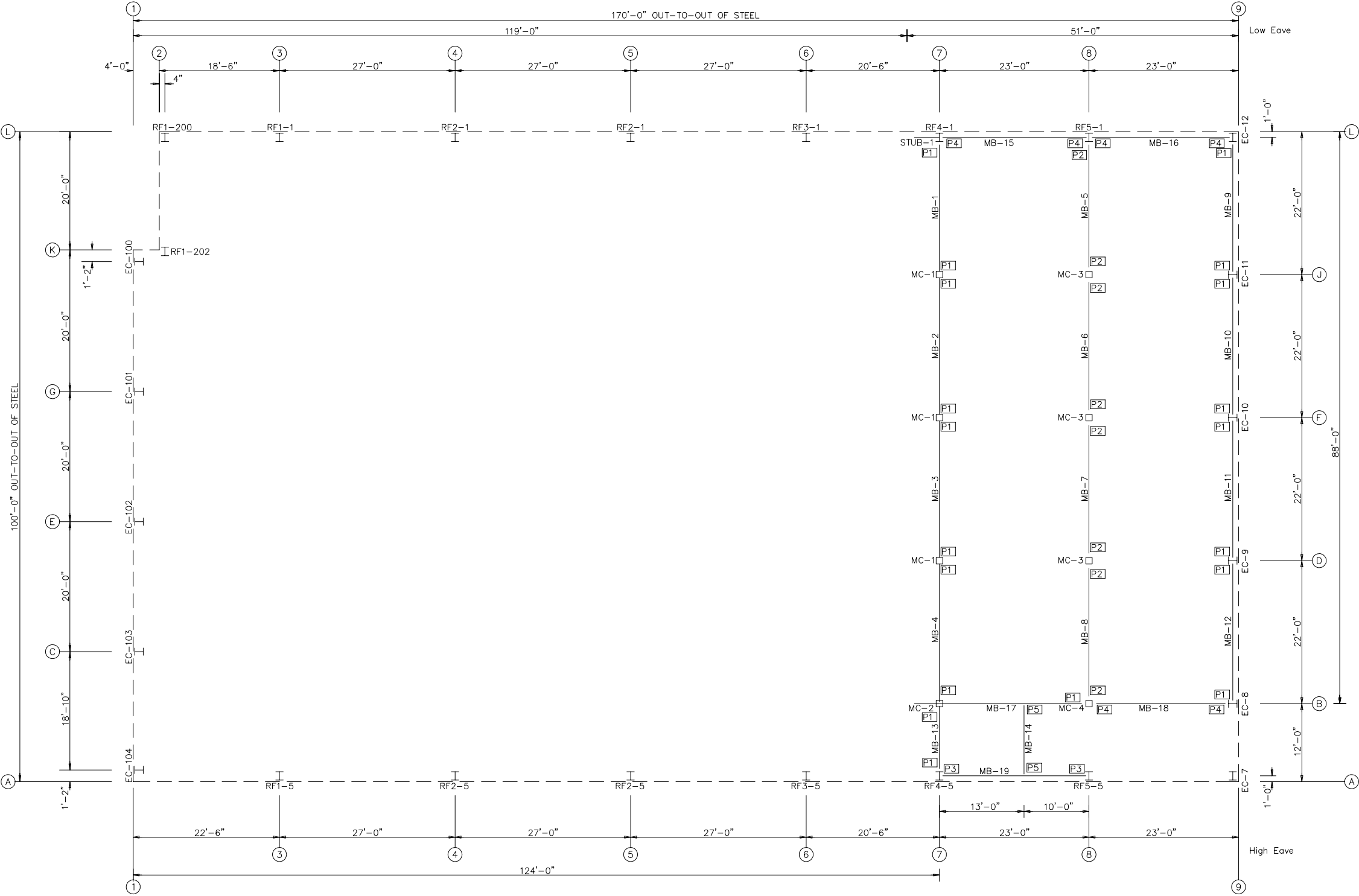
NOTE(S): (1.) FIELD DRILL GIRTS TO
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PASS.



MEMBER TABLE			
FRAME LINE 9			
QUAN	MARK	PART	LENGTH
1	EC-7	W12X26	28'-0 1/8"
1	EC-8	W12X16	27'-6 7/8"
1	EC-9	W12X16	26'-10 5/8"
1	EC-10	W12X16	26'-2 3/8"
1	EC-11	W12X16	25'-6 1/8"
1	EC-12	W12X26	24'-11"
1	ER-5	W10X12	32'-2"
1	ER-6	W10X12	21'-10 1/16"
1	ER-7	W10X12	22'-0 1/8"
1	ER-8	W10X12	24'-0 11/16"
6	DJ-3	08X35C16	7'-4 3/4"
5	G-10	08X25Z16	10'-9 5/16"
3	G-11	08X25Z14	21'-7 1/2"
2	G-12	08X25Z13	21'-7 1/2"
7	G-13	08X25Z10	21'-7 1/2"
1	G-14	08X25Z14	20'-9 5/16"
2	G-15	08X25Z10	20'-9 5/16"
1	G-16	08X25Z12	20'-9 5/16"
2	CB-3	CABLE500	21'-4 5/16"
1	CB-4	CABLE500	22'-9 11/16"
1	CB-5	CABLE500	23'-1 13/16"

FLANGE BRACE TABLE		
FRAME LINE 9		
VID	MARK	LENGTH
1	FB3B	1'-3 5/8"

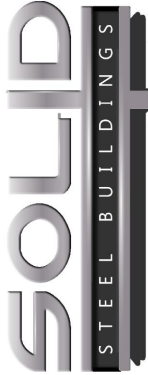
The seal is circular with a double-lined border. The outer ring contains the text "STATE OF MARYLAND" at the top and "PROFESSIONAL ENGINEER" at the bottom, separated by two stars. Inside this ring, the name "T. JAMES EISENMAN, JR." is written in a larger font. At the center is the Seal of the State of Maryland, which depicts two figures holding a shield with a cross and four smaller shields, topped by a crest with a bent arm holding a sword. The words "SEAL OF THE STATE OF MARYLAND" are inscribed around the central seal. Below the central seal, the number "No. 30830" is printed.



BOLT TABLE				
FLOOR FRAMING				
LOCATION		QUAN	TYPE	DIA LENGTH
STUB-1 / RF		4	A325	3/4" 1 1/2"
MB-17 / MC-2		4	A325	3/4" 2 1/4"

MEMBER TABLE			
QUAN	MARK	PART	LENGTH
1	MB-1	W16X50	19'-5 3/16"
1	MB-2	W16X50	21'-4 1/2"
1	MB-3	W16X50	21'-4 1/2"
1	MB-4	W16X50	21'-6 7/8"
1	MB-5	W16X57	19'-2 5/8"
1	MB-6	W16X57	21'-3 1/2"
1	MB-7	W16X57	21'-3 1/2"
1	MB-8	W16X57	21'-3 1/2"
1	MB-9	W16X31	20'-9 3/16"
1	MB-10	W16X31	21'-9 1/4"
1	MB-11	W16X31	21'-9 1/4"
1	MB-12	W16X31	21'-9 1/4"
1	MB-13	W16X31	9'-7 13/16"
1	MB-14	W8X18	10'-9 3/16"
1	MB-15	W12X26	22'-9 1/4"
1	MB-16	W12X26	22'-3 1/4"
1	MB-17	W18X40	27'-7 1/2"
1	MB-18	W12X26	21'-6 1/2"
1	MB-19	W12X40	22'-9 1/4"
3	MC-1	HSS05054	11'-8"
1	MC-2	HSS05054	10'-2 1/8"
3	MC-3	HSS06064	11'-8"
1	MC-4	HSS06064	11'-8"
1	STUB-1	W12X26	4'-11 5/8"

PERIMETER BEAM BOLT TABLE				
ID	QUAN	TYPE	DIA	LENGTH
P1	4	A325	3/4"	1 3/4"
P2	4	A325	3/4"	2"
P3	3	A325	3/4"	1 3/4"
P4	3	A325	3/4"	1 1/2"
P5	2	A325	3/4"	1 1/2"



7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23 REVISION: 0

ENG: IRM DWN: BJC APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

CITY: OWINGS ST / PV: MD

REVISION HISTORY

REV. DESCRIPTION DATE

DRAWING STATUS

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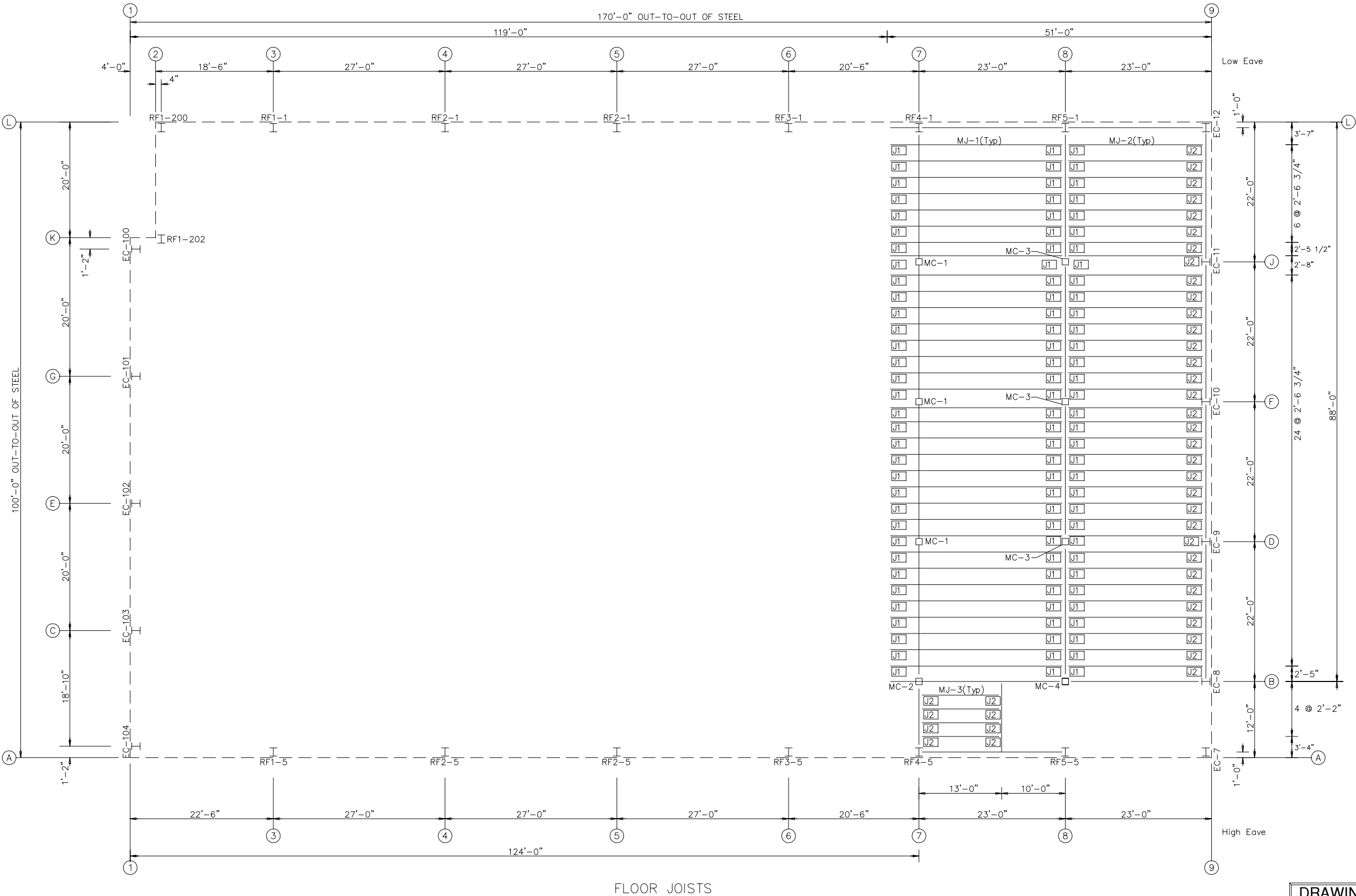
FOR CONSTRUCTION:
FINAL DRAWINGS.



5/11/23

DRAWING IS NOT TO SCALE

NOTE(S): 1) STAIRS, RAILS, AND LANDING ARE SUPPLIED BY OTHERS.
2) BRIDGING PER JOIST MANUFACTURER'S DRAWING.



MEMBER TABLE			
QUAN	MARK	PART	LENGTH
33	MJ-1	20K449/323	27'-11 1/2"
33	MJ-2	20K449/323	22'-11 1/2"
4	MJ-3	20K449/323	13'-4 7/8"

JOIST BOLT TABLE				
ID	QUAN	TYPE	DIA	LENGTH
J1	2	A325	1/2"	1 1/2"
J2	2	A325	1/2"	1 1/4"



7630 MEADOW RUN LANE

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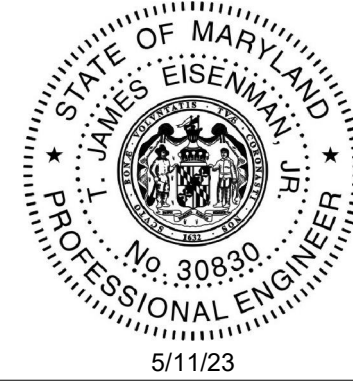
7630 MEADOW RUN LANE

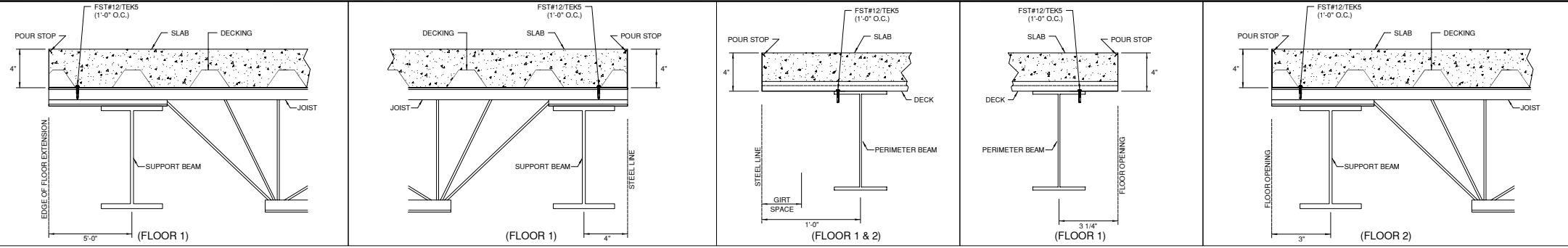
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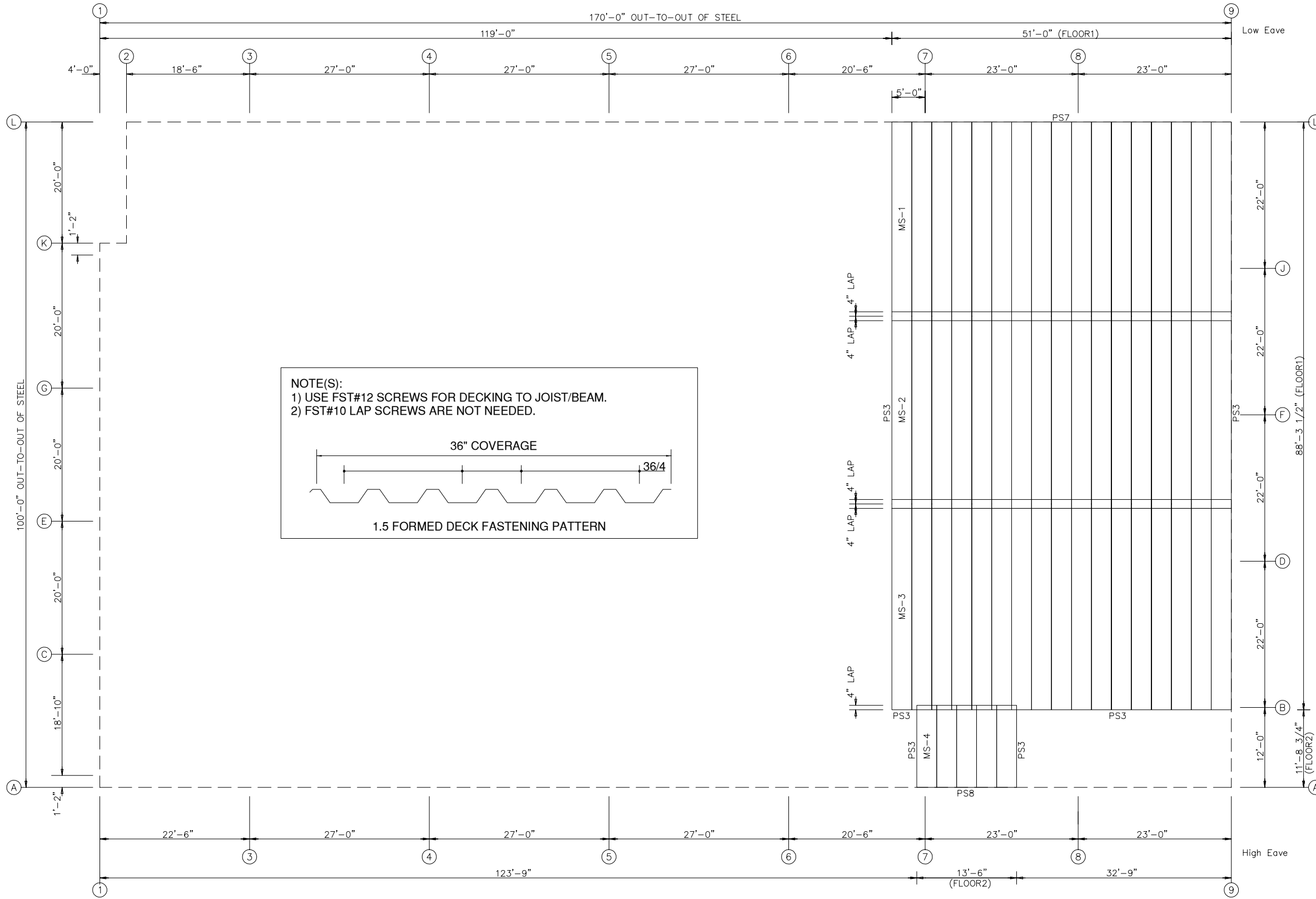
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CITY: OWINGS		ST / PV: MD
REVISION HISTORY		
REV.	DESCRIPTION	DATE





FLOOR PANEL TABLE			
QUAN	MARK	PART	LENGTH
17	MS-1	1.5FD22	29'-6 1/4"
17	MS-2	1.5FD22	28'-10 1/4"
17	MS-3	1.5FD22	31'-2 1/4"
5	MS-4	1.5FD22	12'-0 3/4"



NOTE(S):
1) USE FST#12 SCREWS FOR DECKING TO JOIST/BEAM.
2) FST#10 LAP SCREWS ARE NOT NEEDED.

36" COVERAGE

1.5 FORMED DECK FASTENING PATTERN

FLOOR SHEETING & TRIM
PANELS: 22 Ga. 1.5FD - Galvanized

DRAWING IS NOT TO SCALE

7630 MEADOW RUN LANE

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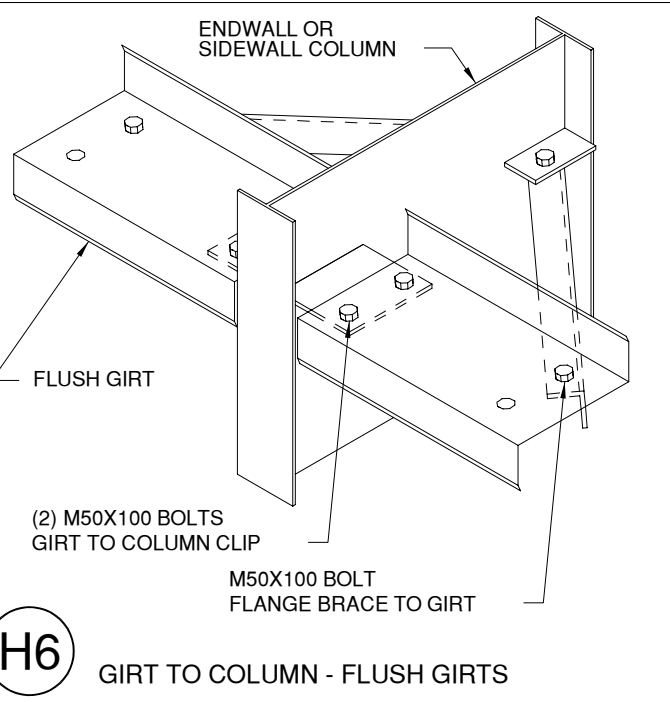
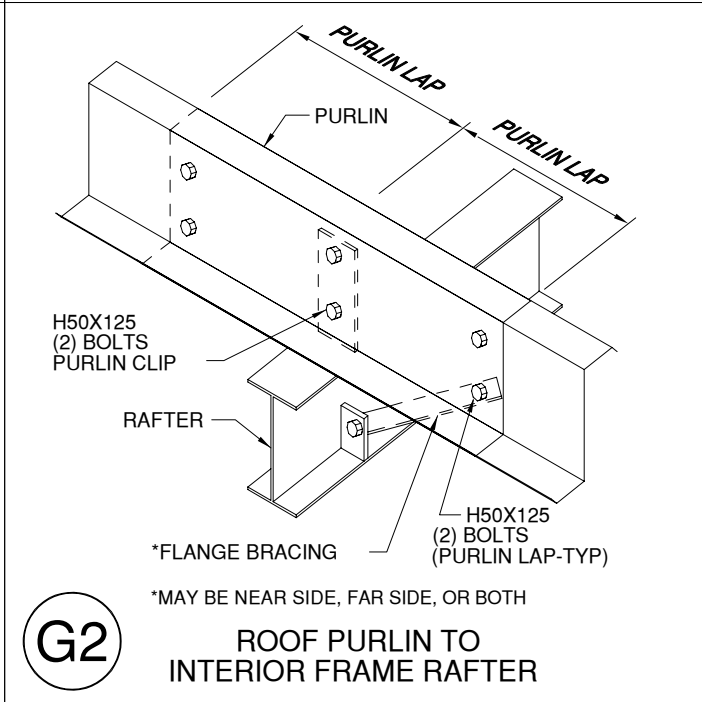
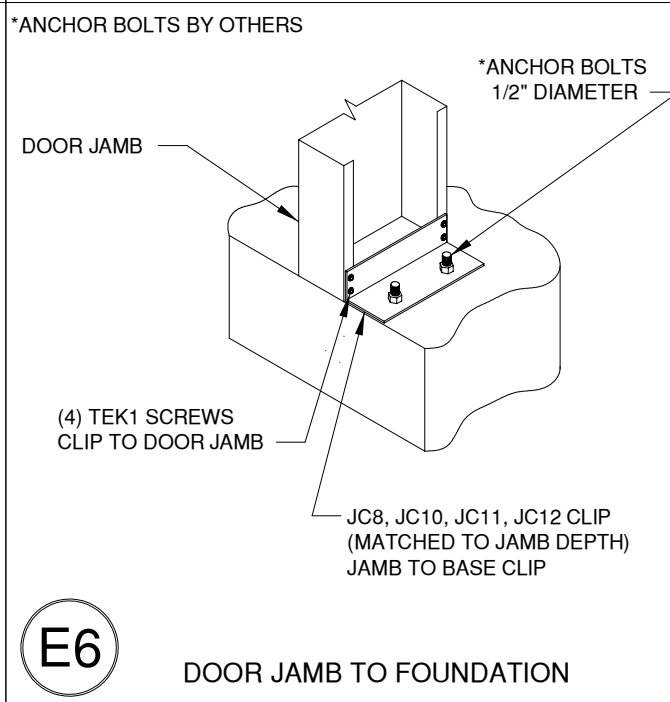
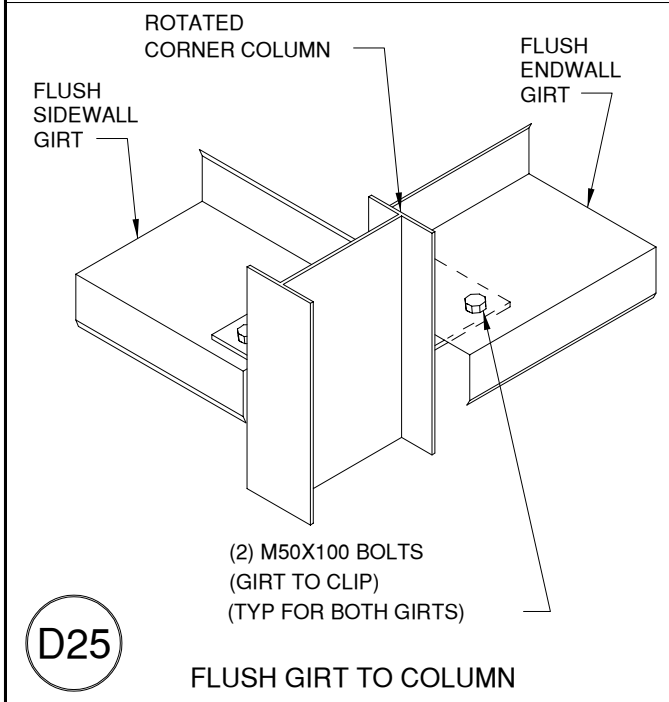
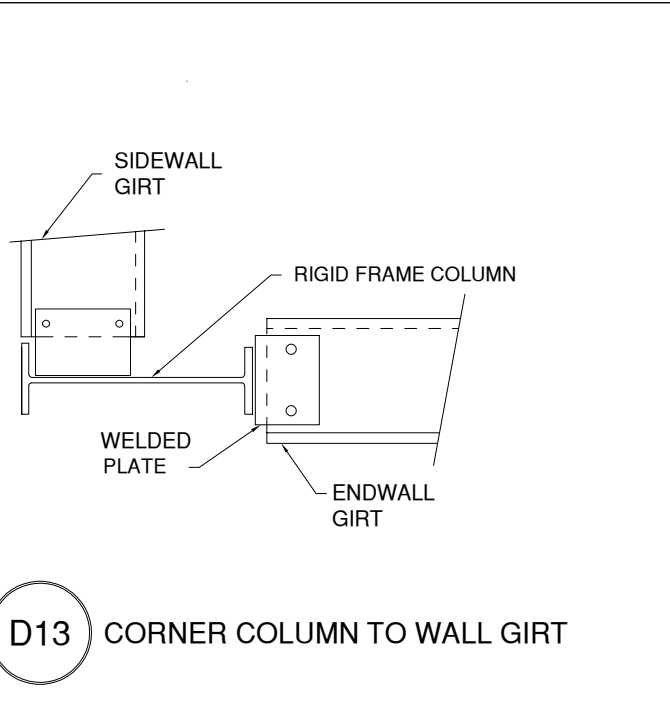
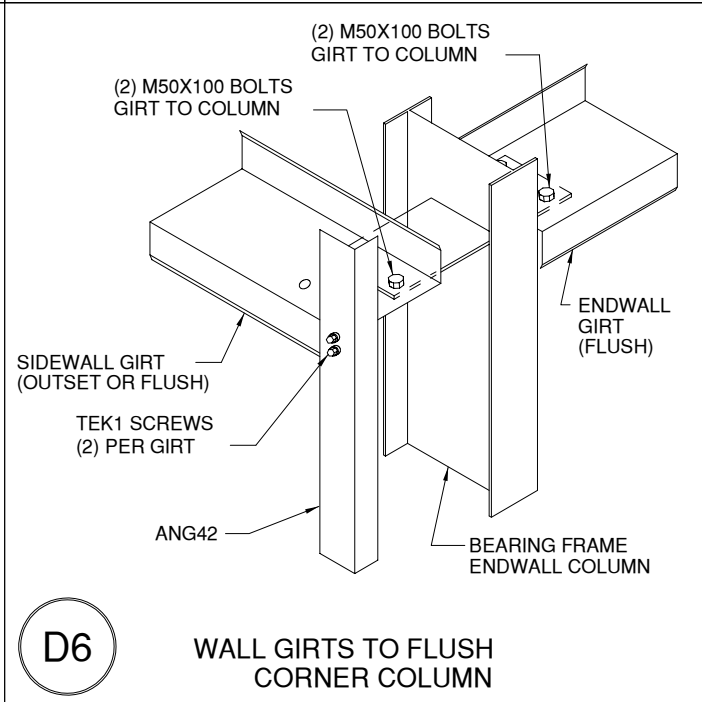
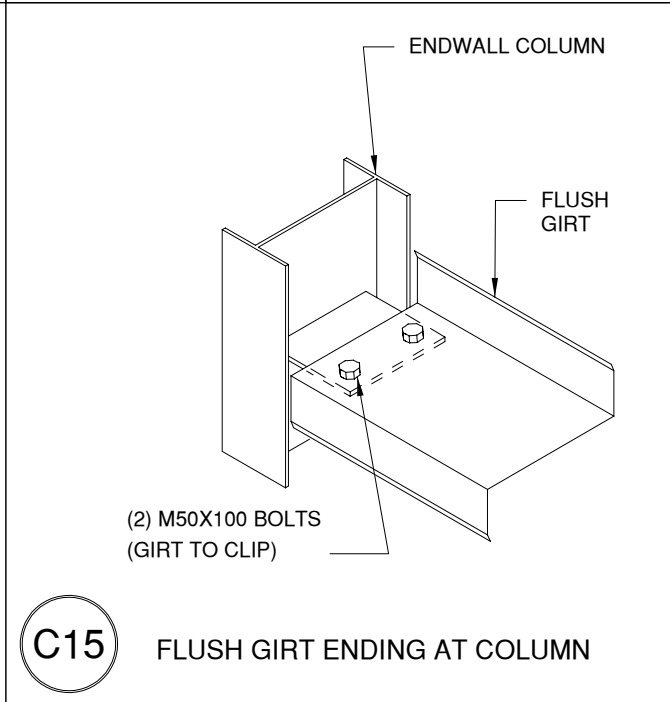
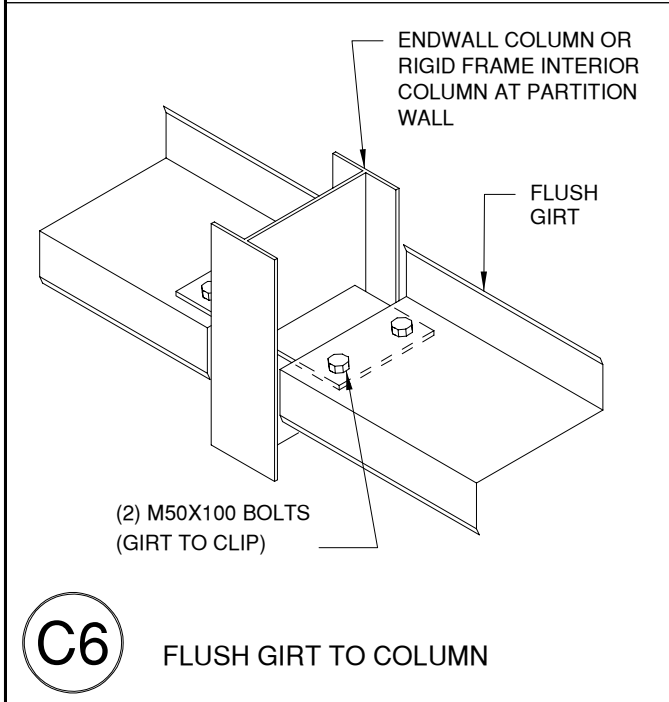
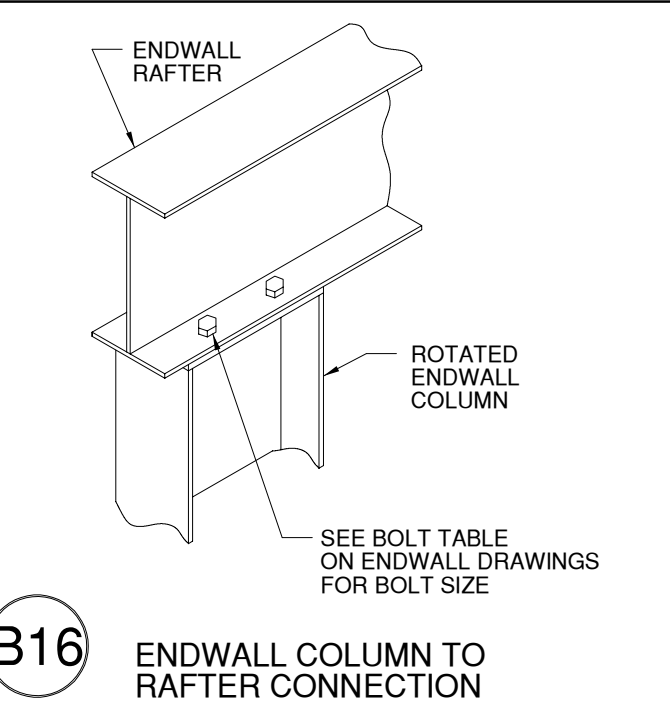
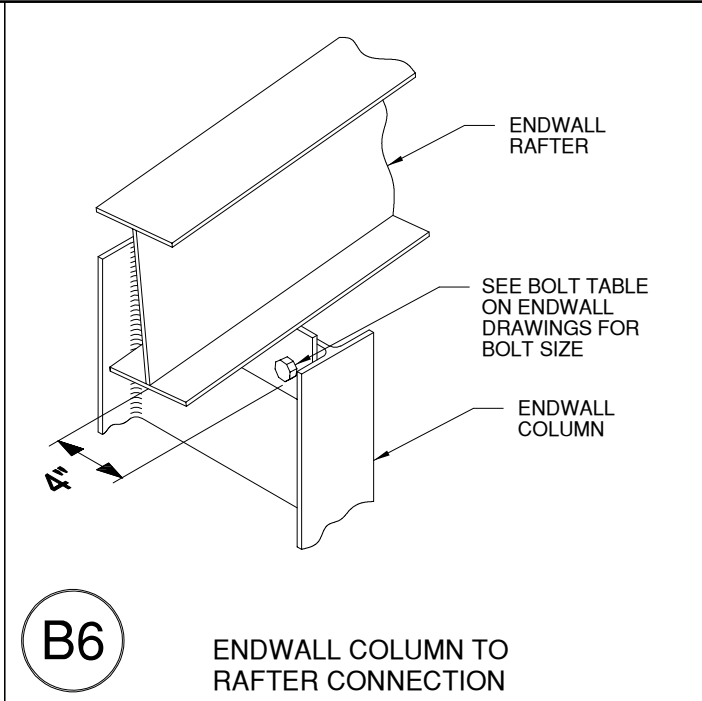
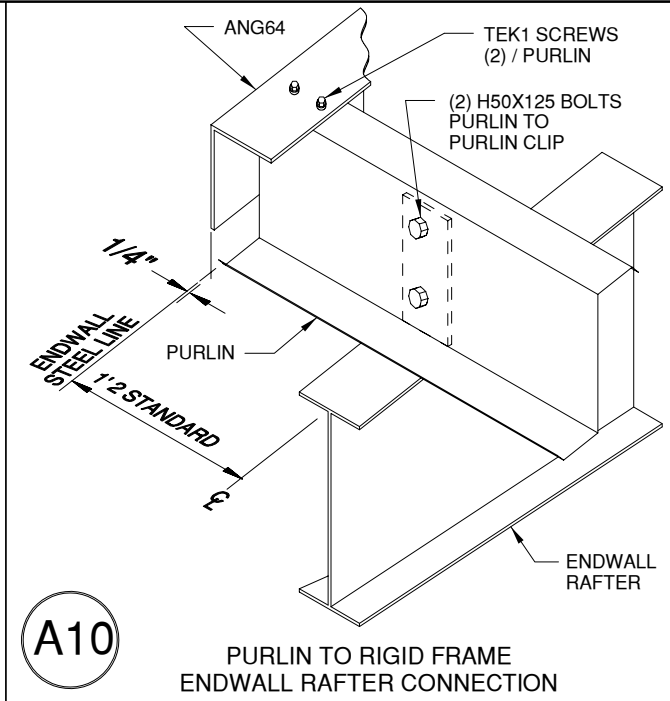
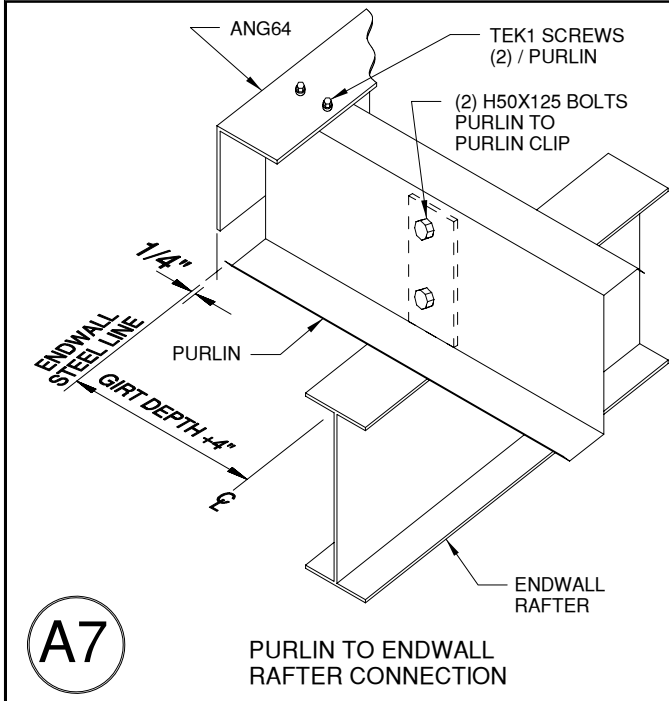
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5/11/23

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7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

DATE: 3/31/23

REVISION: 0

ENG: IRM DWN: BJC APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

CITY: OWINGS

ST / PV: MD

REVISION HISTORY

DESCRIPTION

DATE

REV.

DRAWING STATUS

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FOR CONSTRUCTION: FINAL DRAWINGS.



5/11/23

PAGE 21 OF 24

I18 EAVE STRUT TO ENDWALL RAFTER
LOW EAVE

*MATCHED TO JAMB DEPTH

K1 WALL GIRT TO DOOR JAMB

I17 EAVE STRUT TO ENDWALL RAFTER
HIGH EAVE

L4 DOOR JAMB TO EAVE STRUT

*MATCHED TO JAMB DEPTH

J1 EAVE STRUT TO RIGID FRAME
FLUSH GIRT CONDITION

*MATCHED TO JAMB DEPTH

L9 DOOR JAMB TO WALL GIRT

*MATCHED TO JAMB DEPTH

J7 EAVE STRUT TO RIGID FRAME
FLUSH GIRT CONDITION
HIGH EAVE

*MATCHED TO JAMB DEPTH

M1 DOOR HEADER TO DOOR JAMB

7630 MEADOW RUN LANE

100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"

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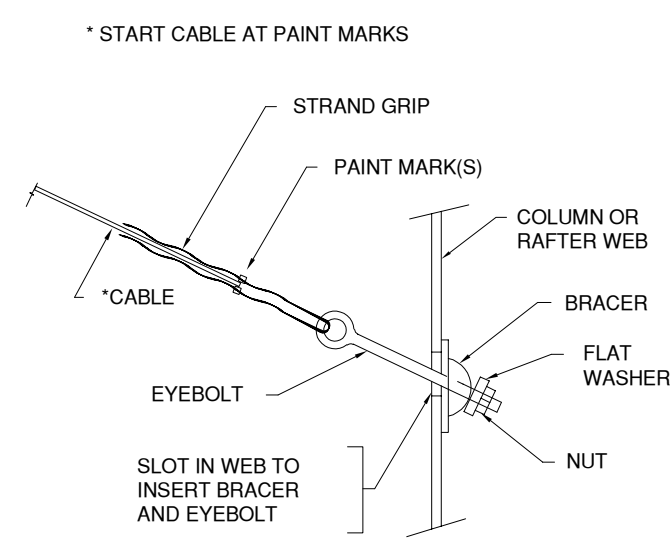
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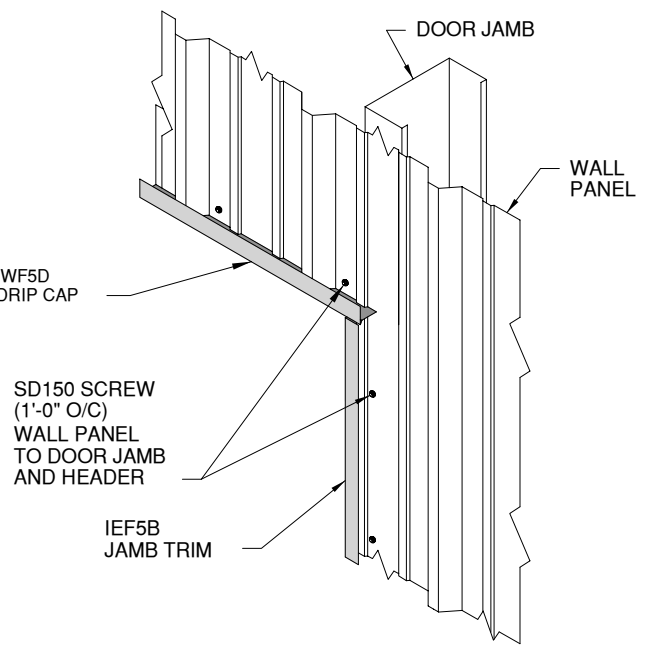
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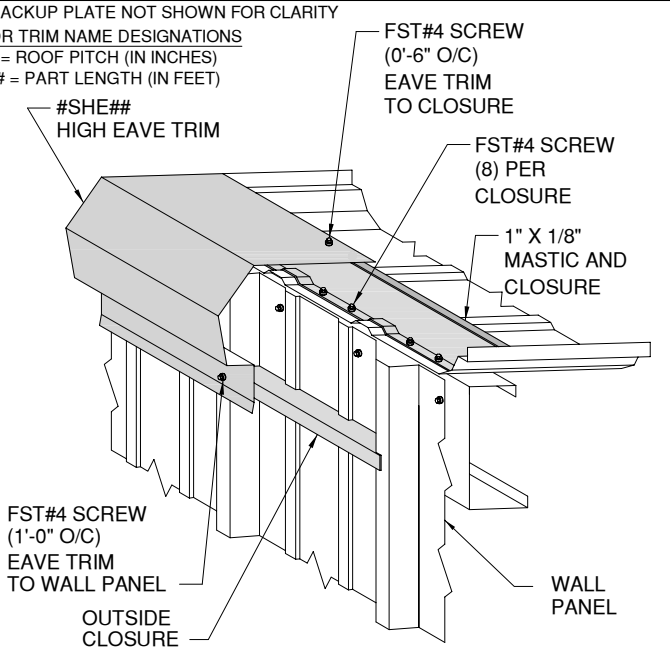
Q2



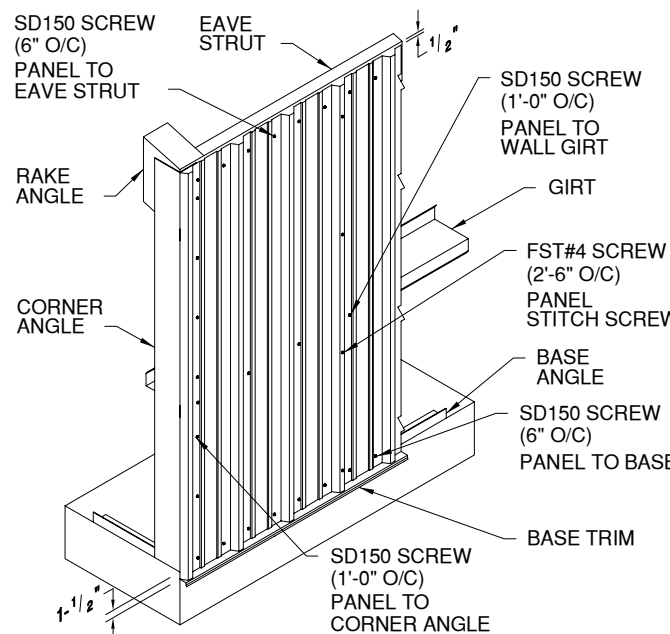
CABLE BRACE / EYEBOLT



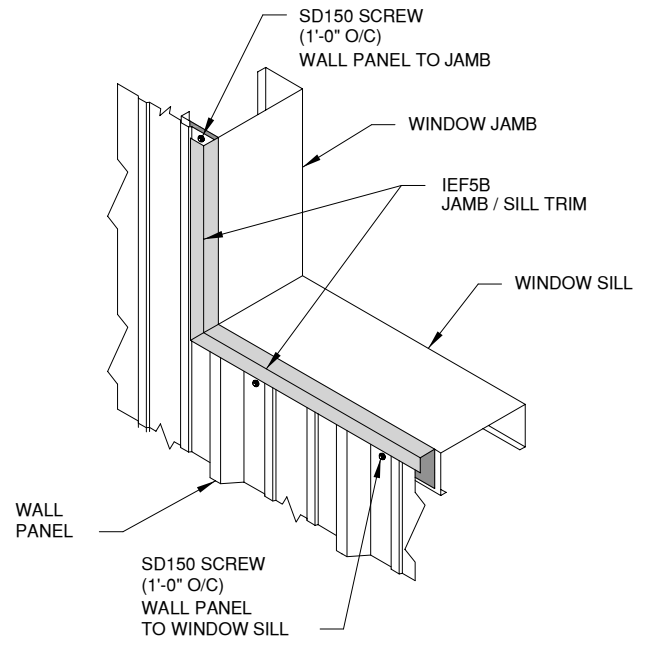
DOOR & WINDOW JAMB / HEADER TRIM



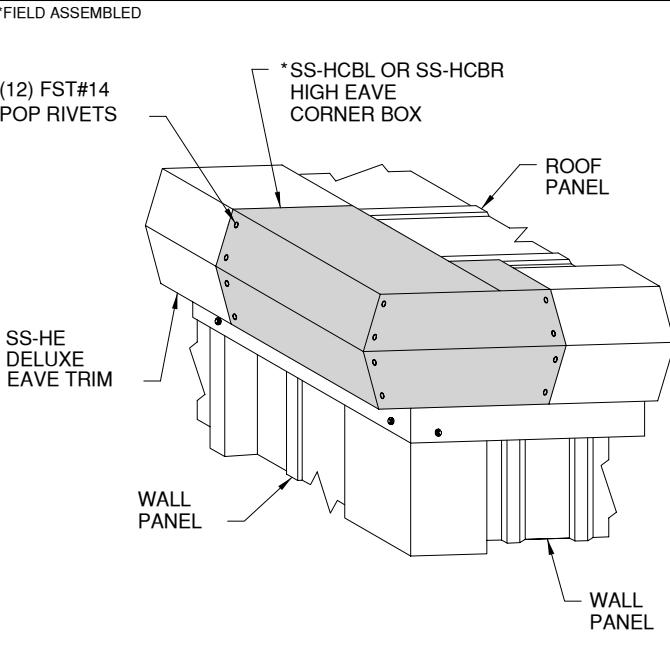
STANDARD HIGH EAVE TRIM



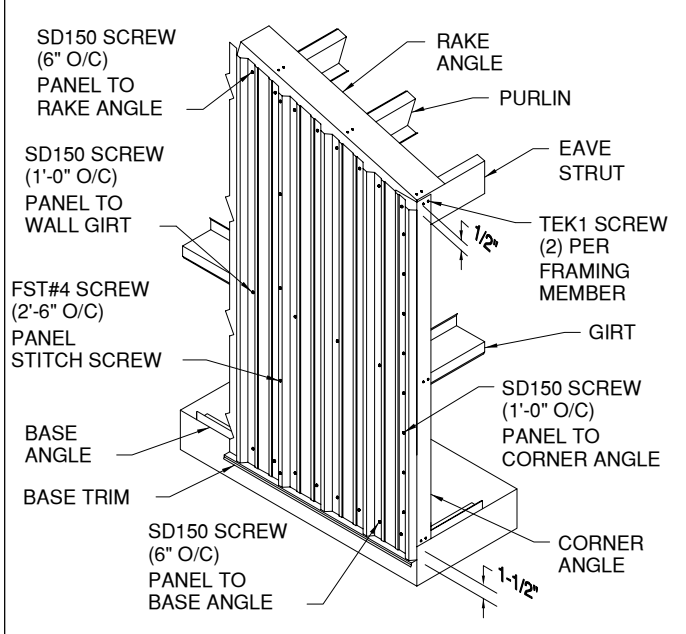
"R" PANEL SIDEWALL



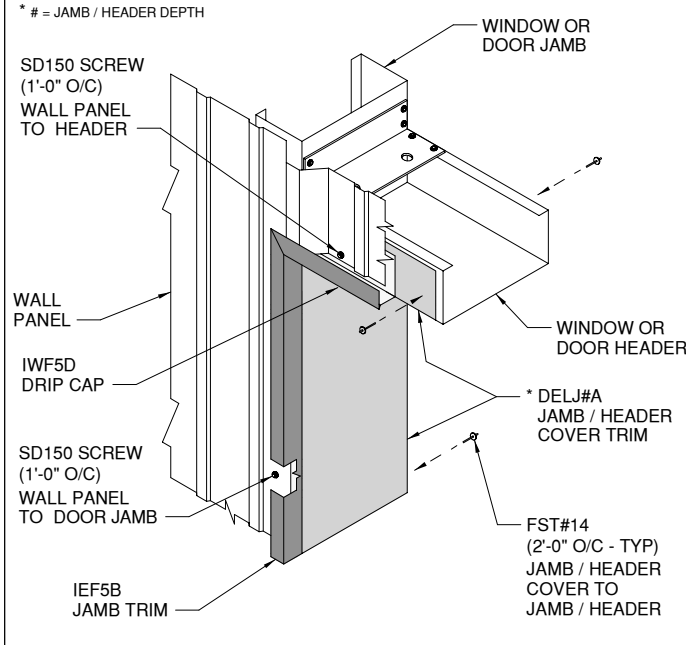
WINDOW JAMB / SILL TRIM



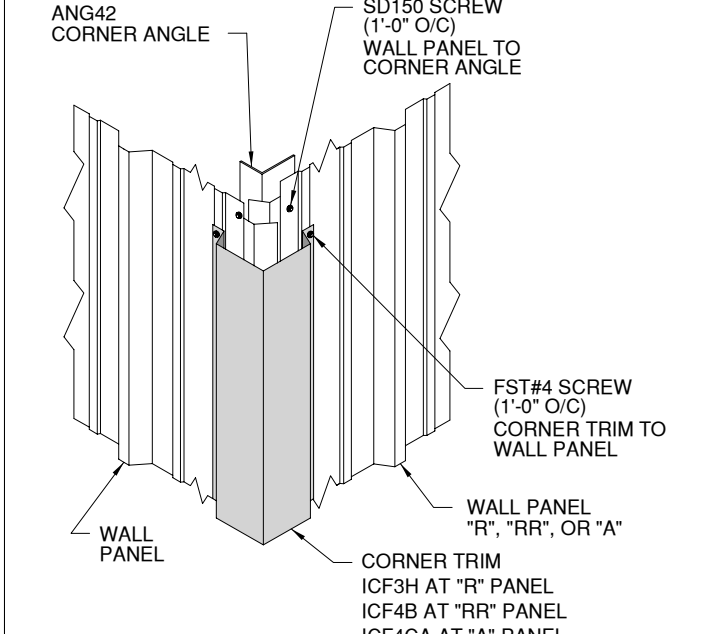
HIGH EAVE CORNER BOX



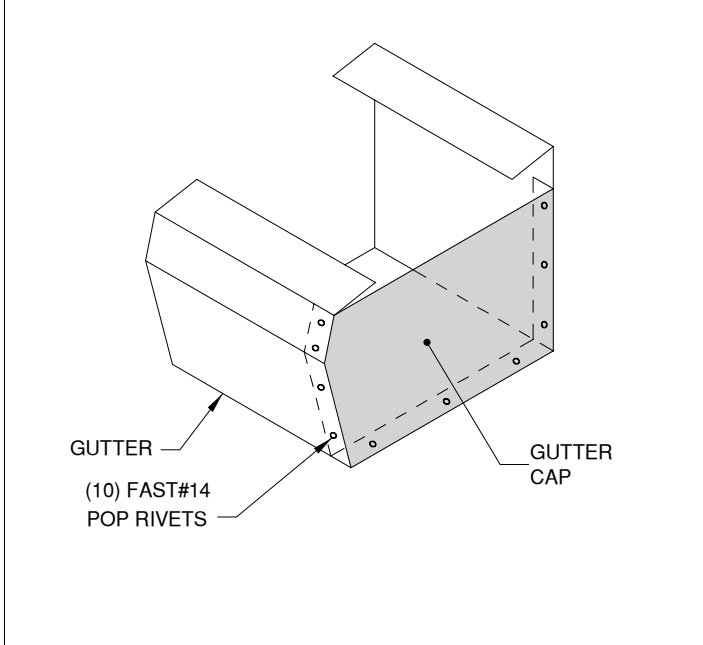
"R" PANEL ENDWALL



DOOR & WINDOW DELUXE JAMB TRIM



CORNER TRIM



GUTTER CAP INSTALLATION



7630 MEADOW RUN LANE
100'-0" x 170'-0" x 26'-4 1/2" x 29'-6"
DATE: 3/31/23
REVISION: 0
ENG: IRM
DWN: BJC
APPD: IRM

F.O. 27081

7630 MEADOW RUN LANE

DRAWING STATUS		REVISION HISTORY	
CITY: OWINGS	ST / PV: MD	DATE	REV.
DESCRIPTION		DESCRIPTION	
FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.		FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.	
FOR CONSTRUCTION: FINAL DRAWINGS.			

