

INBAR
ARCHITECTURE, AIA

FLORIDA LICENSE NO. AA0002701

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ENGINEER'S RECORD
RICHARD D. WILSON
FL P.E. # 37784

REVISIONS

Δ PERMIT RESPONSE 9-7-07

OWNER APPROVALS

01 DESIGN STAGE
02 DD STAGE
03 CD STAGE

PROPOSED NEW HOME FOR:

NOKOMIS MEDICAL CENTER

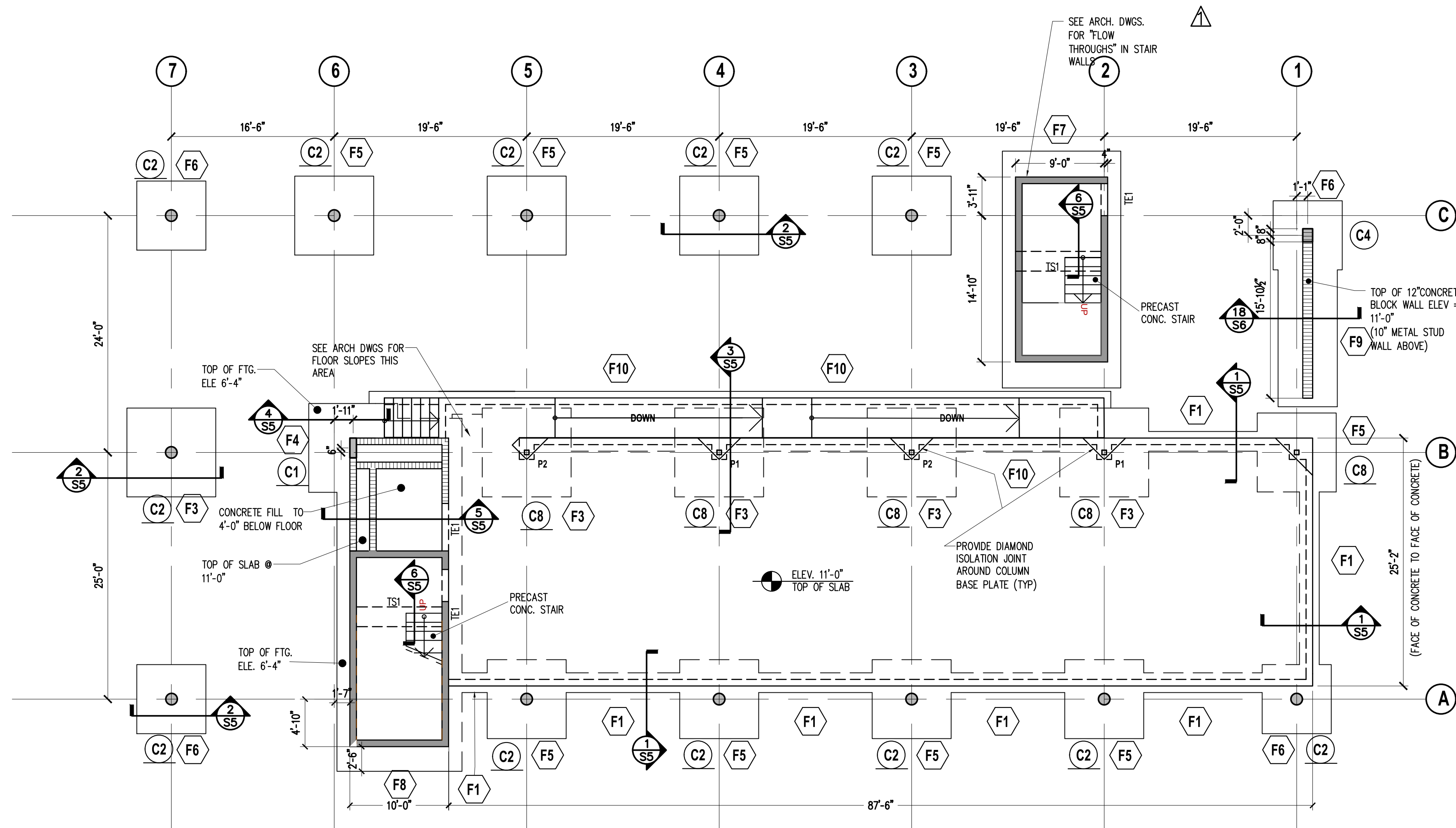
498 S. TAMiami TrL.
NOKOMIS, FL.

PERMIT #

FOUNDATION
PLAN

Drawing Name: NOKOMIS OFFICE
Design: RDW
Issues: 06/16/2022
Job No.:
Sheet Number:

S1



STAIR TOWER (SHEARWALL) REINFORCING
FOOTING TO 2ND FLOOR: 8" CONCRETE W/ #6 VERTL @ 12" O.C. & #5 HORIZ'L @ 12" O.C.
2ND FLOOR TO 3RD FLOOR: 8" CONCRETE W/ #5 @ 12" O.C. EA. WAY
3RD FLOOR TO TOP: 8" BLOCK #5 VERTICAL @ 24" O.C.

AT 8" CONCRETE WALL AT STAIRS PROVIDE 4" X 18" X 12" RECESS FOR FIRE EXTINGUISHER CABINET. SEE ARCH. DWGS. FOR LOCATIONS. PROVIDE (2) BARS EACH SIDE OF RECESS (SIZES NOTED ABOVE). AT RECESS PROVIDE #4 X 4'-0" @ 6" O.C. EACH WAY CENTERED IN 4" REMAINING THICKNESS.

FOOTING SCHEDULE					
	FOOTING SIZE				
FOOTING MARK	WIDTH	LENGTH	DEPTH	REINFORCING	REMARKS
F1	2'-0"	CONT.	12"	(3) #5 CONT. B.	
F2	2'-0"	CONT.	12"		OUTSIDE FACE OF WALL 4" FROM OUTSIDE FACE OF FTG.
F3	9'-0"	9'-0"	1'-6"	(10) #9 EA. WAY B.	
F4	9'-0"	9'-0"	2'-0"	#8 @ 12" O.C. EA. WAY B.	
F5	8'-0"	8'-0"	1'-6"	(9) #8 EA. WAY B.	(EDGE FTG.)
F6	7'-0"	7'-0"	1'-6"	(8) #7 EA. WAY B.	(CORNER FTG.)
F7	12'-0"	24'-0"	2'-0"	#8 @ 12" O.C. EA. WAY TAB.	TOP OF FTG. 16" BELOW GRADE
F8	12'-0"	35'-0"	2'-0"	#8 @ 12" O.C. EA. WAY TAB.	
F9	6'-0"	CONT.	1'-0"	SEE SECTION 3/S-6	
F10	6'-0"	CONT.	1'-0"	SEE SECTION 3/S-5	
TS1	2'-0"	CONT.	12"	(3) #5 CONT.	
TE1	8"	CONT.	8"	(1) #5 CONT.	OVER TOP OF WALL @ DOORS

- FOUNDATION PLAN NOTES:**
- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
 - STAIRWELL WALLS (2) FROM THE FOOTING TO THE 2ND FLOOR ARE TO BE 8" CAST-IN-PLACE CONCRETE REINFORCED WITH #6 @ 12" O/C EACH WAY CENTERED IN THE WALL.
 - INDICATES 8" MASONRY WALL. PROVIDE 1# 5 IN GROUTED CELL AT CORNERS, ENDS AND INTERSECTIONS OF WALLS; UNDER POINT LOADS; AND AT 48 " O.C. MAXIMUM SPACING UNLESS NOTED OTHERWISE ON PLAN.
 - FLOOR SLAB IS A 4" MINIMUM THICKNESS CONCRETE SLAB ON GRADE. REINFORCE WITH 6 x 6-W1.4 x W1.4 WWF AT MID DEPTH. SEE ARCHITECTURAL DRAWINGS FOR VAPOR BARRIER.
 - TOP OF FINISHED FLOOR SHALL BE SET TO 11'-0" UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS AND SLOPES
 - CENTERLINES OF WALLS AND COLUMNS SHALL COINCIDE WITH CENTERLINES OF FOOTINGS UNLESS NOTED OTHERWISE.
 - FOR FOOTING SIZE AND REINFORCING SEE SECTIONS AND SCHEDULE.
 - PROVIDE CONTROL JOINTS IN SLAB ON GRADE. CONTROL JOINTS SHALL BE TOOLED OR SAWCUT. CONTROL JOINT PATTERN SHALL BE APPROXIMATELY SQUARE AND LIMITED TO AN AREA NOT TO EXCEED 225 SF.
 - CONTRACTOR SHALL BACKFILL SIMULTANEOUSLY ON EACH SIDE OF FOUNDATION WALLS.
 - FOUNDATION WALLS WHICH HAVE UNEVEN BACKFILLS SHALL BE TEMPORARILY BRACED UNTIL FLOOR SLABS ARE POURED AND HAVE REACHED 2000 PSI STRENGTH. 11. SEE DRAWING S-2 FOR COLUMN SCHEDULE.
 - ELEVATION TO OF FOOTING = 6'-0" U.N.O. (CONTRACTOR TO CONFIRM TOP FTG 1'-0" MIN BELOW GRADE)

This drawing has been electronically signed and sealed by Richard D. Wilson, P.E. on 06/16/2022 using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on an electronic device.

COLUMN SYMBOLS

○ - INDICATES COLUMN BELOW

○ - INDICATES COLUMN THRU

○ - INDICATES COLUMN ABOVE

STEEL SYMBOLS

BAR SIZE / 12 T

BAR SPACING

BAR PLACEMENT

T=TOP
B=BOTTOM
E=



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REVISIONS
PERMIT RESPONSE 9-7-07

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498 S. TAMiami TrL.
NOKOMIS, FL.

PERMIT #

2ND FLOOR
FRAMING
PLAN

Drawing Name: NOKOMIS OFFICE

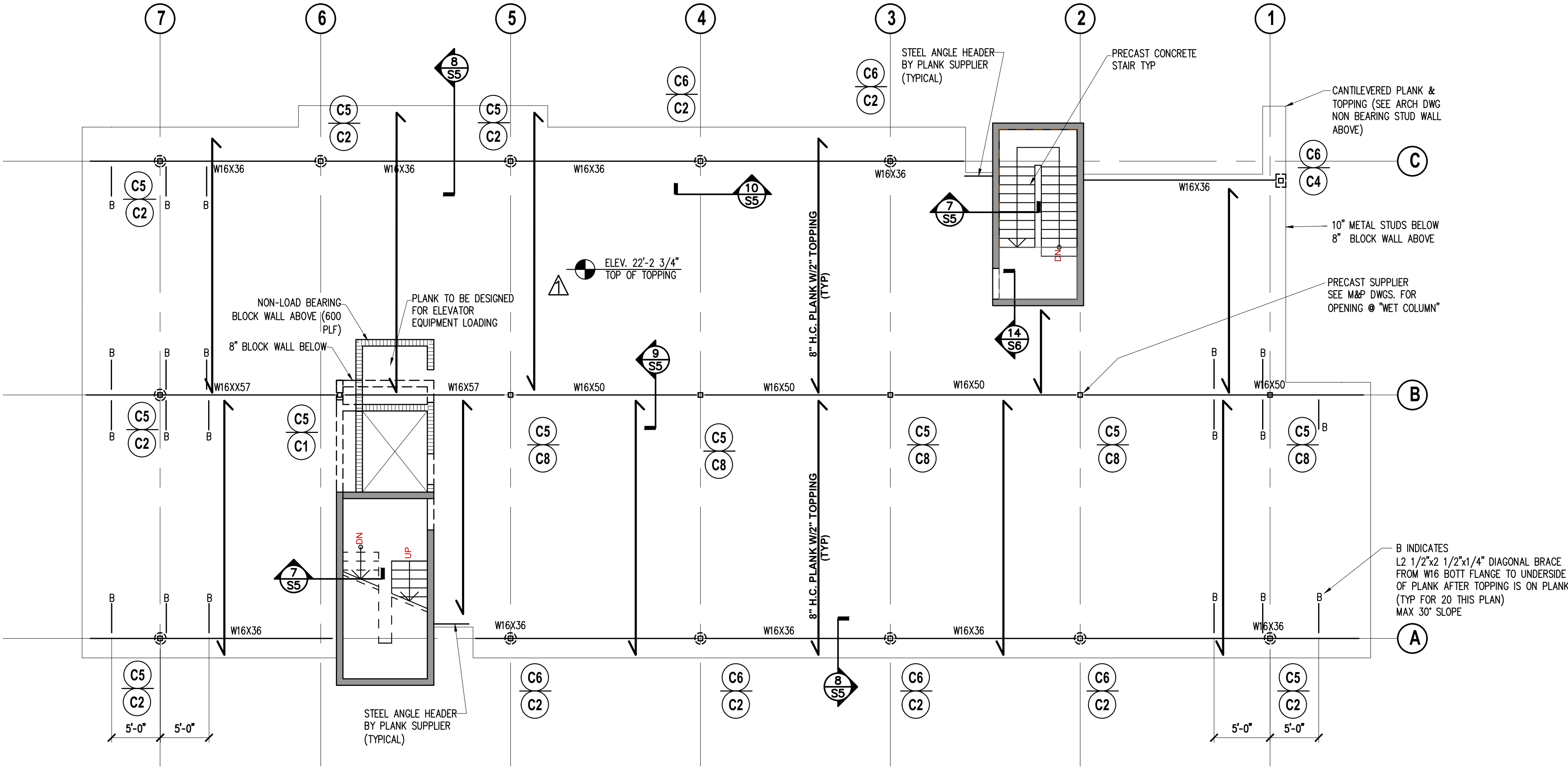
Design: RDW

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S2



STEEL FABRICATOR NOTE:
PROVIDE HSS 4" X 3" X 1/4" TUBES BETWEEN WINDOWS @ 7'-9"± ON CENTER (AS SHOWN ON ARCH. DWGS.). TUBES TO HAVE 5" X 10" X 3/8" CAP & BASE PLATE W/ (2) 3/8" EPOXY ANCHOR W/ 4" EMBEDMENT INTO THE CONCRETE PLANK/TOPPING. THE TUBES SHOULD BE INSTALLED AFTER THE 2ND FLOOR & 3RD FLOOR PLANK & TOPPING ARE INSTALLED. UP TO 1/4" SHIM PLATES MAY BE USED TOP & BOTTOM.

COLUMN SCHEDULE

MARK #	COLUMN MATERIAL	SIZE	REINFORCING	COMMENTS
C1	CONCRETE	8 X 24	(6) # 5 W/ # 3 TIES @ 8"	-
C2	CONCRETE	14" Ø	6 # 8 VERTICAL & # 3 TIES @ 14"	TIES @ 5" CTRS FOR LOWER 6' OF COL.
C3	NOT USED	-	-	-
C4	CONCRETE	12 X 16	(6) # 6 & # 3 TIES @ 12"	-
C5	STEEL	HSS 6x6x1/2	-	-
C6	STEEL	HSS 6x6x3/8	-	-
C7	STEEL	HSS 5x5x3/8	-	-
C8	STEEL	HSS 6x6x9/16	-	-

PRECAST SUPPLIER:
SEE M&P DRAWINGS FOR PIPE/DUCT CHASE LOCATIONS
GC TO COORDINATE PRIOR TO SHOP DRAWING SUBMISSION

SECOND FLOOR FRAMING PLAN

SCALE 1/8" = 1'-0"

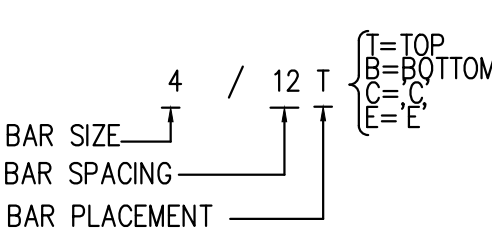
SECOND FLOOR FRAMING PLAN NOTES:

- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
- STAIRWELL WALLS (2) FROM THE 2ND FLOOR TO THE 3RD FLOOR ARE TO BE 8" CAST-IN-PLACE CONCRETE REINFORCED WITH #5 @ 12" O/C EACH WAY CENTERED IN THE WALL.
- INDICATES 8" MASONRY WALL. PROVIDE 1# 5 IN GROUTED CELL AT CORNERS, ENDS AND INTERSECTIONS OF WALLS; UNDER POINT LOADS; AND AT 48" O.C. MAXIMUM SPACING UNLESS NOTED OTHERWISE ON PLAN.
- FLOOR FRAMING SHALL CONSIST OF 8" PRE-ENGINEERED HOLLOW CORE PLANK WITH 2" STRUCTURAL BONDED TOPPING REINFORCED WITH 6x6-W1.4xW1.4 WWF. ALL PLANK SHALL BE GROUTED AND CONNECTED TO THE STRUCTURE AT ALL FLOOR EDGES AS DETAILED IN THE PLANS. SEE SECTIONS FOR REQUIRED EMBED PLATES FOR WELDING TO STEEL SUPPORTS.
- TOP OF TOPPING SHALL BE SET TO 23'-2 3/4" UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS AND SLOPES.
- SEE DRAWING S-2 FOR COLUMN SCHEDULE.

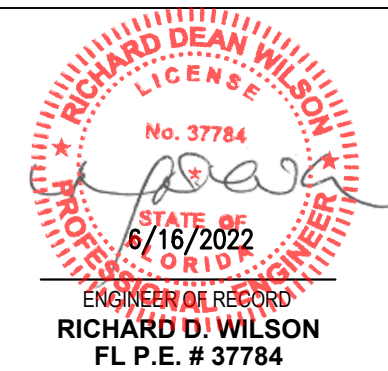
COLUMN SYMBOLS

- - INDICATES COLUMN BELOW
- - INDICATES COLUMN THRU
- - INDICATES COLUMN ABOVE

STEEL SYMBOLS



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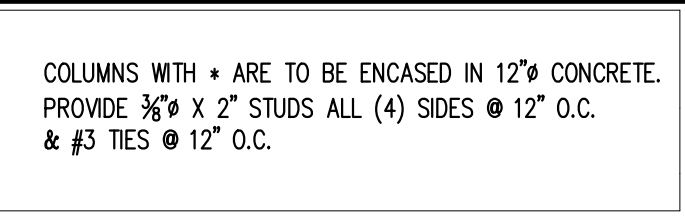
PERMIT RESPONSE 9-7-07

01	DESIGN STAGE
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498 S. TAMiami TRl.
NOKOMIS, FL.




3RD FLOOR FRAMING PLAN

S3



SCALE 1/8" = 1'-0"

1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
2. STAIRWELL WALLS FROM THE 3RD FLOOR TO THE ROOF ARE TO BE 8" CONCRETE BLOCK REINFORCED WITH #5 VERTICAL REINFORCING AT 24" CENTERS AND #5 HORIZONTAL REINFORCING (IN KNOCKOUT BLOCKS) @ 32" O/C.
3. XXXXXXXXXX INDICATES 8" MASONRY WALL. PROVIDE 1" #5 IN GROUTED CELL CORNERS, ENDS AND INTERSECTIONS OF WALLS; UNDATED PLOT LOADS; AND AT 48" O.C. MAXIMUM SPACING UNLESS NOTED OTHERWISE ON PLAN.
4. FLOOR FRAMING SHALL CONSIST OF 8" PRE-ENGINEERED HOLLOW CORE PLANK WITH 2" STRUCTURAL BONDED TOPPING REINFORCED WITH 6x6=W1.4xW1.4 WWF. ALL PLANK SHALL BE GROUTED AND CONNECTED TO THE STRUCTURE AT ALL FLOOR EDGES AS DETAILED IN THE PLANS. SEE SECTIONS FOR REQUIRED EMBED PLATES FOR WELDING TO STEEL SUPPORTS.
5. TOP OF TOPPING SHALL BE SET TO 33'-5 1/2" UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR SLAB DEPRESSIONS AND SLOPES.
6. SEE DRAWING S-2 FOR COLUMN SCHEDULE.

 - INDICATES COLUMN BELOW
 - INDICATES COLUMN THRU
 - INDICATES COLUMN ABOVE

BAR SIZE — 4 / 12 T
 BAR SPACING —
 BAR PLACEMENT —

$\begin{cases} T = \text{TOP} \\ B = \text{BOTTOM} \\ C = \text{C} \\ E = \text{E} \end{cases}$





REVISIONS
PERMIT RESPONSE 9-7-07

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PROPOSED NEW HOME FOR:

NOKOMIS MEDICAL CENTER

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NOKOMIS, FL.

PERMIT #

ROOF FRAMING
PLAN

Drawing Name: NOKOMIS OFFICE

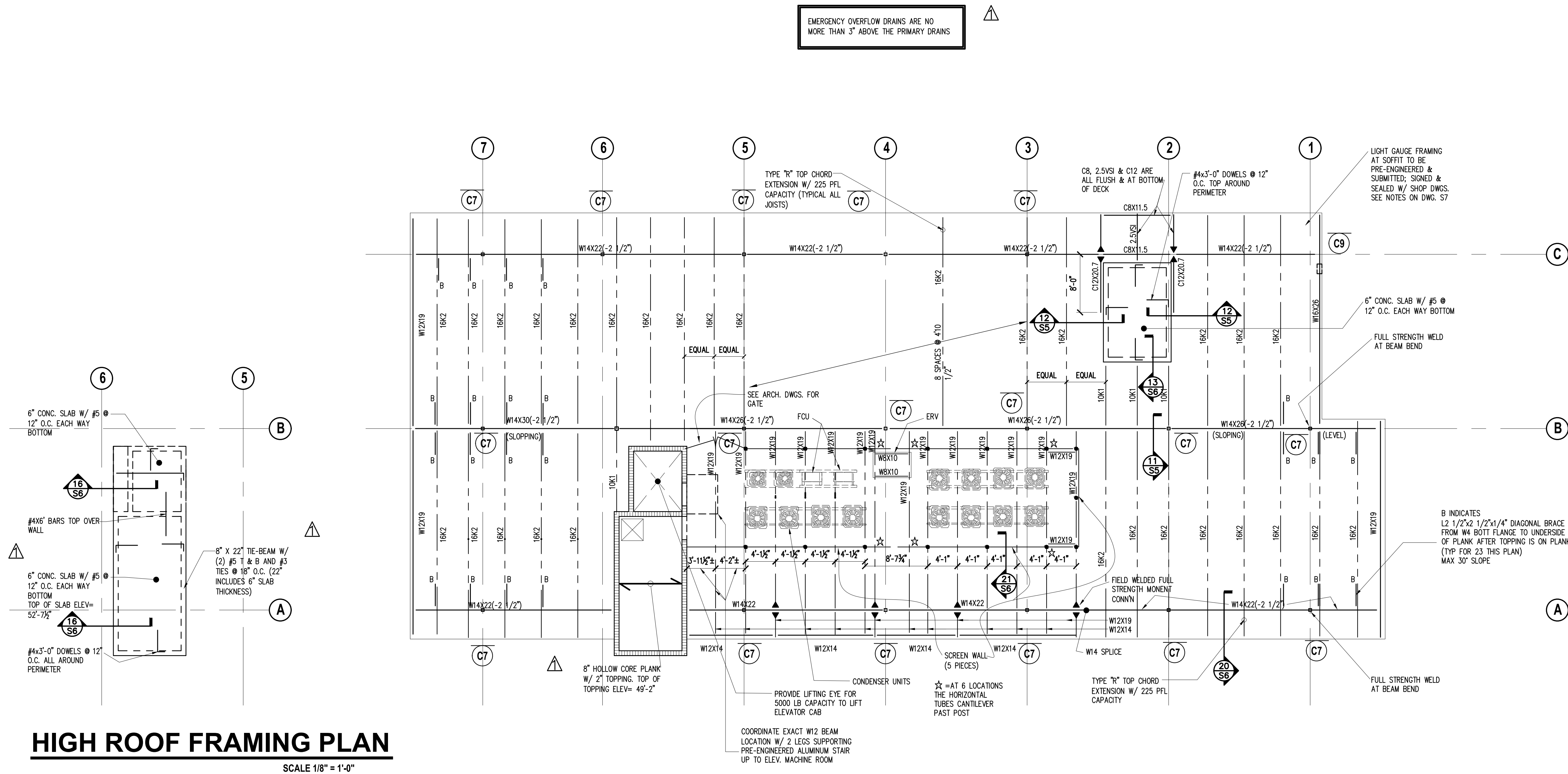
Design: RDW

Issues: 06/16/2022

Job No.:

Sheet Number:

S4



HIGH ROOF FRAMING PLAN

SCALE 1/8" = 1'-0"

ROOF FRAMING PLAN

SCALE 1/8" = 1'-0"

ROOF FRAMING PLAN NOTES.

1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS AND THE MEP PLANS.
2. INDICATES 8" MASONRY WALL. PROVIDE 1# 5 IN GROUTED CELL AT CORNERS, ENDS AND INTERSECTIONS OF WALLS; UNDER POINT LOADS; AND AT 48" O.C. MAXIMUM SPACING UNLESS NOTED OTHERWISE ON PLAN.
3. THE ROOF FRAMING SYSTEM SHALL CONSIST OF STEEL BAR JOISTS AT THE SPACING INDICATED ON THE PLANS SUPPORTING 1-1/2" DEEP, WIDE RIB 22 GAGE GALVANIZED METAL DECK. THE DECK SHALL BE ATTACHED WITH 5/8" PUDDLE WELDS AT IN A 36/4 PATTERN AND TWO SIDELAP #10 TEK SCREWS. ALL EDGES OF DECK ARE TO BE SUPPORTED.
4. THE ROOF DECK IS TO BE INSTALLED PER THE MANUFACTURERS RECOMMENDATIONS AND THE STEEL DECK INSTITUTE (SDI).
5. JOIST BRIDGING AND ANCHORING SHALL BE PER THE STEEL JOIST INSTITUTE AND SHALL BE INSTALLED SO THAT THE ROOF CAN RESIST THE UPLIFT LOADS NOTED ON THE DRAWINGS.
6. CONCENTRATED LOADS (IF ANY) SHALL BE SPECIFICALLY DESIGNED FOR BY THE JOIST SUPPLIER.
7. SEE THE PLAN FOR TOP OF ROOF DECK ELEVATIONS.
8. TOP OF STEEL ELEVATIONS ARE NOTED ON PLAN RELATIVE TO THE BOTTOM OF DECK ELEVATION AT THAT LOCATION.
9. OPENINGS IN THE DECK SMALLER THAN 8" IN DIAMETER OR 8" SQUARE ARE TO HAVE A 20" SQUARE 1/8" THICK STEEL PLATE (WITH HOLE) WELDED OR SCREWED TO THE TOP OF THE DECK AT THE OPENING.
10. OPENINGS LARGER THAN 8" PROVIDE AN ANGLE 3x3x1/4" FRAME WELDED TO THE ROOF FRAMING.
11. SEE DRAWING S-2 FOR COLUMN SCHEDULE.
12. ERV UNIT SHOWN ABOVE IS 900 LBS.
13. CONDENSING UNITS (14) SHOWN ABOVE ARE 280 LBS EACH.
14. FAN COIL UNITS SHOWN ABOVE ARE 100 LBS EACH.
15. SEE ARCH DWGS FOR PERFORATED SCREEN WALL AROUND CONDENSING UNITS. COORDINATE BEAM LOCATIONS W/ CONDENSING UNIT RACKS & SCREEN WALL POST LOCATION.

SLOPING STEEL NOTES

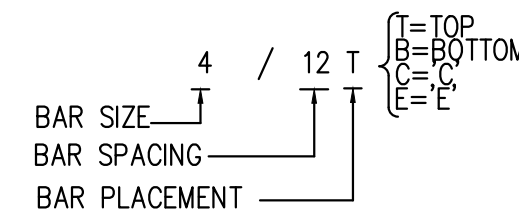
THE ELEVATIONS NOTED BELOW ARE BOTTOM OF ROOF DECK. STEEL BEAMS SUPPORTING JOISTS ARE 2 1/2" LOWER, AS NOTED ON PLANS.

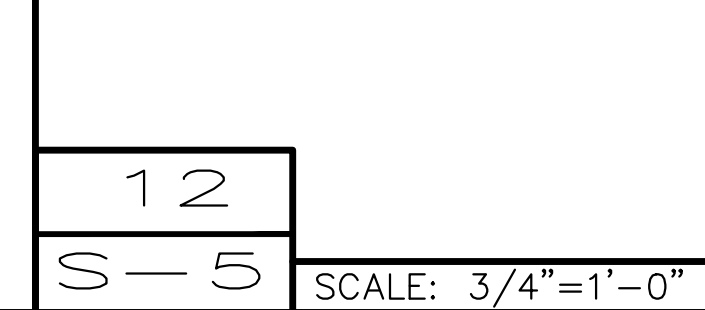
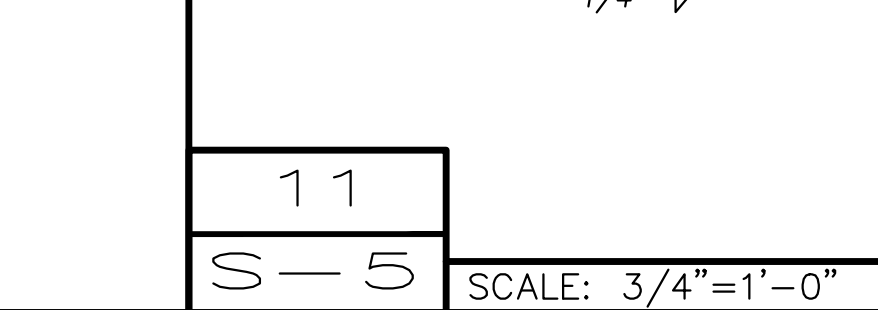
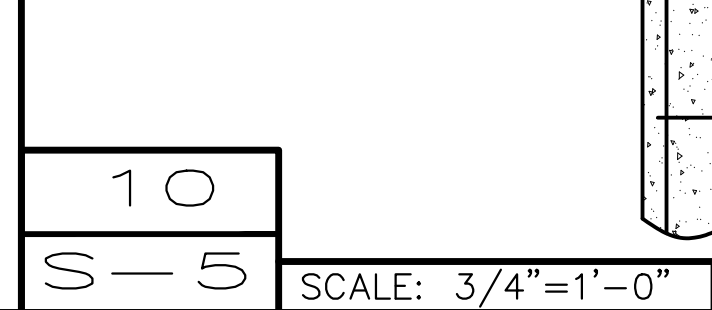
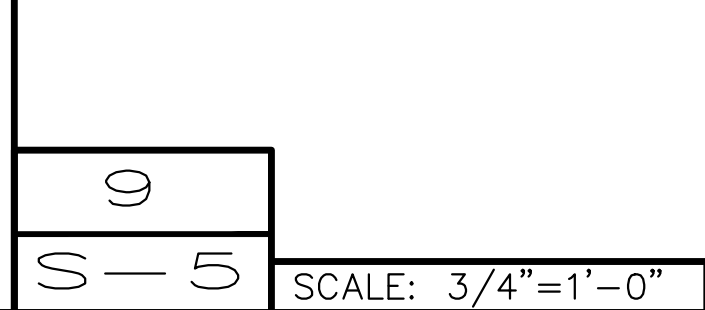
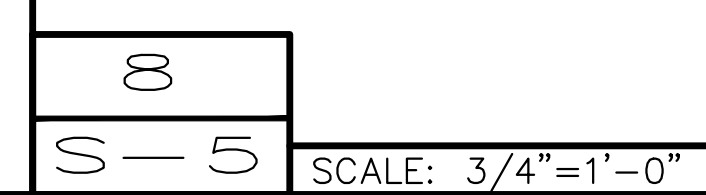
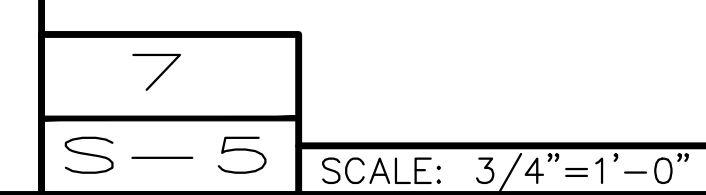
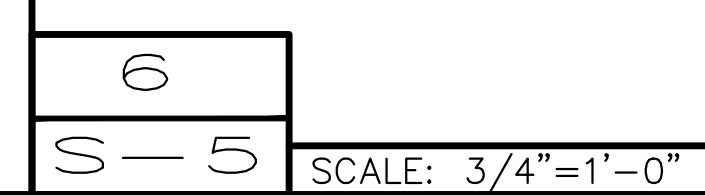
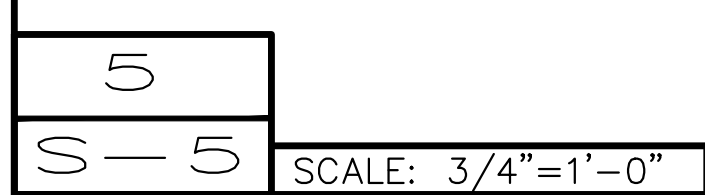
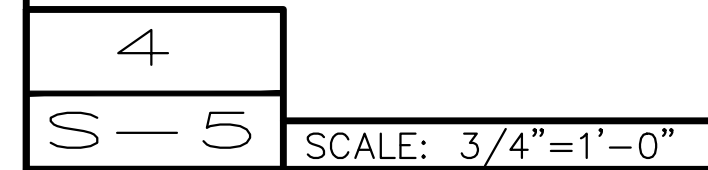
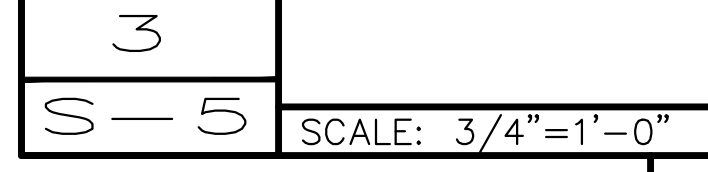
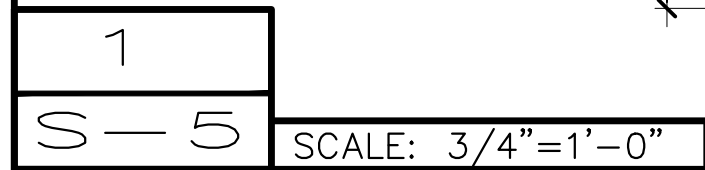
1. THE ENTIRE PERIMETER OF THE BUILDING ELEVATION= 45'-7".
2. GRID A/2 TO A/6 ELEVATION= 45'-6 1/2".
3. GRID A/1 ELEVATION= 45'-7".
4. GRID A/7 ELEVATION= 45'-6 3/4".
5. GRID B/2 TO B/6 ELEVATION= 44'-11 3/4".
6. GRID B/1 ELEVATION= 45'-7".
7. GRID B/7 ELEVATION= 45'-5 3/4".
8. GRID C/2 TO C/6 ELEVATION= 45'-5 3/4".
9. GRID C/1 ELEVATION= 45'-7".
10. GRID C/7 ELEVATION= 45'-6 1/4".

COLUMN SYMBOLS

- INDICATES COLUMN BELOW
INDICATES COLUMN THRU
INDICATES COLUMN ABOVE

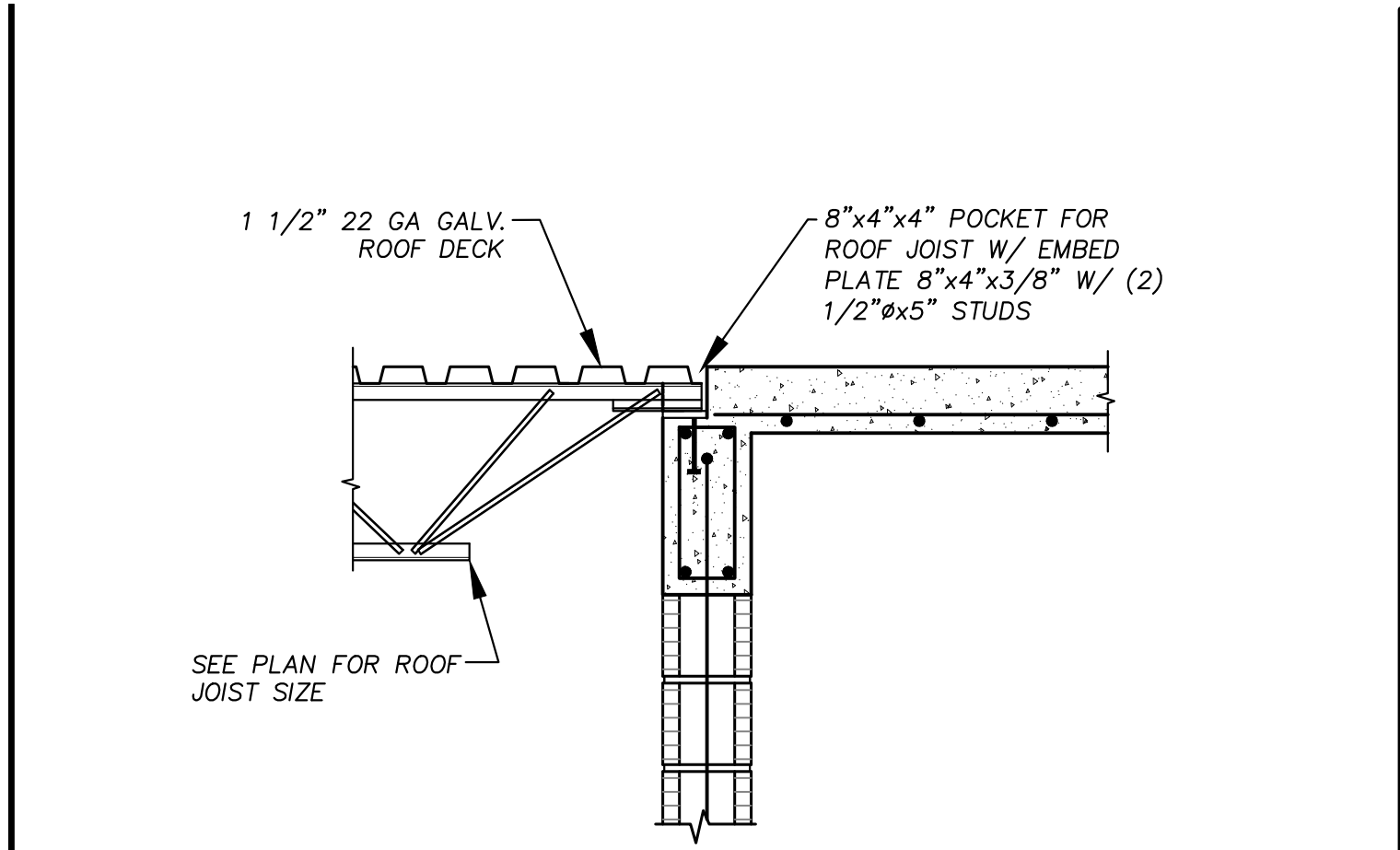
STEEL SYMBOLS



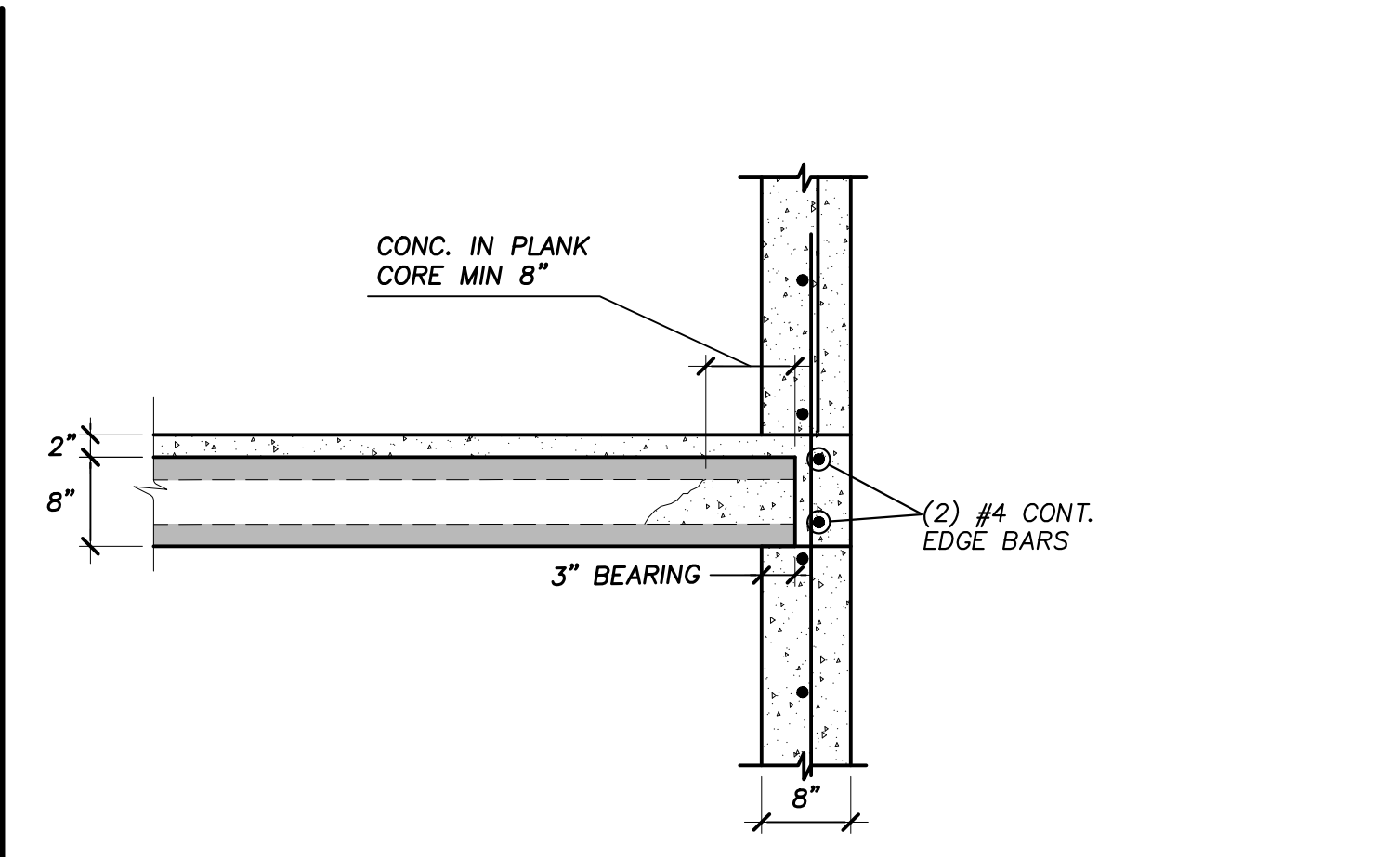


S5

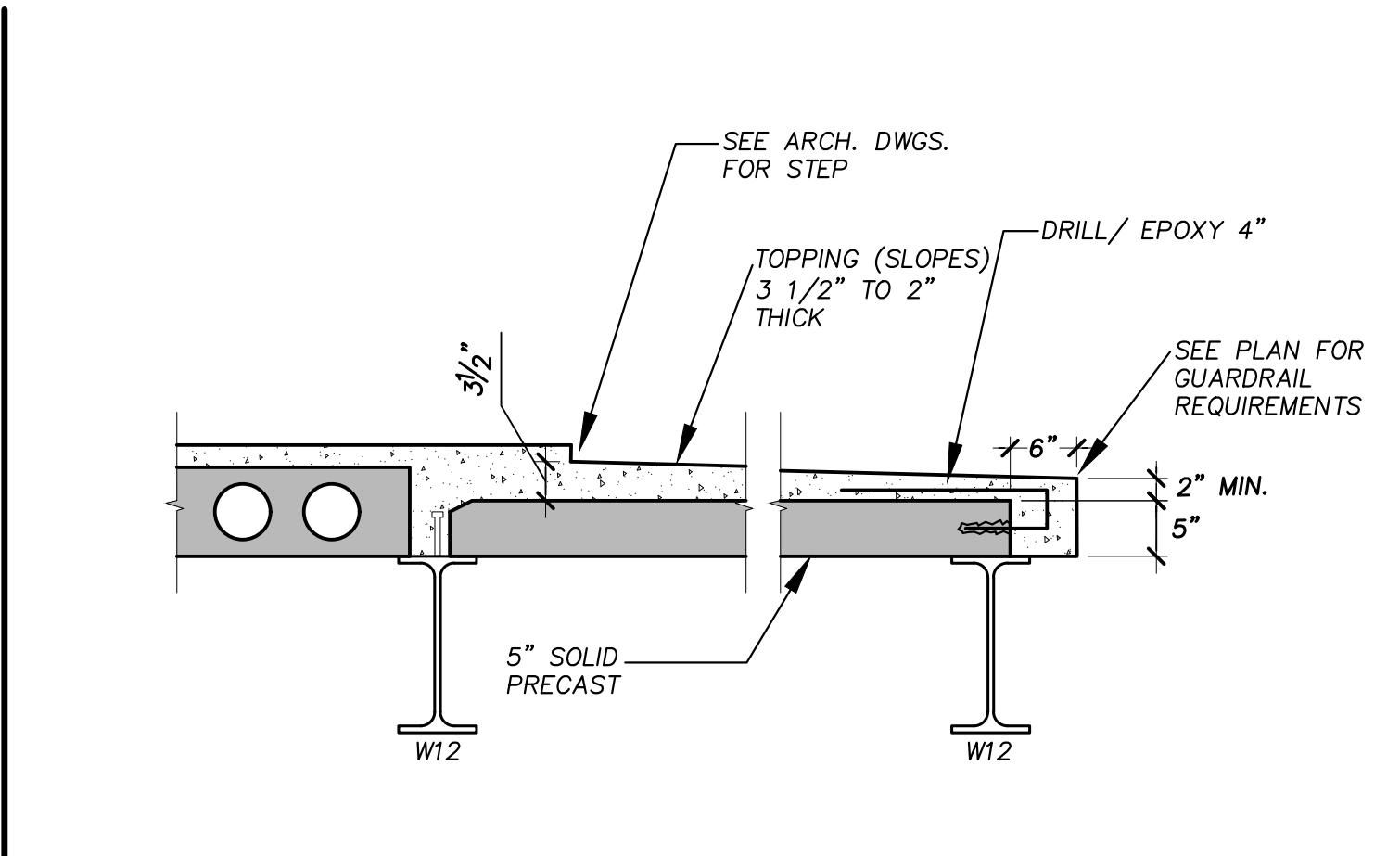
Wilson Structural Consultants, Inc.
 6731 PROFESSIONAL PKWY. WEST (941) 907-4789
 SUITE 100 SARASOTA, FL 34240 FAX (941) 907-0576
 TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED
 STRUCTURE DEPICTED ON THE PLANS COMPLIES
 WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE: 07-001
Cert. of Authorization #9099



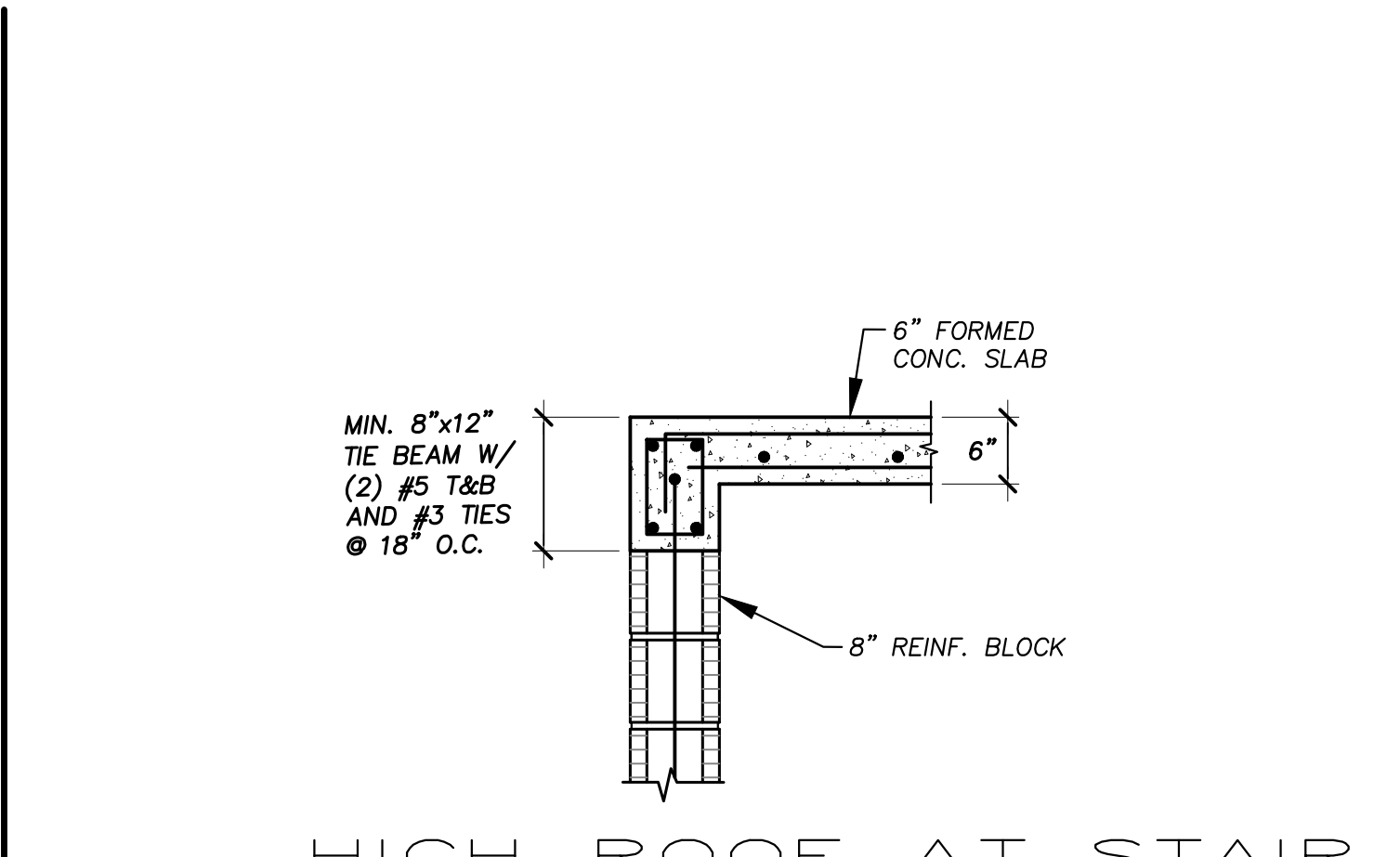
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S-6 SCALE: 3/4"=1'-0"



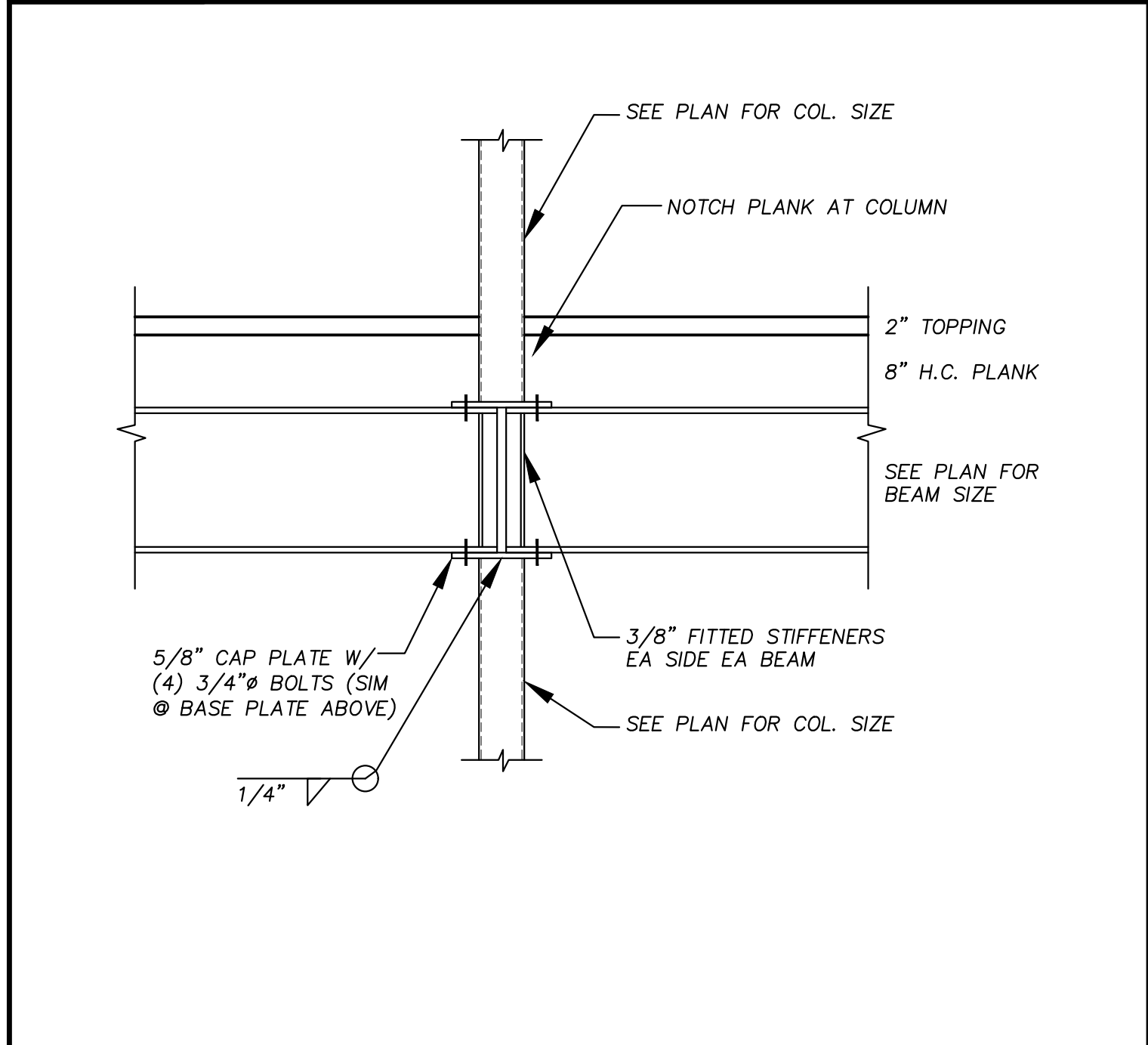
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S-6 SCALE: 3/4"=1'-0"



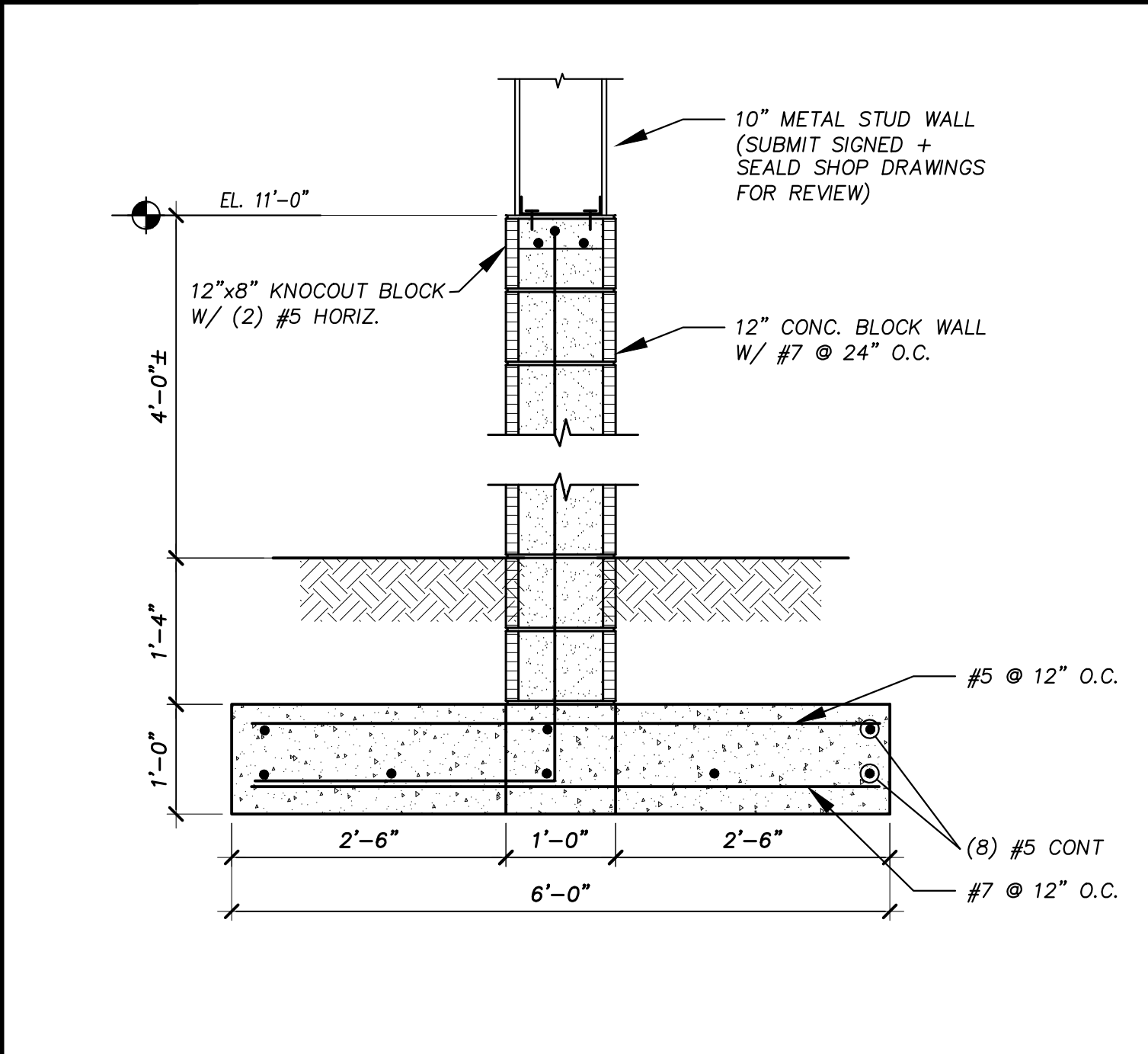
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S-6 SCALE: 3/4"=1'-0"



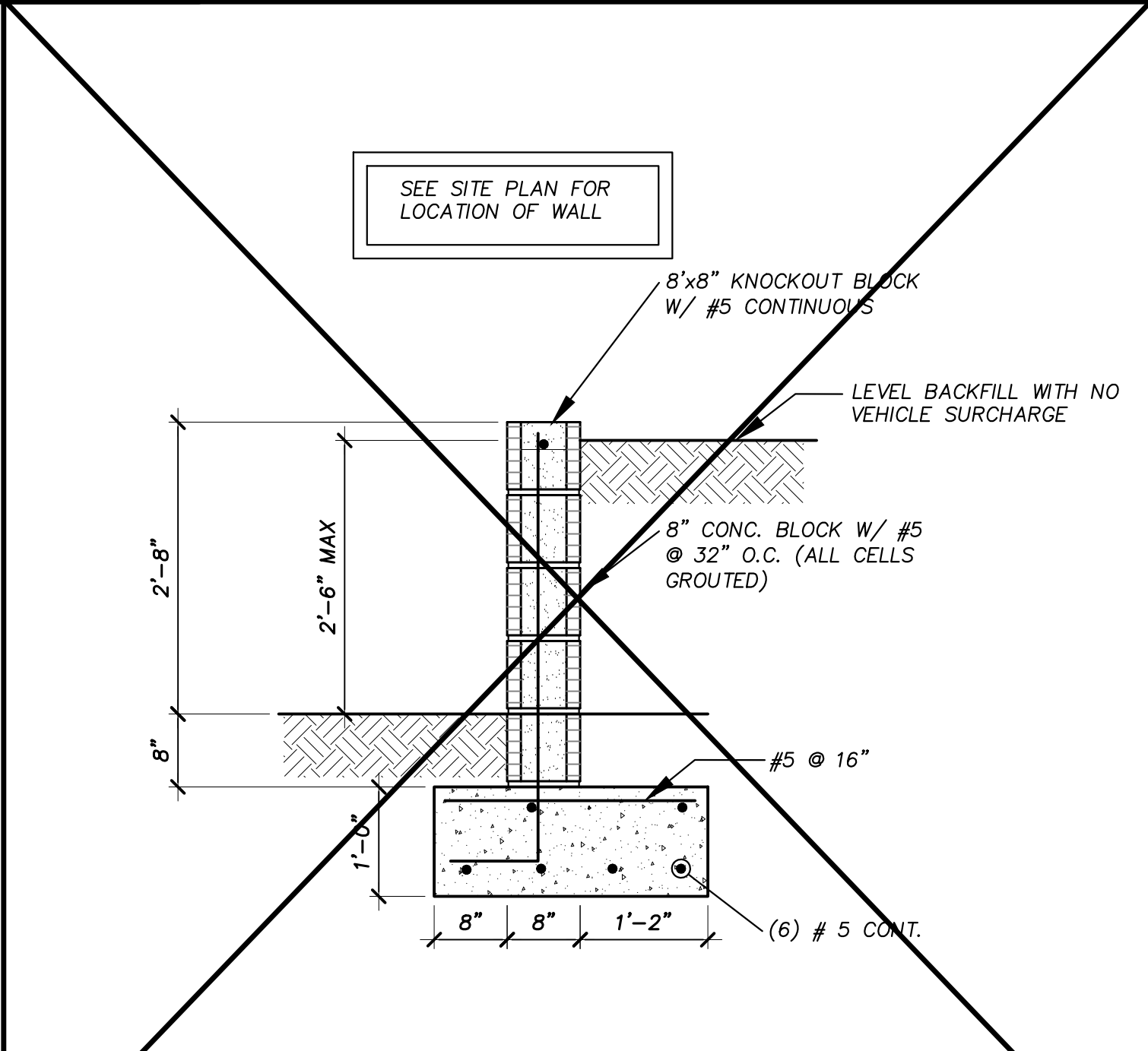
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S-6 SCALE: 3/4"=1'-0"



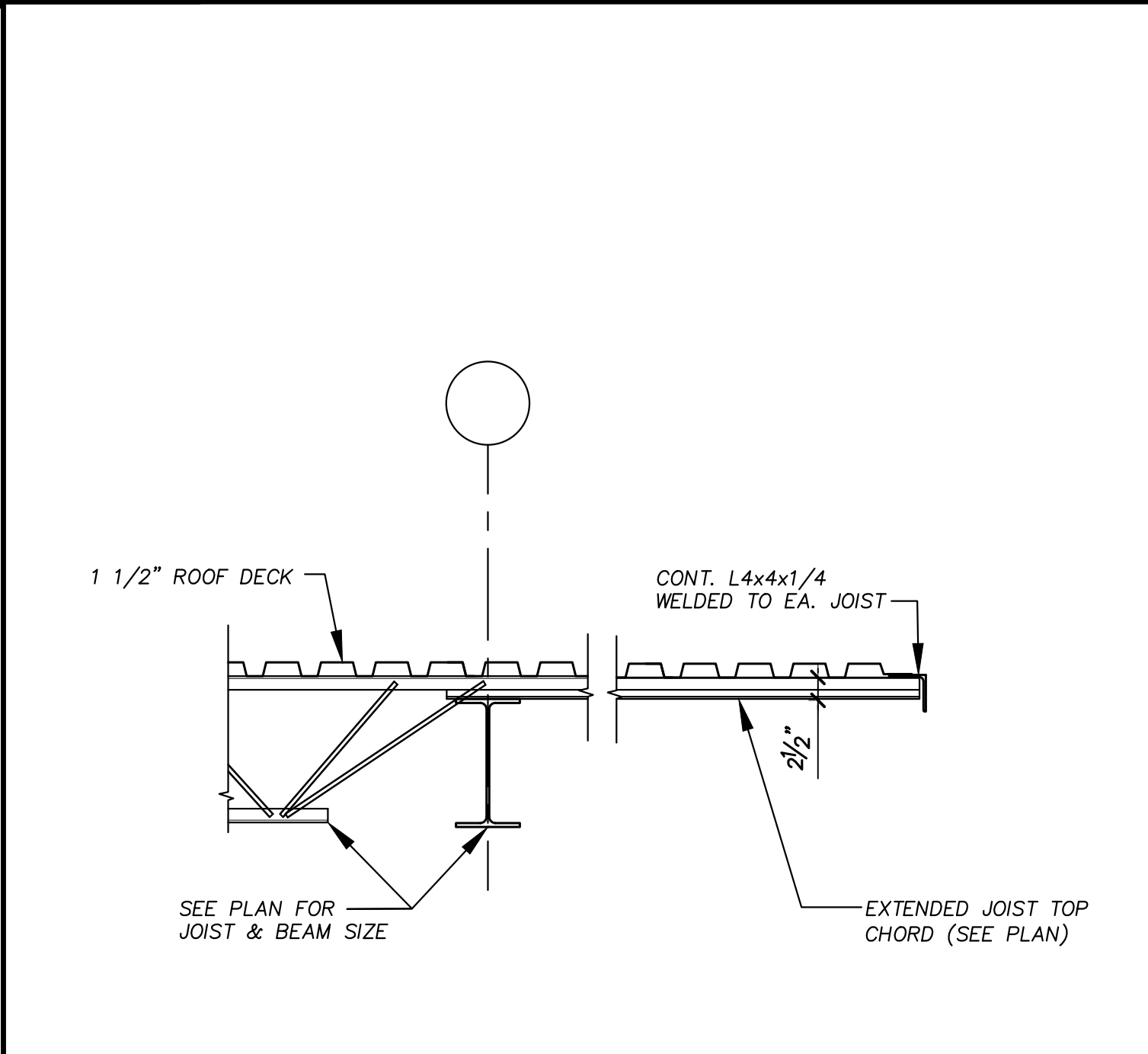
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S-6 SCALE: 3/4"=1'-0"



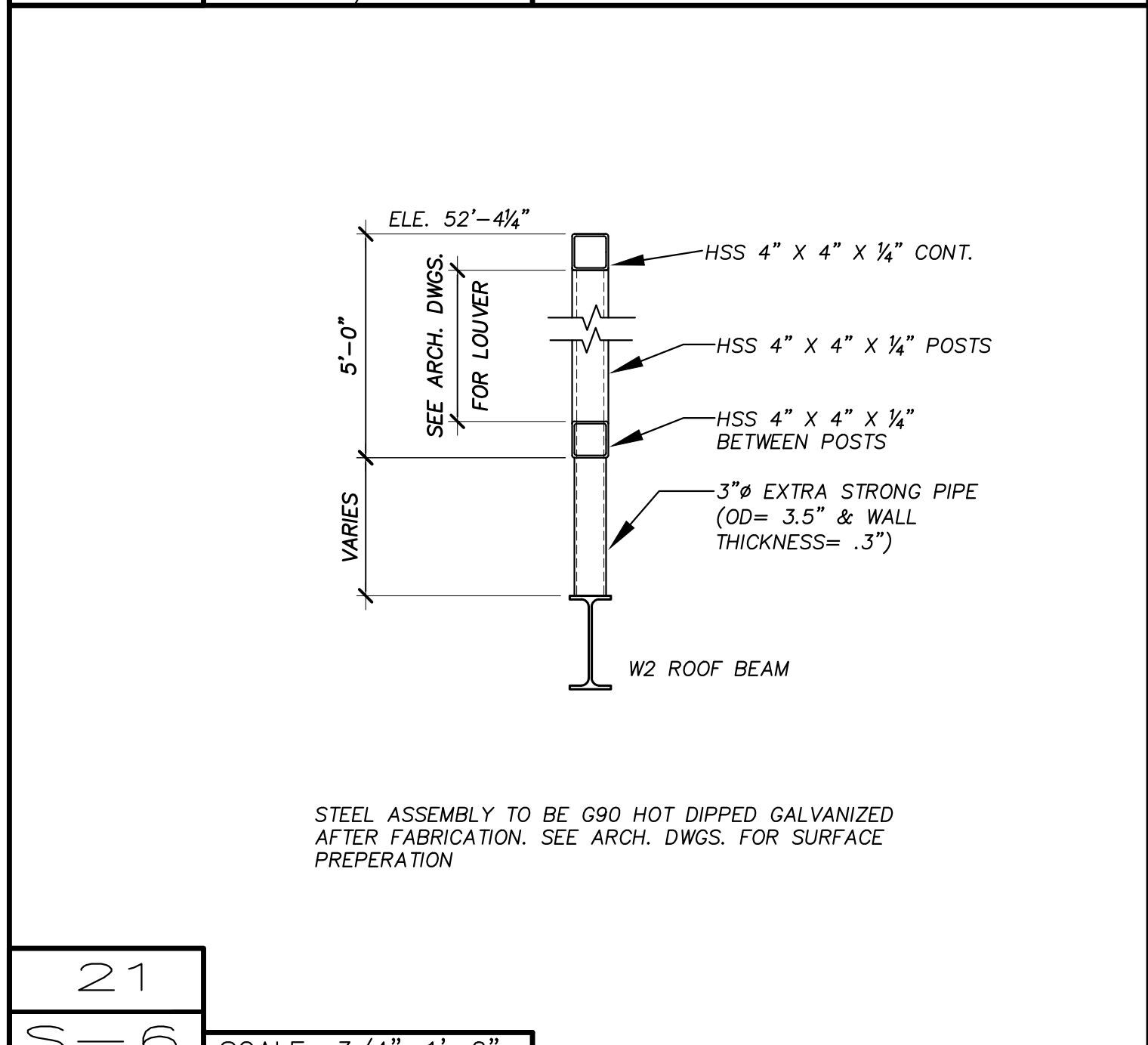
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S-6 SCALE: 3/4"=1'-0"



19
S-6 SCALE: 3/4"=1'-0"



20
S-6 SCALE: 3/4"=1'-0"



21
S-6 SCALE: 3/4"=1'-0"

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STATE OF FLORIDA
6/16/2022
ENGINEER OF RECORD
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SECTIONS

Drawing Name:	NOKOMIS OFFICE
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S6

Wilson Structural Consultants, Inc.
0731 PROFESSIONAL PKWY, WEST SUITE 100 SARASOTA, FL 34234
(941) 907-4789 FAX (941) 907-6578
TO THE BEST OF MY KNOWLEDGE AND ABILITY, THE COMPLETED STRUCTURE DEPICTED ON THE PLANS COMPLIES WITH THE APPLICABLE MINIMUM BUILDING CODES.
FILE: 07-001
Cert. of Authorization #9099

STRUCTURAL NOTES

GENERAL NOTES

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.

THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO INSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

DESIGN LOADS

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE, 7th EDITION (FBC2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF:	LIVE LOAD	-	20 PSF.
	DEAD LOAD	-	25 PSF.
	DEAD LOAD	-	10 PSF. (AVAILABLE TO RESIST UPLIFT)
	MECH EQUIPMENT	-	AS SHOWN ON PLANS
STAIRWAYS:	LIVE LOAD	-	100 PSF.
OFFICE AREAS:	LIVE LOAD	-	50 PSF.
	PARTITION LIVE LOAD	-	20 PSF.
	SUPERIMPOSED DEAD LOAD	-	15 PSF.
	PUBLIC AREAS AND CORRIDORS:	-	100 PSF.
PUBLIC AREAS AND CORRIDORS:	LOBBIES & FIRST FLOOR CORRIDORS	-	100 PSF.
	CORRIDORS ABOVE FIRST FLOOR	-	80 PSF.
	SUPERIMPOSED DEAD LOAD	-	15 PSF.
WIND: ASCE 7-16	ULTIMATE WIND SPEED	-	160 mph
	ALLOWABLE WIND SPEED	-	124 mph
	EXPOSURE B	-	
	ENCLOSED STRUCTURE	-	
	RISK FACTOR II	-	

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED.

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT. THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER.

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO:
CONCRETE MIX DESIGNS
MASONRY BLOCK ACCESSORIES
MASONRY REINFORCING
CONCRETE REINFORCEMENT
STRUCTURAL STEEL (INCLUDING ANCHOR BOLTS)
LIGHT GAUGE METAL FRAMING
PRE-ENGINEERED CONCRETE STAIRS
STEEL BAR JOISTS
METAL ROOF DECK
ANY ALTERNATE MATERIAL/PRODUCT SUBSTITUTIONS

FOUNDATIONS

SEE THE FOLLOWING REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH REPORT No.: 6680
PREPARED BY: UNIVERSAL ENGINEERING SCIENCES
WIND: NOKOMIS OFFICE SUITES

FOUNDATION DESIGN IS BASED ON A SOIL BEARING PRESSURE OF 2,500 PSF.

FORMWORK AND SHORING

NO STRUCTURAL CONCRETE SHALL BE STRIPPED UNTIL IT HAS REACHED AT LEAST TWO-THIRDS OF THE 28 DAY DESIGN STRENGTH. DESIGN, ERECTION AND REMOVAL OF ALL FORMWORK, SHORES AND RESHORES SHALL MEET THE REQUIREMENTS SET FORTH IN ACI STANDARDS 347 AND 301.

PLUMBING SLEEVES

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER.

REINFORCING STEEL

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

WELDED WIRE FABRIC

TO CONFORM TO ASTM A1064, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

CONCRETE

SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 psi FOR FOUNDATIONS AND SLABS ON GRADE.	
5000 psi FOR ALL GROUND FLOOR COLUMNS.	
4000 psi FOR ALL OTHER STRUCTURAL CONCRETE.	

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS.

SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C93 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE-HALF (1 1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE-10 AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED. OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

ALL CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 26 OF ACI 318.14 AND ACI 214R. WATER/CEMENT RATIO FOR CONCRETE AT EXTERIOR BALCONIES SHALL NOT EXCEED 0.40 BY WEIGHT AND HAVE 5.000psi MINIMUM COMPRESSIVE CAPACITY.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
	#5 BARS AND SMALLER	-	1 1/2"

WHERE NOT EXPOSED TO EARTH OR WEATHER:

SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1/2"
	#11 BARS AND SMALLER	-	3/4"
BEAMS AND COLUMNS:	ALL BARS	-	1 1/2"

CONCRETE TESTING: AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST-IN-PLACE CONCRETE:

- ASTM C143 - STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE - MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER.
- ASTM C93 - STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS - A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:
1 AT 7 DAYS
2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER MUST BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

NON-SHRINK GROUT

NON-SHRINK GROUT SHALL BE A HIGH-STRENGTH MORTAR OR GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 psi AT 28 DAYS. THE GROUT IS TO BE NON-METALLIC, NON-CORROSIVE, CEMENT-BASED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM C1107. IT SHALL BOND PERMANENTLY TO A CLEAN METAL BASEPLATE AND CONCRETE SUBSTRATE AND WILL NOT SHRINK IN ITS PLASTIC STATE, AS TESTED IN ACCORDANCE WITH ASTM C827.

HOLLOW CORE SLABS

SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED FOR ALL WORK AND SUBMITTED FOR REVIEW. SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

PRECAST SLAB BOTTOM UNITS:

GENERAL: PROVIDE SHOP DRAWINGS, DESIGN, FABRICATION AND ERECTION OF 2" THICK PRECAST PRESTRESSED SLAB BOTTOM UNITS TO BE PART OF COMPOSITE STRUCTURAL TWO-WAY SLAB SYSTEM. SEE PLANS FOR DESIGN MOMENTS AND MINIMUM REINFORCING.

MATERIALS:

- PORTLAND CEMENT SHALL CONFORM TO ASTM C-150 TYPE I OR TYPE II, EXCEPT HIGH EARLY TYPE III MAY BE SUBSTITUTED.
- AGGREGATES SHALL CONFORM TO ASTM C-33 OR C-330.
- REINFORCING SHALL CONFORM TO ASTM A-615.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.
- STRAND SHALL BE COLD DRAWN STRESS, RELIEVED CLEAN AND FREE FROM CORROSION, HAVING A GUARANTEED MINIMUM TENSILE STRENGTH OF 270 KSI, 3/8" DIAMETER AND CONFORMING TO ASTM A-416.
- TRUSSES SHALL BE RESISTANT WELDED, FABRICATED FROM LOW CARBON AISI WELDABLE STEEL RODS AND FLAT BARS AND HAVE MINIMUM YIELD STRENGTH OF 40 KSI.
- ADDITIVES CONTAINING CHLORIDES SHALL NOT BE USED.

SHOP DRAWINGS: DRAWINGS SHALL SHOW THE GENERAL ARRANGEMENT OF SLAB UNITS IN THE STRUCTURE AND DETAILS OF DEPTH, WIDTH, LENGTH, REINFORCING, PLACEMENT, SHORING, RE-SHORING, ETC. SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

PHYSICAL PARAMETERS:

- TOLERANCES WIDTH +/- 1/8 INCH
LENGTH +/- 1/2 INCH
THICKNESS +/- 1/4 INCH
- TOP SURFACE SHALL BE VIBRATOR FORMED AND SHALL PROVIDE A ROUGH SURFACE FOR THE ADHERENCE OF THE FIELD-PLACED CONCRETE.

TESTING:

- CONCRETE: PROVIDE SLUMP TESTS FOR EACH TRUCK LOAD LOAD IN ACCORDANCE WITH ASTM C-172 AND ASTM C-143. TWO CYLINDERS SHALL BE TAKEN FOR EACH CASTING BED AND COMPRESSIONS TESTS MADE IN ACCORDANCE WITH ASTM C-192, ASTM C-617 AND ASTM C-39.
- STRANDS AND REINFORCING: MILL TEST REPORTS SHOWING COMPLIANCE WITH THE APPLICABLE ASTM SPECIFICATION SHALL BE AVAILABLE UPON REQUEST.

CONCRETE STAIRS:

ENGINEERED PRECAST CONCRETE STAIR SYSTEM AND ALL CONNECTIONS OF SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. SUBMIT SHOP DRAWINGS BEARING THE EMBOSSED SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.

THE CONFIGURATION OF THE STAIR SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS. THE STAIR SYSTEM AND ALL CONNECTIONS (INCLUDING CONNECTIONS TO THE STRUCTURE) SHALL BE DESIGNED FOR ALL APPLICABLE LOADS AS INDICATED BELOW AND IN THE BUILDING CODE WHICHEVER IS GREATER. THE LOADS SHALL BE CLEARLY INDICATED ON ALL SHOP DRAWINGS.

MINIMUM STAIR DESIGN LOADS:

UNIFORM LIVE LOAD	100 psf.
CONCENTRATED LIVE LOAD	300 lbs
ADDITIONAL DEAD LOAD	20 psf.

SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS UTILIZED WITHIN THE STAIR SYSTEM AS WELL AS CONNECTIONS (INCLUDING CONNECTIONS TO THE STRUCTURE) AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HLT-HY-HY 150 MAX-50 OR ENGINEER-APPROVED SUBSTITUTION. INSTALL IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 1900 psi ON THE NET AREA (fm = 1500 psi). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270. GROUT SHALL BE 3000 psi MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-416. PROVIDE HOOKED DOVELS IN FOOTINGS FOR ALL VERTICAL REINFORCING ABOVE. LAP SPICES 48 BAR DIAMETERS.

BLOCK CELLS AS SHOWN ON PLANS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS. SEE PLAN NOTES FOR BAR SIZE AND SPACING. DOVELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED.

PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (OUR, O-WALL OR ENGINEER-APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES, BEGINNING 8" ABOVE FOOTINGS AND FLOOR LEVELS.

GROUT LIFT: AN INCREMENT OF GROUT HEIGHT WITHIN A TOTAL GROUT POUR.
GROUT POUR: THE TOTAL HEIGHT OF MASONRY TO BE GROUTED PRIOR TO ERECTION OF ADDITIONAL MASONRY. A GROUT POUR CONSISTS OF ONE OR MORE GROUT LIFTS. GROUT POURS SHALL SET FOR A MINIMUM OF 4 HOURS BEFORE ANY ADDITIONAL GROUT PLACEMENT.

GROUT SHALL HAVE A SLUMP BETWEEN 8 AND 11 INCHES, EXCEPT SELF-CONSOLIDATING GROUT. JOB-SITE PROPORTIONING OF SELF-CONSOLIDATING GROUT IS NOT PERMITTED.

MASONRY GROUTING REQUIREMENTS:

- FIELD-MIXED GROUT SHALL BE PLACED WITHIN 1-1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE AND BEFORE INITIAL SET.
- GROUT SLUMP REQUIREMENTS:
 - FOR GROUT SLUMP BETWEEN 8 AND 10 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 5 FEET.
 - FOR GROUT SLUMP BETWEEN 10 AND 11 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 12.67 FEET.
 - FOR SELF-CONSOLIDATING GROUT, THE GROUT LIFT HEIGHT SHALL NOT EXCEED THE GROUT POUR HEIGHT (24 FEET MAX).
- GROUT LIFT HEIGHTS EXCEEDING 5 FEET SHALL MEET THE FOLLOWING REQUIREMENTS:
 - MASONRY MORTAR HAS CURED FOR AT LEAST 4 HOURS.
 - GROUT SLUMP IS BETWEEN 10 AND 11 INCHES.
 - NO INTERMEDIATE BOND BEAMS ARE PLACED BETWEEN THE TOP AND BOTTOM OF THE GROUT LIFT HEIGHT.
- EACH GROUT LIFT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION AT THE TIME OF PLACEMENT. CONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
- EACH GROUT LIFT SHALL BE RECONSOLIDATED BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED, AND BEFORE ADDING THE SUBSEQUENT GROUT LIFT. RECONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.
- THE TIME BETWEEN PLACING GROUT LIFTS SHALL NOT EXCEED 1 HOUR.
- THE MAXIMUM POUR HEIGHT IS 24 FEET.
- A GROUT KEY SHALL BE PROVIDED AT THE TOP OF EACH GROUT LIFT AND GROUT POUR. GROUT KEYS SHOULD BE FORMED BY TERMINATING THE GROUT 1-1/2 INCHES BELOW A MORTAR JOINT.

THE BEAMS:

BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH THE BEAMS WITH MINIMUM LAP SPICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED. IN ACCORDANCE WITH ACI 318.1, SECTION 4.3.3.3 (SOLID METAL OR Felt CAVITY CAPS ARE PROHIBITED).

THE COLUMNS:

CONCRETE THE COLUMNS SHALL BE PLACED AFTER THE MASONRY CMU WALLS. THE CONCRETE BLOCK FACING THE COLUMN SHALL BE REMOVED SO THAT WHEN THE CONCRETE COLUMN IS PLACED, THE CONCRETE WILL FLOW INTO THE BLOCK CELL INTERLOCKING THE COLUMN WITH THE BLOCK. THIS SHALL OCCUR AT THE TOP AND BOTTOM OF THE WALL AND AT 24" ON CENTER FOR THE FULL HEIGHT OF THE INTERFACE BETWEEN THE BLOCK AND THE COLUMN.

LINTELS

MASONRY OPENINGS LESS THAN 6 FEET SHALL BE SPANNED WITH AN 8" SPAN RATED PRECAST/PRESTRESSED CONCRETE LINTEL. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END ON A GROUT FILLED CELL.

MASONRY OPENINGS 6 FEET OR GREATER SHALL BE SPANNED WITH AN 8" SPAN RATED PRECAST/PRESTRESSED CONCRETE LINTEL WITH 16 BAR CONTINUOUS. PRECAST LINTEL AND ALL CELLS ABOVE. TO THE BOTTOM OF THE TIE BEAM OR BOND BEAM, SHALL BE GROUTED SOLID. ALL PRECAST LINTELS SHALL BEAR A MINIMUM OF 8" AT EACH END ON A GROUT FILLED CELL.

WHERE A CONCRETE COLUMN OR CONCRETE COLUMN IS WITHIN 8" OF A MASONRY OPENING, THE LINTEL SHALL BE AN 8"X16" CONCRETE CAST-IN-PLACE BEAM WITH (2) #5 BARS TOP AND BOTTOM, AND #3 STIRRUPS AT 18" ON CENTER.

STRUCTURAL STEEL

WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-992 WITH A MINIMUM YIELD STRENGTH OF 50 KSI. STEEL SECTIONS EXCEPT WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-36 WITH A 36 KSI YIELD STRENGTH. ALL STEEL SHALL CONFORM TO THE SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. ALL SHOP CONDITIONS TO BE WELDED (UTILIZING F70X ELECTRODES) AND WELD CONNECTIONS TO BE BOLTED, UNLESS OTHERWISE NOTED ON STRUCTURAL DRAWINGS. STEEL TO RECEIVE ONE SHOP COAT AND ONE FIELD TOUCH-UP COAT OF APPROVED PAINT, EXCEPT WHERE GALVANIZING IS INDICATED ON THE DRAWINGS.

STRUCTURAL TUBING SHALL CONFORM TO ASTM A-500, GRADE B, Fy = 46 ksi. STRUCTURAL PIPE SHALL CONFORM TO ASTM A-53 GRADE B, TYPE E OR S, Fy = 35 ksi. WHERE CONCRETE-FILLED COLUMNS ARE SPECIFIED, TUBE & PIPE COLUMNS SHALL BE CONCRETE FILLED IN THE SHOP OR, WHEN THEY SUPPORT CONCRETE BEAMS OR FORMED CONCRETE SLABS, A HOLE PROVIDED IN THE CAP PLATE FOR FIELD FILLING.

ALL BOLTED CONNECTIONS SHALL CONSIST OF MINIMUM 3/4 INCH DIAMETER ASTM A-325 HIGH STRENGTH BOLTS. BEAM CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR FOR THE REACTIONS SHOWN ON THE PLANS. IF NOT SHOWN, THE FABRICATOR SHALL DESIGN THE BEAM CONNECTIONS TO SUPPORT A MINIMUM OF 1/2 INCHES FROM TOTAL 1/4" MAXIMUM TOTAL UNIFORM LOAD OF STEEL CONSTRUCTION MANUAL (13th EDITION), BUT CONNECTIONS SHALL NOT HAVE LESS THAN 2 ROWS OF BOLTS.

MINIMUM CONNECTIONS: AT A MINIMUM THE BOLT ROWS (AT 3" CENTERS) SHALL EXTEND FOR 2/3 OF THE BEAM DEPTH AND AS REQUIRED ABOVE.

SLOTTED HOLES: WHEREVER LONG SLOTTED HOLES ARE USED 5/16" THICK WASHERS SHALL BE PROVIDED PER AISC REQUIREMENTS.

ANCHOR BOLTS

SHALL CONFORM TO ASTM A-307 OR F1554 (THREADED ROD). ANCHOR BOLTS SHALL BE MINIMUM ASTM A-36 (GRADE 36) STEEL.

MACHINE AND LAG BOLTS

SHALL BE A-307 HOT DIPPED GALVANIZED WITH GALVANIZED WASHERS.

SHEAR STUD CONNECTORS

SHEAR STUD CONNECTORS SHALL BE FABRICATED AND INSTALLED IN ACCORDANCE WITH AWS D1.1 "STRUCTURAL WELDING CODE", SECTION 7. STUD WELDING: STUDS SHALL BE TYPE "B", HEADED STUDS HAVING A MINIMUM TENSILE STRENGTH OF 60,000 psi, AND SHALL BE OF LENGTH AND DIAMETER SHOWN ON STRUCTURAL DRAWINGS.

STEEL JOISTS

SHALL BE THE SIZE AND SPACING AS SHOWN ON THE STRUCTURAL DRAWINGS AND SHALL BE DESIGNED, FABRICATED, INSTALLED AND BRIDGED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS. ENDS OF ALL BRIDGING LINES TERMINATING AT WALLS OR BEAMS SHALL BE ANCHORED THEREAT AT TOP AND BOTTOM CHORDS. MINIMUM JOIST BRIDGING TERMINATION CONNECTIONS TO MASONRY SHALL BE L3 X 3 X 1/4 X 3" LONG WITH (1) 1/2" DIAMETER ANCHOR BOLT OR 1/4 X 4 X 4 WITH (1) 1/2" X 5" HEADED STUD TO CONCRETE. BRIDGING SHALL BE WELDED OR BOLTED AT ALL POINTS OF CONTACT. WELD SHALL NOT DAMAGE THE JOIST. CROSS BRIDGINGS SHALL BE WELDED OR BOLTED AT ITS CENTER POINT.

K-SERIES JOISTS SHALL BEAR A MINIMUM OF 2, 1/2" ON STEEL BEAMS AND 4" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 3/8" X 4" X 7, 1/2" WITH (2) 1/2" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 3/8" X 7, 1/2" X 7, 1/2" WITH (3) 1/2" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATES WITH A MINIMUM OF (2) 1/8 X 1" FILLET WELDS, UNLESS NOTED OTHERWISE.

LH-SERIES JOISTS SHALL BEAR A MINIMUM OF 4" ON STEEL BEAMS AND 6" ON CONCRETE BEAMS. JOIST BEARING PLATES TO BE MINIMUM 3/8" X 6" X 9" WITH (2) 1/2" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES FOR BACK TO BACK SINGLE JOISTS SHALL BE MINIMUM 3/8" X 9" X 11, 5/8" WITH (4) 1/2" DIAMETER X 5" SHEAR STUD CONNECTORS. BEARING PLATES SHALL BE CAST INTEGRALLY WITH THE CONCRETE BEAM. WELD JOISTS TO BEARING PLATE WITH A MINIMUM OF (2) 1/4 X 2" FILLET WELDS, UNLESS NOTED OTHERWISE. BACK TO BACK JOISTS SHALL BE OFFSET IF CONCRETE BEAM IS LESS THAN 12" NOMINAL WIDTH OR STEEL BEAM IS LESS THAN 9" WIDE.

SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWING SUBMITTAL SHALL INCLUDE LAYOUT, COMPONENT DESIGNATION, BRIDGING, AND DETAILS. SUBMITTALS FOR JOISTS, OTHER THAN STANDARD SJI CATALOGUE SELECTIONS WHICH HAVE BEEN CHECKED BY SJI, SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

JOIST BEARING: WHERE STEEL BEAMS SUPPORT JOISTS FRAMING FROM ONE SIDE ONLY, OR WHERE JOISTS FROM ONE SIDE ARE 30% LONGER THAN THE JOISTS ON THE OPPOSITE OF THE BEAM, JOISTS SHALL BEAR FULL WIDTH OF THE STEEL BEAM OR 5, 1/2", WHICHEVER IS LESS.

NET UPLIFT DESIGN: (STEEL ROOF JOISTS SHALL BE DESIGNED TO WITHSTAND " " psi NET UPLIFT DUE TO WIND) STEEL ROOF JOISTS SHALL BE DESIGNED TO WITHSTAND THE NET UPLIFT DUE TO WIND AS REQUIRED ASCE 7, NOT LESS THAN ZONE 2 WIND PRESSURES. IN ADDITION TO THE STANDARD SJI BOTTOM CHORD BRIDGING, WHICH INCLUDES THE FIRST END PANELS, THE JOIST MANUFACTURER SHALL PROVIDE DESIGN CALCULATIONS FOR UPLIFT, EITHER CONFIRMING THE SJI BRIDGING REQUIREMENT OR PROVIDING A DESIGN ADEQUATE FOR THE UPLIFT. THIS SUBMITTAL SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

STEEL JOIST GIRDERS: SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS FOR JOIST GIRDERS AS PUBLISHED BY THE STEEL JOIST INSTITUTE (SJI). JOIST GIRDERS SHALL BE OF THE DEPTH AND SPACING SHOWN ON THE STRUCTURAL DRAWINGS, AND UNLESS OTHERWISE NOTED, JOIST GIRDERS SHALL BE DESIGNED AS SIMPLY SUPPORTED PRIMARY MEMBERS PROPORTIONED SUCH THAT THEY MAY BE ERECTED WITHOUT BRIDGING. JOIST GIRDER PANEL POINTS SHALL BE ALIGNED WITH SECONDARY MEMBERS.

CONCRETE SUPPORTS: UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS, JOIST GIRDERS SHALL BEAR ON 5/8" X 7" X 1, 0" STEEL BEARING PLATES WITH A MINIMUM OF FOUR 1/2" DIAMETER 8" LONG SHEAR STUD CONNECTORS (CONFORMING TO AWS D1.1). THE CONNECTION TO THE BEARING PLATE SHALL BE DESIGNED TO RESIST ALL UPLIFT AND SHEAR LOADS, HOWEVER, A MINIMUM OF TWO 1/4" FILLET WELDS 2 INCHES LONG SHALL BE UTILIZED. MINIMUM JOIST GIRDER BEARING SHALL BE 6".

STEEL SUPPORTS: UNLESS NOTED OTHERWISE ON STRUCTURAL DRAWINGS, JOIST GIRDERS MAY BE DIRECTLY CONNECTED TO STEEL GIRDERS HAVING A FLANGE THICKNESS GREATER THAN 3/8". THE CONNECTION SHALL BE DESIGNED TO RESIST ALL UPLIFT AND SHEAR LOADS; HOWEVER, A MINIMUM OF TWO 1/4" FILLET WELDS 2 INCHES LONG OR TWO 3/4" DIAMETER HIGH STRENGTH BOLTS SHALL BE UTILIZED. MINIMUM JOIST GIRDER BEARING SHALL BE 4 INCHES.

SUPPLIER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW BY THE STRUCTURAL ENGINEER PRIOR TO FABRICATION. SUBMITTALS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER.

STEEL ROOF DECK

SHALL BE 1/2" DEEP, 22 GAGE GALVANIZED (G60) CORRUGATED STEEL DECK, AND SHALL CONFORM TO PROVISIONS OF THE STEEL DECK INSTITUTE (SDI). FASTEN DECK TO STRUCTURAL STEEL SUPPORTS BY PUDDLE WELDS IN A 3/64" PATTERN. USE WELDING WASHERS FOR DECK THINNER THAN 22 GAGE. FASTEN SIDE LAPS AT MID-SPAN WITH (1) #10 SCREW, (UNLESS OTHERWISE INDICATED ON THE PLANS).

LIGHT GAUGE STRUCTURAL STEEL

ROOF DECK: SHALL HAVE A MINIMUM YIELD STRENGTH OF 50,000 psi. THE PANELS SHALL BE GALVANIZED WITH 1.25 OUNCES OF COMMERCIAL ZINC COATING. ATTACHMENTS, CLOSURES ETC. SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. SUBMIT MANUFACTURER'S DATA AND LOAD TABLES FOR REVIEW.

ROOF PURLINS: SHALL BE COLD FORMED STEEL WITH A MINIMUM YIELD STRENGTH OF 55,000 psi AS MANUFACTURED BY AMERICAN BUILDINGS OR EQUAL. ATTACHMENTS / HARDWARE SHALL BE AS SHOWN AND INDICATED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

METAL STUDS: ALL STEEL MEMBERS SHALL BE DESIGNED IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS' 2012 EDITION. ALL STUDS AND JOISTS SHALL BE GALVANIZED AND FORMED FROM STEEL THAT CORRESPONDS TO THE MINIMUM REQUIREMENTS OF THE 2012 AISI STANDARDS. STEEL STUDS, LINTELS, AND RUNNER TRACK MEMBERS SHALL BE OF TYPE SHOWN ON THE DRAWINGS AND SPECIFICATIONS CONFORMING TO ASTM A1008 SS (STRUCTURAL STEEL) GRADE 33 (MINIMUM YIELD POINT 33,000 psi) WITH HOT-DIPPED GALVANIZED COATING CONFORMING TO ASTM A653 (CLASS 80). GALVANIZED RUNNER TRACK STEEL SHALL BE FORMED WITH MATERIAL MEETING REQUIREMENTS OF ASTM A1008 SS (STRUCTURAL STEEL) GRADE 33 (MINIMUM YIELD POINT 33,000 psi) WITH HOT-DIPPED GALVANIZED COATING CONFORMING ASTM A653 (CLASS 80). ALL PRODUCTS TO BE MANUFACTURED BY THE CURRENT MEMBERS OF THE STEEL STUD MANUFACTURERS ASSOCIATION, OR ENGINEER-APPROVED ALTERNATIVE.

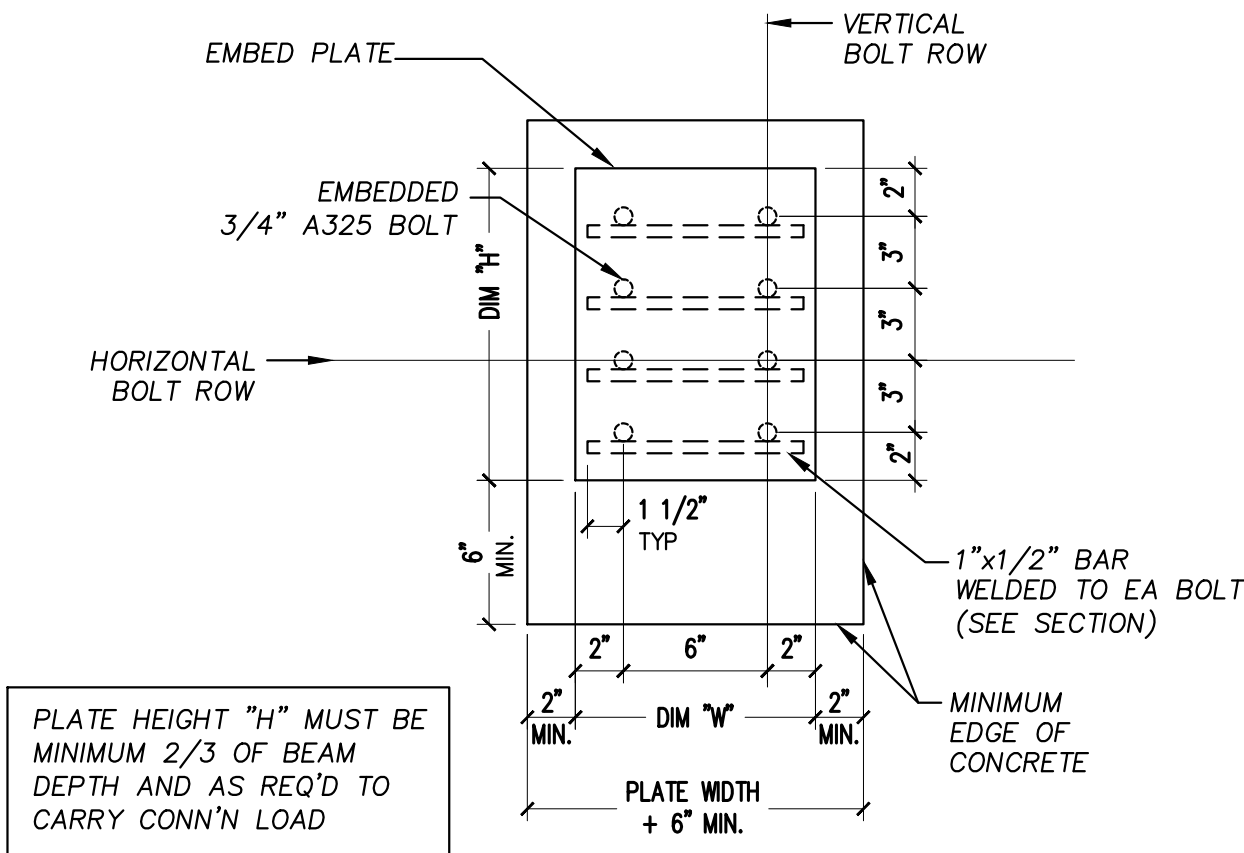
STUDS WITH 33 AND 43 MIL THICKNESSES SHALL BE GRADE 33 (33,000 psi YIELD POINT), UNLESS NOTED OTHERWISE. STUDS WITH 54, 68 AND 97 MIL THICKNESSES SHALL BE GRADE 50 (50,000 psi YIELD POINT), UNLESS NOTED OTHERWISE.

ASSEMBLY: ALL FRAMING MEMBERS SHALL BE CUT SQUARELY OR AT AN ANGLE AS REQUIRED TO FIT SQUARELY AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD FIRMLY IN PLACE UNTIL PROPERLY JOINED. JOINING OF STRUCTURAL MEMBERS SHALL BE MADE USING SELF-DRILLING SCREWS OR WELDING. WIRE TYING OF FRAMING MEMBERS IN STRUCTURAL APPLICATIONS SHALL NOT BE PERMITTED. ATTACHMENT OF COLLATERAL MATERIALS TO STEEL MEMBERS SHALL BE MADE WITH SELF-DRILLING SCREWS OR HARDENED SCREW SHANK NAILS. METAL LATH MAY ALSO BE CONNECTED TO STEEL BY STAPLES OR OTHER FASTENERS, IF APPROVED BY LOCAL BUILDING CODE.

INSTALLATION: STUDS SHALL SIT SQUARELY IN THE TOP AND BOTTOM RUNNER TRACK WITH FIRM ABUTMENT AGAINST TRACK WEBS. STUDS SHALL BE ALIGNED OR PLUMBED AND SECURELY FASTENED TO THE FLANGES OF BOTH TOP AND BOTTOM RUNNER TRACK.

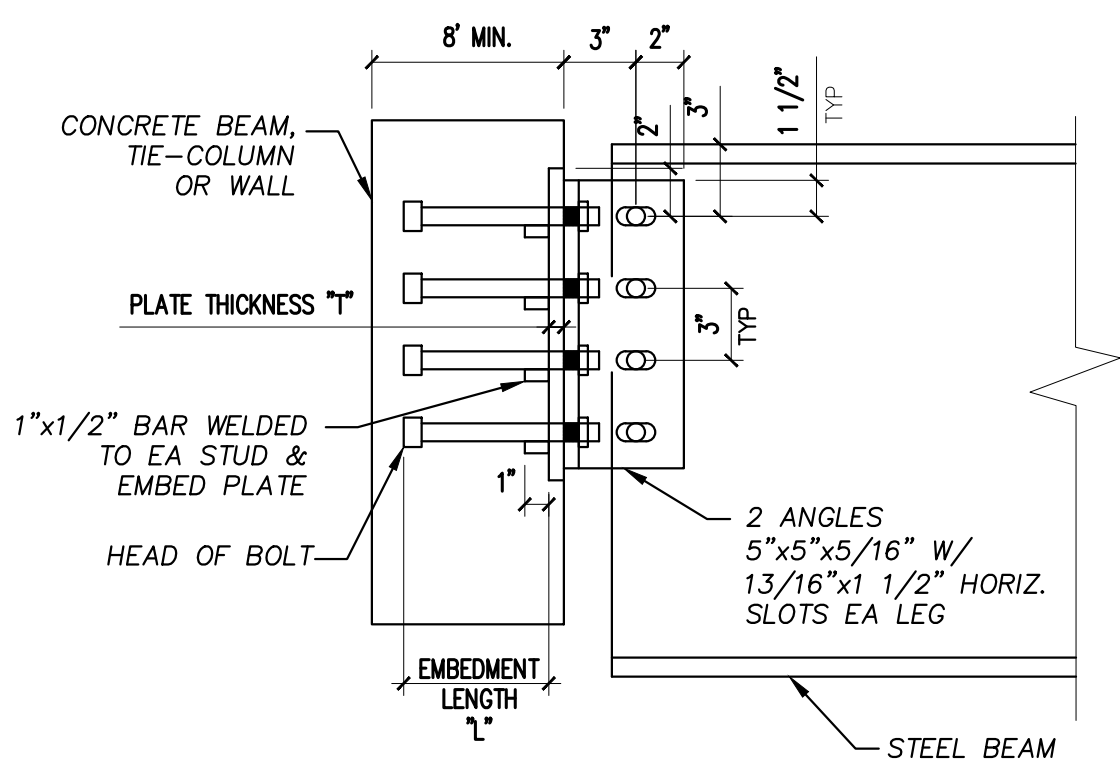
SHOP DRAWINGS: ENGINEERED LIGHT GAUGE FRAMING FOR EXTERIOR WALLS AND ALL CONNECTIONS OF SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. SUBMIT SHOP DRAWINGS BEARING THE EMBOSSED SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. LIGHT GAUGE FRAMING SYSTEM AND ALL CONNECTIONS SHALL BE DESIGNED FOR ALL APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN THE BUILDING CODE. THE LOADS SHALL BE CLEARLY INDICATED ON ALL SHOP DRAWINGS.

PRE-ENGINEERED LIGHT GAUGE METAL TRUSSES: ENGINEERED TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, TRUSS PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND LOADING, EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE ENGINEER OF RECORD. SUBMIT

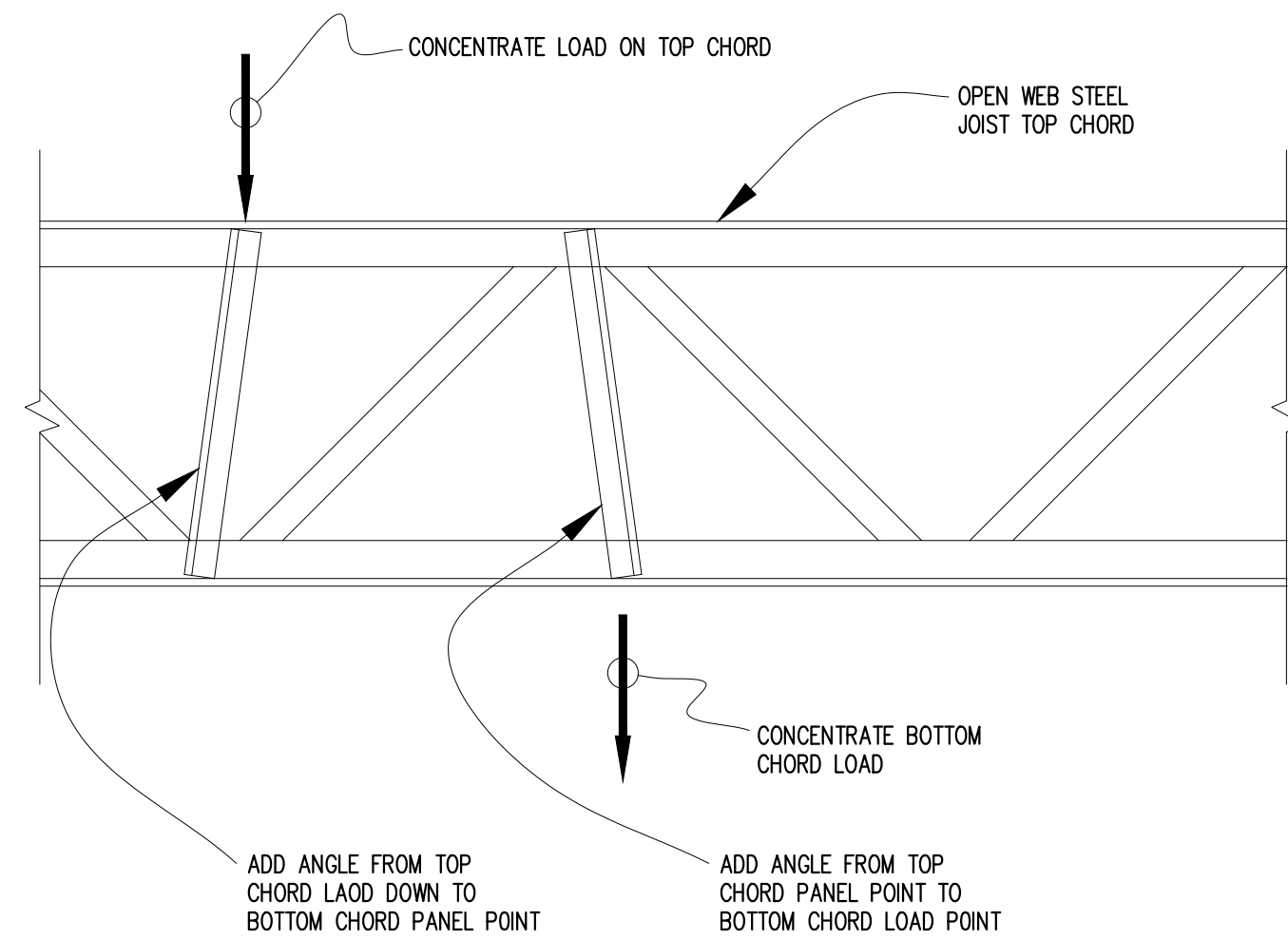


EMBED PLATE ELEVATION
SCALE: NONE

DESIGN NUMBER	SHEAR CAPACITY	EMBEDDED PLATE			EMBEDDED ANCHORS			
		WIDTH "W"	HEIGHT "H"	THICKNESS "T"	BOLT D/A	LENGTH "L"	#VERT'L ROWS	#HORIZ. ROWS
P22	30K	10"	7"	1/2"	3/4"	5"	2	2
P23	45K	10"	10"	1/2"	3/4"	5"	2	3
P24	60K	10"	13"	5/8"	3/4"	5"	2	4
P25	70K	10"	16"	5/8"	3/4"	5"	2	5
P26	85K	10"	19"	3/4"	3/4"	5"	2	6
P27	100K	10"	22"	3/4"	3/4"	5"	2	7
P28	110K	10"	25"	3/4"	3/4"	5"	2	8
P29	120K	10"	28"	3/4"	3/4"	5"	2	9
NOTES								
1.	DESIGN TENSILE FORCE = 2000 LBS PER EMBEDDED BOLT SIMULTANEOUSLY WITH SHEAR LOAD							
2.	4000 PSI CONC.							



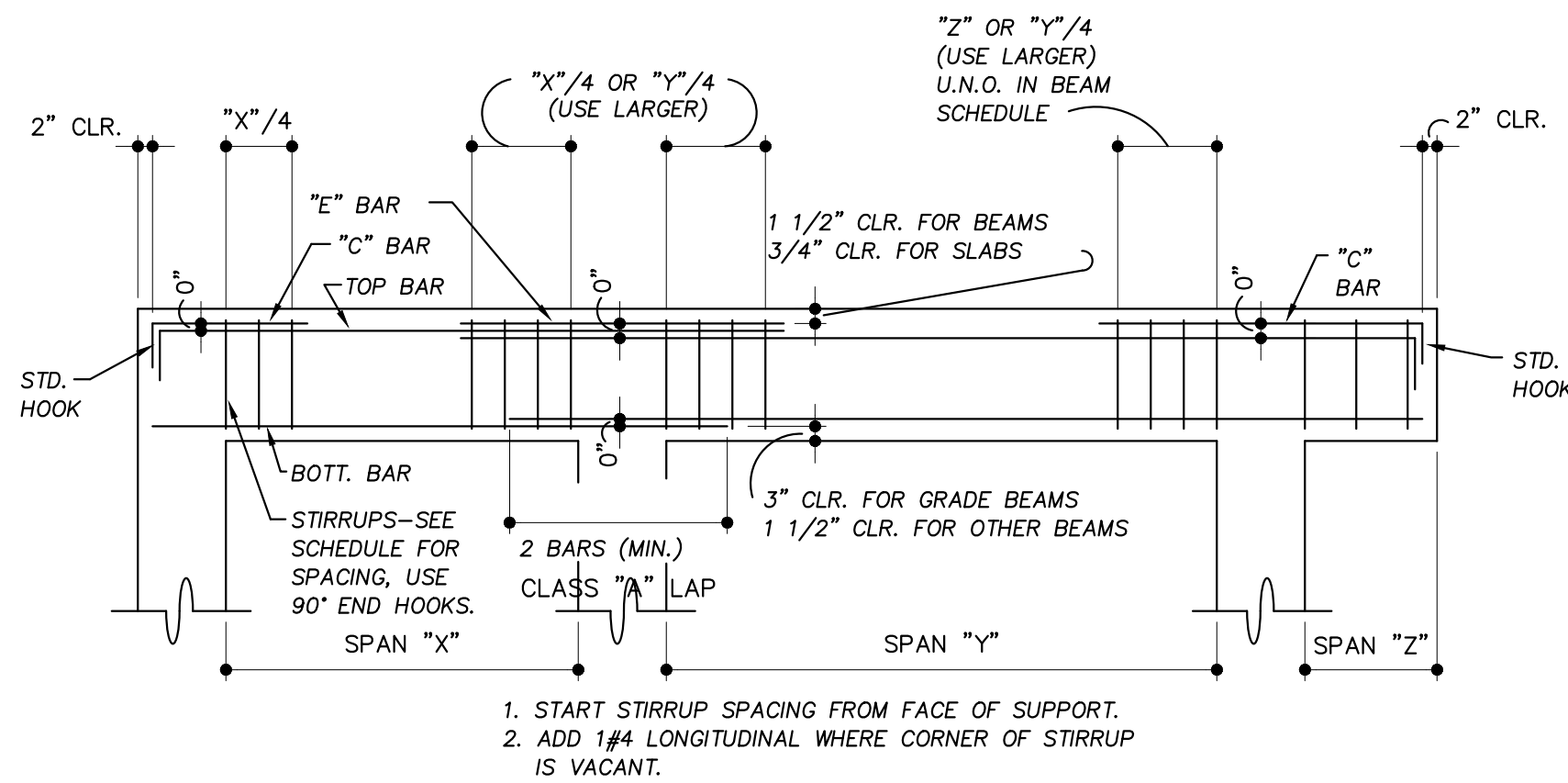
STEEL BEAM / EMBED PLATE SECTION
SCALE: NONE



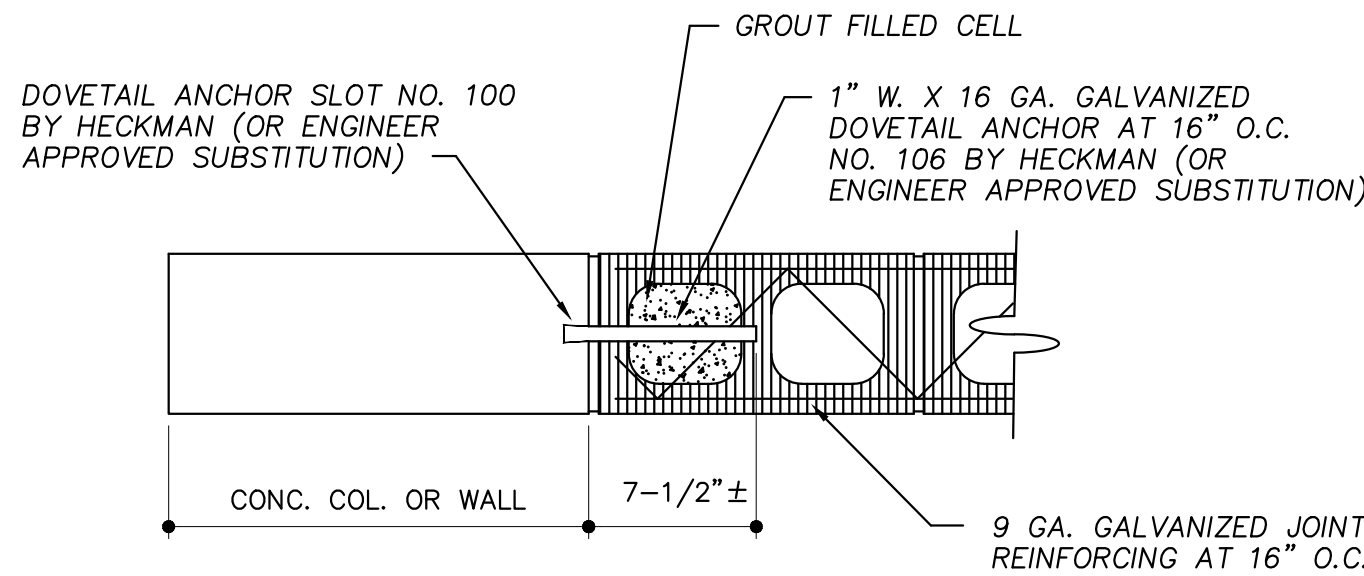
ADD STEEL ANGLE TO BE SIZED AS FOLLOWS
JOIST DEPTH UP TO 16" L2 1/2x2 1/2x1/4
JOIST DEPTH 16" - 28" L3x3x1/4
JOIST DEPTH 28" - 48" L4x4x5/16"

TYPICAL JOIST REINFORCEMENT DETAIL AT CONCENTRATED LOADS

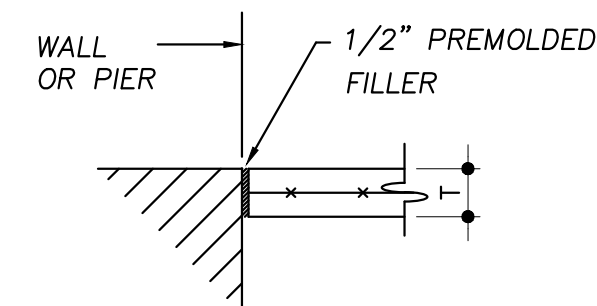
SCALE 1" = 1'-0"



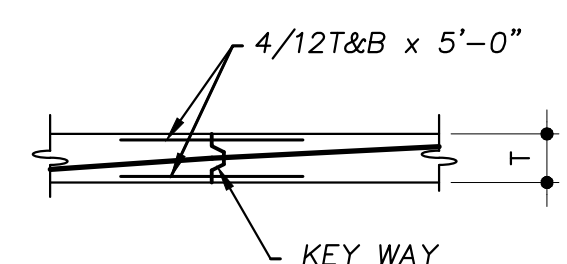
TYP. BENDING DIAGRAM FOR BEAMS AND ONE WAY SLABS
SCALE: NONE



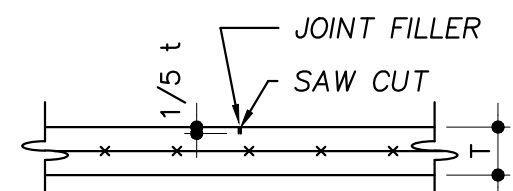
DOVETAIL ANCHOR DETAIL
SCALE: NONE



ISOLATION JOINT (IJ)
SCALE: NONE

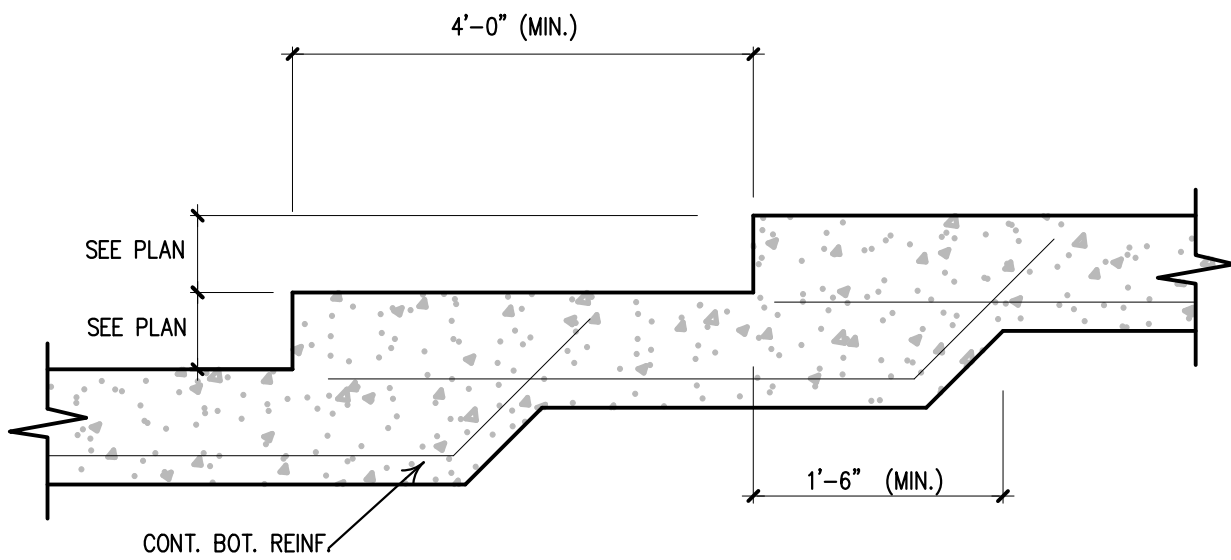


TYP.CONSTR. JOINT DETAILS
SCALE: NONE

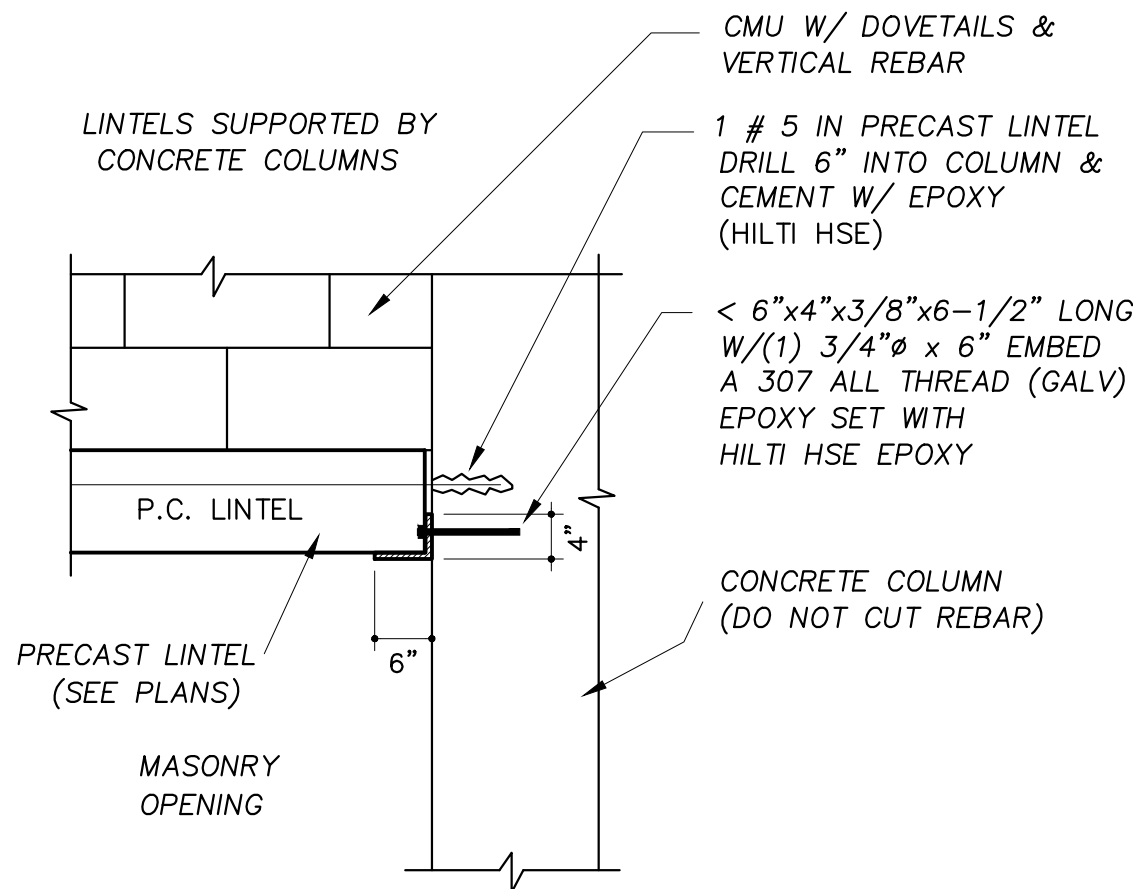


CONTROL JOINT (CJ)
SCALE: NONE

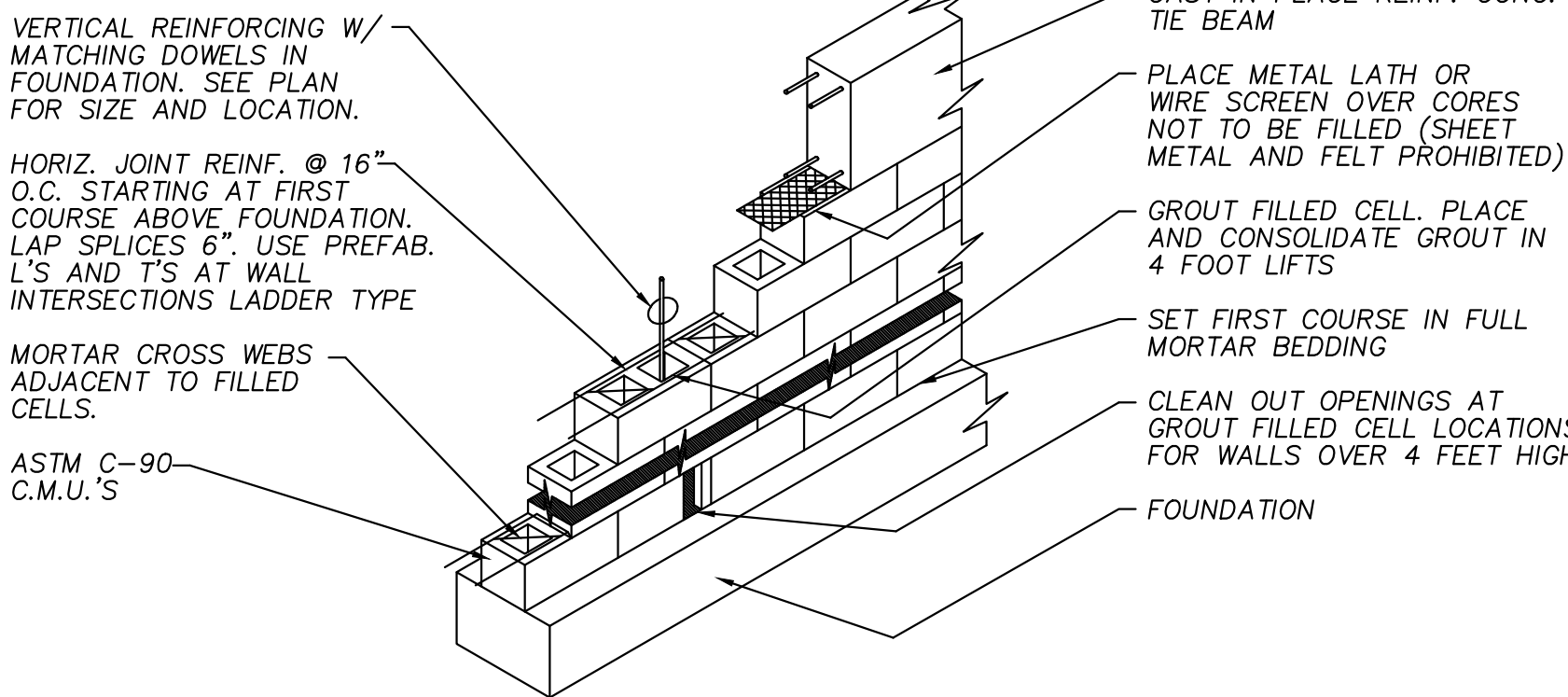
SAW CUT AS SOON AS POSSIBLE WITHOUT RAVELING CONCRETE. (4 TO 16 HOURS AFTER POUR)



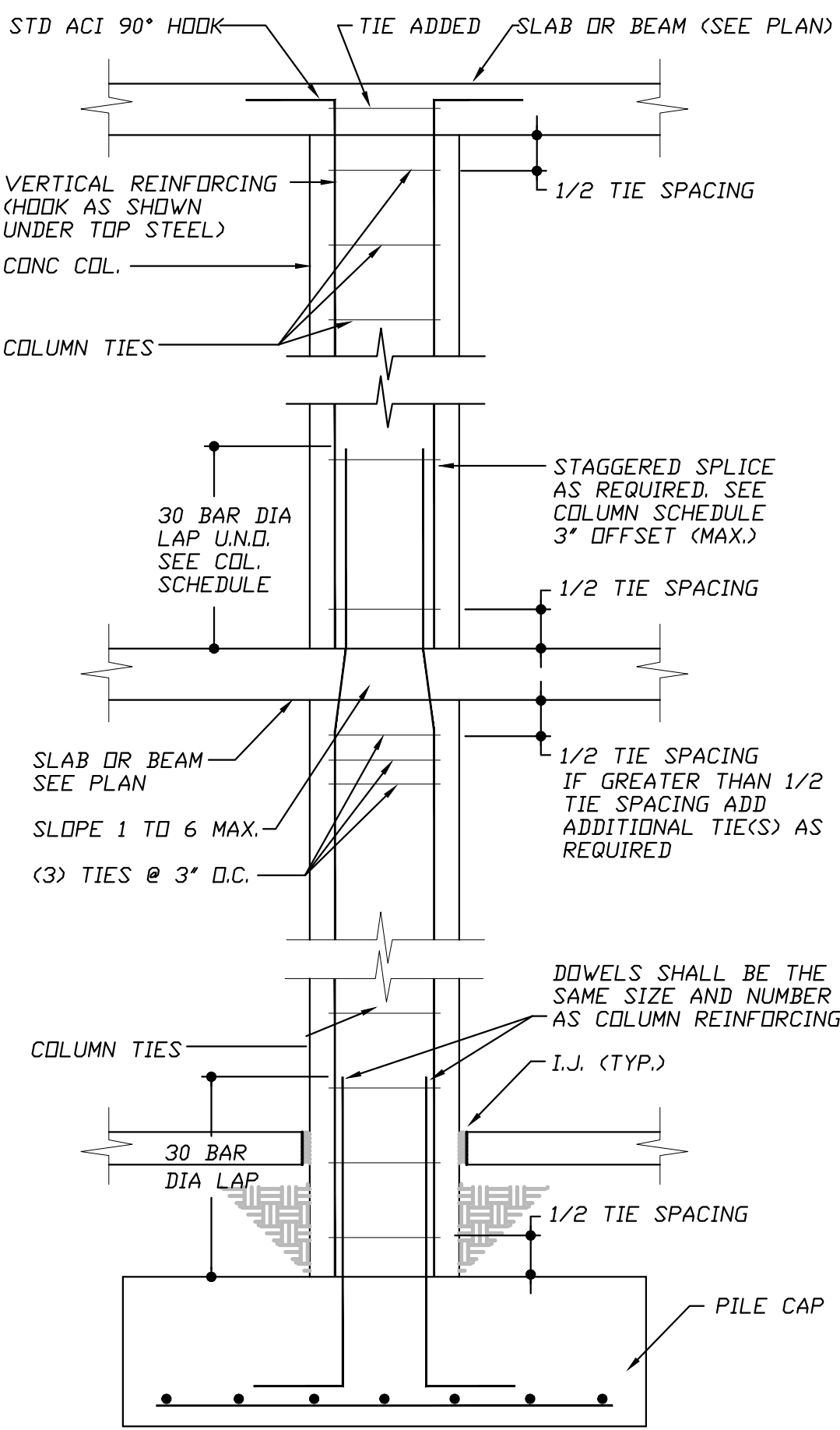
STEPPED FOOTING DETAIL
N.T.S.



PRECAST LINTEL AT COLUMN DETAIL
N.T.S.



TYPICAL MASONRY WALL CONSTRUCTION
SCALE: NONE



TYPICAL COLUMN REINFORCEMENT DETAIL
SCALE: NONE

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EMAIL: inbararchitecture@gmail.com



REVISIONS

OWNER APPROVALS

- | | |
|----|--------------|
| 01 | DESIGN STAGE |
| 02 | DD STAGE |
| 03 | CD STAGE |

PROPOSED NEW HOME FOR:

NOKOMIS MEDICAL CENTER

498 S. TAMiami TrL.
NOKOMIS, FL.

PERMIT #

STRUCTURAL DETAILS

Drawing Name:	NOKOMIS OFFICE
Design:	RDW
Issues:	06/16/2022
Job No.:	
Sheet Number:	