GENERAL STRUCTURAL NOTES:

1.) DESIGN AND CONSTRUCTION SHALL CONFORM TO THE BUILDING CODE OF NEW YORK STATE -2010 FOR AN OCCUPANCY CATEGORY OF II.

2.) THE DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE AS A COMPLETE UNIT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL MEANS, METHODS, AND SEQUENCES OF ALL PHASES OF CONSTRUCTION AND DEMOLITION INCLUDING TEMPORARY SHORING, BRACING, COLD- AND HOT-WEATHER PROTECTION, PROTECTION OF ADJACENT PROPERTY AND FOUNDATION UNDERPINNING. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE CODES AND STANDARDS THAT PERTAIN TO SAFETY, MEANS, METHODS, AND SEQUENCES OF CONSTRUCTION.

3.) THE STRUCTURAL DRAWINGS ARE TO BE USED IN CONJUNCTION AND COORDINATED WITH ALL OTHER CONTRACT DRAWINGS AND SPECIFICATIONS. THE CONTRACTOR SHALL COORDINATE SUCH REQUIREMENTS INTO THEIR WORK.

4.) WHERE A CONFLICT EXISTS IN THE CONSTRUCTION DOCUMENTS, THE MOST STRINGENT SPECIFICATION SHALL GOVERN AS DETERMINED BY THE ARCHITECT.

5.) CONTRACTOR TO COORDINATE ALL OPENINGS, EQUIPMENT LOCATIONS, AND INSERTS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE TRADES THAT REQUIRE THEM. PENETRATIONS THROUGH STRUCTURAL MEMBERS ARE NOT PERMITTED EXCEPT AS DETAILED.

6.) CONTRACTOR SHALL VERIFY EXISTING CONDITIONS IN THE FIELD AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES.

7.) SHOP DRAWINGS PREPARED BY SUPPLIERS AND SUBCONTRACTORS SHALL BE REVIEWED AND APPROVED BY THE CONTRACTOR PRIOR TO SUBMISSION TO THE ARCHITECT AND ENGINEER. REPRODUCTIONS OF THE STRUCTURAL DRAWINGS SHALL NOT BE USED FOR THE PREPARATION OF SHOP DRAWINGS. MANUFACTURED COMPONENTS SHALL BE SIGNED AND STAMPED BY A LICENSED PROFESSIONAL ENGINEER PRIOR TO SUBMISSION.

CONCRETE:

1.) CONCRETE DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 318 AND ACI 301.

2.) MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE:
FOOTINGS......................4,000psi, MAX. W/C RATIO = 0.45
PIERS, FND. WALLS..........4,000psi, MAX. W/C RATIO = 0.45
SLAB ON GRADE..............3,000psi, MAX. W/C RATIO = 0.54
SLAB ON DECK.................3,500psi, LIGHTWEIGHT (107-113pcf)
CONCRETE FILL (STAIRS)......3,000psi
GYPSUM FILL....................2,000psi

3.) FLYASH CONTENT SHALL NOT EXCEED 15%

4.) LEVELING GROUT SHALL BE NON-SHRINK CONFORMING TO ASTM C1107 WITH A MINIMUM 2-DAY STRENGTH OF 5,000psi.

5.) ALL CONCRETE EXPOSED TO FROST SHALL HAVE ENTRAINED AIR AS FOLLOWS:

MAXIMUM AGGREGATE SIZE	AIR CONTENT
2 in.	4% TO 6.5%
1.5 in.	4% TO 7%
1 in.	4.5% TO 7%
0.75 in.	4.5% TO 7%

6.) REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, AND SHALL BE SUPPLIED IN SHEETS ONLY. THE WELDING OF REINFORCING BARS SHALL CONFORM TO ASTM A706 AND AWS D1.4.

7.) BASIS OF DESIGN FOR EPOXY ADHESIVE ANCHORS IS HILTI HY150. ALTERNATE ANCHOR SYSTEMS MAY BE SUBMITTED FOR REVIEW. CONTRACTOR TO SUBMIT ENGINEERING DATA INDICATING EQUIVALENCE OF THE ALTERNATE SYSTEM TO THAT WHICH IS SPECIFIED.

8.) THE CONTRACTOR SHALL FABRICATE ALL REINFORCEMENT AND FURNISH ALL ACCESSORIES, CHAIRS, SPACER BARS AND SUPPORTS NECESSARY TO SECURE AND PROPERLY POSITION REINFORCEMENT PRIOR TO POURING CONCRETE. THE POSITIONING OR SETTING OF REBAR OR WIRE MESH IN WET CONCRETE IS NOT PERMITTED.

9.) CLEAR COVER TO REINFORCING SHALL BE AS SPECIFIED BY ACI-318, EXCEPT WHERE NOTED OTHERWISE.

10.) UNLESS SHOWN OTHERWISE, DOWELS INTO FOUNDATION ELEMENTS SHALL HAVE A STANDARD 90 DEGREE HOOK AND SHALL BE THE SAME SIZE AND QUANTITY OF PIERS AND WALLS ABOVE THE FOUNDATION.

11.) PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCING IN ALL WALLS AND FOOTINGS.

12.) CONCRETE TESTING SHALL BE AS FOLLOWS:

A.) OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIX EXCEEDING 5 CU. YD., BUT LESS THAN 25 CU. YD. PLUS ONE SET FOR EACH ADDITIONAL 50 CU. YD. OR FRACTION THEREOF (ASTM C 172).

B.) OBTAIN: SLUMP (ASTM C 143), AIR CONTENT (ASTM C 231), TEMPERATURE (ASTM C 1064), UNIT WEIGHT: (ASTM C 567), COMPRESSION TEST SPECIMENS (ASTM C 31) FOR EACH COMPOSITE SAMPLE.

C.) CAST AND FIELD CURE ONE SET OF FOUR STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE. TEST (1) AT 7 DAYS, (2) AT 28 DAYS AND (1) AT 56 DAYS.

13.) UNLESS SHOWN OTHERWISE, PIPES OR OTHER TYPES OF CONDUIT ARE NOT PERMITTED TO BE EMBEDDED IN CONCRETE. SLEEVES OR OTHER INSERTS SHALL NOT BE PLACED IN OR THROUGH CONCRETE UNLESS SHOWN ON THE SHOP DRAWINGS.

14.) SLABS ON GRADE TO HAVE A MAXIMUM MOISTURE EMISSION RATE OF 3 lbs. PER 1000 SQ. FT. IN 24 HOURS PRIOR TO PLACING FLOOR FINISHES, OR AT A MOISTURE EMISSION RATE REQUIRED BY THE FLOOR COVERING MANUFACTURER, WHICHEVER IS MORE STRINGENT.

15.) ACTUAL SLAB THICKNESS MAY VARY DUE TO SUPPORT DEFLECTIONS. CONTRACTOR TO ADJUST CONCRETE QUANTITIES AS REQUIRED TO MAINTAIN LEVEL SLAB SURFACES AT THE REQUIRED ELEVATIONS.

16.) CONCRETE SHALL BE PROTECTED AGAINST FROST UNTIL SPECIFIED STRENGTH IS ATTAINED.

17.) COORDINATE LOCATION OF PIPES PENETRATING FOUNDATION WALLS WITH MECHANICAL DRAWINGS. PROVIDE PIPE SLEEVES AT PENETRATIONS, REFER TO TYPICAL DETAILS.

18.) REFERENCE ARCHITECTURAL AND PLUMBING PLANS FOR SLOPED FLOOR SLABS. ALL SLABS TO MAINTAIN FULL THICKNESS. THICKEN SLABS AT DEPRESSIONS AND FLOOR DRAINS TO MAINTAIN FULL THICKNESS.

19.) PROVIDE 15mil VAPOR BARRIER UNDER SLAB ON GRADE THAT MEETS ASTM E1745 CLASS A. INSTALL IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.

20.) PROVIDE 1/2 INCH THICK PREFORMED JOINT FILLER AND CAULK WHERE CONCRETE ABUTS WALLS OR OTHER VERTICAL SURFACES.

21.) SUBMIT SHOP DRAWINGS FOR REINFORCING STEEL, CONCRETE MIX DESIGNS, AND CONTROL JOINT LAYOUT PLAN TO THE ARCHITECT FOR REVIEW BEFORE CONSTRUCTION.

22.) COMPLY WITH ACI 308.1 FOR CURING OPERATIONS.

23.) COMPLY WITH ACI302.1R FOR ALL FINISHING OPERATIONS.

DESIGN LOADS:

1.) LIVE LOADS:
SLAB-ON-GRADE = 100psf
ROOF = 20psf

2.) SNOW LOADS:
Pg = 40psf
Pf,Ps = 31psf
Ce = 1.0
Ct = 1.1
Cs = 1.0
Is = 1.0
EXP = B

3.) WINDS LOADS:
V = 90mph
EXPOSURE = C
Iw = 1.0
COMPONENTS AND CLADDING: PER ASCE 7-05

4.) SEISMIC LOADS:
SDS = 0.225
SD1 = 0.096
Ie = 1.0
SITE CLASS= C (ASSUMED)
R = 3.0 (STEEL FRAMING)
SEISMIC DESIGN CATEGORY = B
SEISMIC FORCE RESISTING SYSTEM: STRUCTURAL STEEL FRAMING NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE.

5.) UNLESS SHOWN OTHERWISE, STAIRS, STAIR COMPONENTS, CONNECTIONS, AND RAILINGS SHALL BE DESIGNED AND INCORPORATED INTO THE STEEL SHOP DRAWINGS AS FOLLOWS. LOADS SHOWN ARE IN ADDITION TO THE DEAD LOAD.

A.) MAIN STRINGERS AND LANDINGS: LIVE LOAD = 100psf
B.) TREADS: LIVE LOAD IS THE LARGEST OF: 100psf OR 300lbs. ON 4 SQ IN.
C.) HANDRAIL ASSEMBLIES: LARGEST OF: 200lb OR 50plf. APPLIED IN ANY DIRECTION AT ANY POINT ON THE ASSEMBLY.
D.) COORDINATE FINAL DESIGN WITH ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS.
E.) MEET NYS CODE FOR ALL OTHER DETAILS AND DIMENSIONS NOT SPECIFICALLY INDICATED.
F.) MEMBER SIZES SHOWN ON THE DRAWINGS ARE A MINIMUM.
G.) FRAMING DETAILS SHOWN ARE SCHEMATIC. CONTRACTOR TO PROVIDE ADEQUATE SUPPLEMENTAL FRAMING AS NEEDED TO SUPPORT STAIR AND RAILING COMPONENTS.

MASONRY:

1.) CONCRETE MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530.

2.) CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, TYPE 1, GRADE N, MOISTURE CONTROLLED UNITS WITH F'm = 3,000psi.

3.) MORTAR SHALL BE TYPE M OR S AND SHALL CONFORM TO ASTM C270.

4.) HORIZONTAL JOINT REINFORCING: 9ga, ASTM A82, GALVANIZED LADDER-TYPE SHALL BE PROVIDED AT 16"O.C. UNLESS NOTED OTHERWISE.

5. CONCRETE MASONRY WALLS SHALL HAVE CONTROL JOINTS SPACED NO FURTHER THAN 25'-0" APART IN CONTINUOUS EXTERIOR AND INTERIOR WALLS.

6.) GROUT FOR FILLING BLOCK CORES SHALL CONFORM TO ASTM C476 WITH A MINIMUM COMPRESSIVE STRENGTH OF 3,000psi AT 28 DAYS. GROUT SHALL BE PLACED IN LIFTS NOT EXCEEDING 7 COURSES IN HEIGHT UNLESS OTHERWISE APPROVED BY THE ENGINEER.

7.) FULL HEAD AND BED JOINTS SHALL BE PROVIDED.

8.) COORDINATE LOCATION OF ALL MASONRY WALLS, PARTITIONS AND OPENINGS WITH ARCHITECTURAL DRAWINGS.

9.) LAP SPLICES IN VERTICAL REINFORCING FOR CONCRETE BLOCK MASONRY WALLS AND PILASTERS SHALL BE A MINIMUM OF 48 BAR DIAMETERS UNLESS DETAILED OTHERWISE ON THE DRAWINGS.

10.) ALL LINTELS AT MASONRY OPENINGS SHALL HAVE 8" OF BEARING AT EACH END. ALL EXTERIOR LINTELS SHALL BE HOT DIPPED GALVANIZED.

11.) REFERENCE ARCHITECTURAL DRAWINGS AND PROJECT SPECIFICATIONS FOR CONTROL JOINT LOCATION AND BOND PATTERN REQUIREMENTS. ALL BOND BEAM REINFORCEMENT TO BE CONTINUOUS AT CONTROL JOINT LOCATIONS. SCORE BOND BEAM SHELL 3/8" EACH SIDE AT CONTROL JOINTS.

12.) MASONRY COURSING INDICATED ON THE STRUCTURAL DRAWINGS IS APPROXIMATE. REFER TO ARCHITECTURAL DRAWINGS FOR ACTUAL COURSING LAYOUT.

13) REINFORCING STEEL AND INSERTS SHALL BE IN PLACE AND SECURE PRIOR TO PLACING GROUT.

14.) ALL MASONRY CELLS BELOW GRADE SHALL BE GROUTED SOLID.

15.) CONCRETE SHALL BE PROTECTED AGAINST FROST UNTIL SPECIFIED STRENGTH IS ATTAINED.

16.) SUBMIT SHOP DRAWINGS FOR REINFORCING STEEL, GROUT AND MORTAR MIX DESIGNS, CONTROL JOINT LOCATION PLAN AND CMU PRODUCT DATA TO THE ARCHITECT FOR REVIEW BEFORE CONSTRUCTION.

MISCELLANEOUS LINTEL SCHEDULE

WALL THICKNESS	MASONRY OPENINGS: UP TO 4'-0"	MASONRY OPENINGS: 4'-0" TO 8'-0"
4"	L4x3 1/2x5/16 LLV	L6x3 1/2x3/8 LLV
6"	WT4X10.5	WT5X11
8"	(2)-L4x3 1/2x5/16 LLV	(2)-L6x3 1/2x5/16 LLV
12"	(3)-L4x3 1/2x5/16 LLV	(3)-L6x3 1/2x5/16 LLV

MISC. LINTEL NOTES:
1.) LINTELS ARE FOR WALLS NOT SHOWN ON PLANS.
2.) LOCATION OF OPENINGS ARE SHOWN ON THE ARCHITECTURAL DRAWINGS.
3.) FOR OPENINGS LARGER THAN 8'-0", CONSULT STRUCTURAL ENGINEER.
4.) LINTELS SHALL HAVE A MINIMUM OF 8" BEARING AT EACH END.
5.) GALVANIZE LINTELS WITHIN EXTERIOR WALLS.

FOUNDATIONS:

1.) FOUNDATIONS HAVE BEEN DESIGNED IN BASED ON THE ASSUMED SOIL PROPERTIES INDICATED BELOW. THE CONTRACTOR SHALL TAKE ALL NECESSARY STEPS TO ENSURE THAT THE ASSUMED SOIL PARAMETERS ARE ACHIEVED.

2.) SOIL BEARING SURFACES SHALL BE DRY AND FREE FROM FROST OR LOOSE MATERIAL. EXISTING SOIL TO BE REMOVED AND REPLACED AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER. ALL SOIL BEARING SURFACES AND BACKFILL SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER OR OTHER APPROVED TESTING AGENT.

3.) EXISTING OBSTRUCTIONS SHALL BE REMOVED AND REPLACED WITH SUITABLE FILL AS REQUIRED TO PLACE THE NEW FOUNDATIONS.

4.) ASSUMED SOIL PARAMETERS ARE AS FOLLOWS:

ALLOWABLE BEARING PRESSURE........3000psf
MOD. OF SUBGRADE REACTION, k......175psi/in.
Ka..45pcf
Ko..60 pcf
Kp..400pcf
COEFFICIENT OF FRICTION, mu...........0.45

ALL NECESSARY MEASURES SHALL BE TAKEN TO ACHIEVE THESE SOIL PARAMETERS.

5.) CONTRACTOR SHALL VERIFY AND LOCATE ALL EXISTING BELOW GRADE UTILITIES PRIOR TO CONSTRUCTION.

6.) ALL FILL PLACED UNDER ALL FOUNDATIONS AND SLABS SHALL BE AS FOLLOWS:

A.) FLOWABLE FILL: NON-SWELLING WITH A 28-DAY STRENGTH OF 250psi.

B.) STRUCTURAL FILL/SUBBASE STONE: MEET NYSDOT 304.12, TYPE 2 WITH THE FOLLOWING GRADATION:

SIEVE SIZE	PERCENT FINER (WEIGHT)
2 in.	100
1/4 in.	25-60
40	5-40
#200	0-10

7.) BACKFILL AGAINST FOUNDATION WALLS AND FILL TO RAISE SITE GRADES BELOW THE STRUCTURAL FILL LAYER SHALL BE WELL GRADED AND FREE FROM DELETERIOUS MATERIAL APPROVED BY THE GEOTECHNICAL ENGINEER AND WITH THE FOLLOWING GRADATION:

SIEVE SIZE	PERCENT FINER (WEIGHT)
6in	100
#4	85
#200	20

ALTERNATIVELY, MATERIAL MEETING NYSDOT ITEM 203.07 CAN BE USED.

8.) EACH LIFT OF FILL SHALL BE COMPACTED TO AT LEAST 95% OF MAXIMUM DENSITY AS DETERMINED BY THE MODIFIED PROCTOR METHOD. THE DEPTH AND EXTENT OF FILL PLACEMENT SHALL BE AS RECOMMENDED BY THE GEOTECHNICAL ENGINEER AND GEOTECHNICAL ENGINEERING REPORT. FILL SHALL BE PLACED DIRECTLY ON STABLE NATURAL SOIL.

9.) ALL PIPING SLEEVES THROUGH FOUNDATION WALLS AND FOOTING STEPS TO ACCOMMODATE PIPING SHALL BE COORDINATED WITH THE PLUMBING CONTRACTOR/DRAWINGS.

10.) EXCEPT WHERE FIELD CONDITIONS REQUIRE AN APPROVED ADJUSTMENT, BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 4'-0" BELOW GRADE AND INTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-0" BELOW FINISHED FLOOR.

11.) BACKFILLING OF FOUNDATION WALLS IS NOT PERMITTED UNLESS WALLS ARE ADEQUATELY BRACED OR FILLING IS BALANCED.

12.) THE CONTRACTOR SHALL SUBMIT DRAWINGS SHOWING THE PROPOSED CONTROL AND CONSTRUCTION JOINTS FOR ALL AREAS FOR REVIEW PRIOR TO POURING ANY CONCRETE. THIS INCLUDES WALLS, TOPPINGS AND SLABS ON GRADE. JOINTS IN CONCRETE FOUNDATION WALLS SHALL BE LOCATED SO THAT NO SINGLE POUR IS LONGER THAN 40 FEET. A JOINT SHALL BE LOCATED WITHIN 3'-0" FROM ANY CORNER.

13.) PROVIDE PERIMETER DRAINAGE AND WATERPROOFING AS REQUIRED BY THE CIVIL AND ARCHITECTURAL DRAWINGS.

15.) THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS OR ALONG STEPPED FOOTINGS SHALL NOT EXCEED A RISE OF 1.0 IN A RUN OF 2.0.

16.) PROVIDE CONTROL JOINT FOR ALL SLABS-ON-GRADE AT COLUMN CENTERLINES AND AT A MAXIMUM OF 15'-0" O.C., UNLESS SHOWN OTHERWISE ON PLANS.

STEEL JOISTS:

1.) STEEL JOISTS AND JOIST GIRDERS SHALL CONFORM TO THE LATEST EDITION OF THE STANDARD SPECIFICATIONS FOR JOISTS AND JOIST GIRDERS PUBLISHED BY THE STEEL JOIST INSTITUTE.

2.) STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER. JOIST SUPPLIER TO PROVIDE CALCULATIONS AND DETAILS FOR EACH JOIST AND JOIST GIRDER. ALL CALCULATIONS SHALL BE SIGNED AND SEALED BY THE MANUFACTURERS ENGINEER AND SHALL BE SUBMITTED WITH THE SHOP DRAWINGS.

3.) THE MANUFACTURER SHALL DESIGN AND PROVIDE BRIDGING IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS, INCLUDING BRIDGING REQUIRED FOR UPLIFT.

4.) MINIMUM BEARING SHALL BE AS FOLLOWS:

LH-SERIES JOISTS: 4" ON STRUCTURAL STEEL
6" ON CONCRETE OR MASONRY

JOIST GIRDERS: 4" ON STRUCTURAL STEEL
6" ON CONCRETE OR MASONRY

5.) MINIMUM END ATTACHMENT SHALL BE AS FOLLOWS:

LH-SERIES JOISTS: (2)1/4"x2" FILLET WELDS OR WITH (2)-3/4" BOLTS

JOIST GIRDERS: (2)1/4"x2" FILLET WELDS OR WITH (2)-3/4" BOLTS

6.) UNLESS NOTED OTHERWISE, TOP CHORD EXTENSIONS SHALL BE DESIGNED FOR THE UNIFORM LOAD CAPACITY OF THE MAIN JOIST.

7.) UNLESS NOTED OTHERWISE, EQUIPMENT AND OTHER SUPERIMPOSED LOADS ARE IN ADDITION TO THE MAXIMUM UNIFORM LOADS GIVEN IN THE SJI STANDARD LOAD TABLES.

8.) LINES OF JOIST BRIDGING SHALL CONTINUE AND BE FASTENED TO BEAMS AND COLUMNS WHICH INTERSECT BRACING LINES.

9.) STEEL JOISTS AND JOISTS GIRDERS TO BE DESIGNED FOR A NET UPLIFT OF 10 psf.

10.) SEE STRUCTURAL STEEL NOTES FOR COATING REQUIREMENTS.

11.) SUBMIT JOIST CALCULATIONS AND SHOP DRAWINGS FOR REVIEW PRIOR TO CONSTRUCTION.

STRUCTURAL STEEL:

1.) STRUCTURAL STEEL SHALL CONFORM TO THE LATEST EDITION OF THE AISC CODE OF STANDARD PRACTICE.

2.) STRUCTURAL STEEL GRADES (UNLESS NOTED OTHERWISE):
A) STRUCTURAL STEEL (W-): ASTM A572 (ASTM A992), Fy=50ksi
B) STRUCTURAL STEEL (C-, L-, PLATES): ASTM A36, Fy = 36ksi
C) STRUCTURAL TUBING: ASTM A500, Fy=46ksi
D) STRUCTURAL STEEL PIPE: ASTM A53 GRADE B, Fy=35ksi
E) BOLTS: ASTM A325N, PRETENSIONED
i.) BOLTS IN BRACED FRAME CONNECTIONS ARE TO BE SLIP CRITICAL
F) ANCHOR RODS: F1554, GRADE 36
G) SHEAR CONNECTORS: ASTM A108
H) WELDS: E70xx
I) GALVANIZING:
i.) STRUCTURAL STEEL: ASTM A123
ii.) BOLTS, FASTENERS, HARDWARE: ASTM A153
J) ADHESIVE ANCHORS (BASIS OF DESIGN):
I.) ACRYLIC: HILTI HY200 OR POWERS AC100 PLUS
ii.) EPOXY: HILTI RE500 OR POWERS PE1000+
ADHESIVE ANCHORS SHALL BE LOCATED AWAY FROM EXISTING REBAR. ALL ANCHORS ARE ASTM A193 B7.
K) MECHANICAL ANCHORS (BASIS OF DESIGN):
i.) HEAVY DUTY SCREW ANCHOR: HILTI KWIK HUS
ii.) WEDGE ANCHORS: HILTI KWIK BOLT III OR POWERS POWER STUD

3.) WELDS NOT INDICATED FOR STEEL-TO-STEEL CONNECTIONS SHALL BE AN ALL AROUND FILLET WELD WITH A MINIMUM THROAT THICKNESS PER AISC AND AWS STANDARDS.

4.) ALL STRUCTURAL STEEL SHALL BE COATED AS INDICATED BELOW. APPLY COATINGS IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS, INCLUDING SURFACE PREPARATIONS AND COMPATIBILITY REQUIREMENTS BETWEEN ALL COATINGS. AFTER ERECTION TOUCH UP ALL AREAS WHERE PAINT OR GALVANIZING IS MISSING OR DAMAGED INCLUDING FIELD WELDS. COLOR SHALL BE AS DETERMINED BY THE OWNER OR ARCHITECT.

A.) EXTERIOR EXPOSED STEEL AND LINTELS WITHIN EXTERIOR WALLS TO BE HOT-DIPPED GALVANIZED.

B.) ALL OTHER STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH A MODIFIED ALKYD RUST INHIBITIVE PRIMER, 2.5 TO 3.5 MILS DFT (BASIS OF DESIGN IS TNEMEC SERIES 10).

C.) THE TOP FLANGE OF BEAMS THAT RECEIVE DECKING SHALL NOT BE COATED. CONNECTION MATERIAL AT SLIP CRITICAL JOINTS TO BE UNPRIMED AND SURFACES PREPARED WITH A WIRE BRUSH (SSPC-SP2).

5.) BEAM CONNECTIONS SHALL BE DESIGNED FOR THE LARGEST OF THE FOLLOWING: END REACTIONS COMPUTED FROM THE TABLE "UNIFORM LOAD CONSTANTS" IN THE AISC MANUAL, THE REACTIONS SHOWN ON THE PLANS, OR A SERVICE LOAD OF 10k.

6.) ALL OTHER CONNECTIONS SHALL BE DESIGNED TO TRANSFER THE FULL DESIGN STRENGTH OF THE MEMBERS IN QUESTION, UNLESS SPECIFICALLY DETAILED OTHERWISE.

7.) ALL CONNECTIONS AND DETAILS NOT SPECIFICALLY INDICATED ON THE DRAWINGS ARE SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND ENGINEER.

8.) UNLESS NOTED OTHERWISE, PROVIDE A 1/4" BENT PLATE AT THE PERIMETER OF ALL ROOF AND FLOOR EDGES AND OPENINGS.

9.) UNLESS INDICATED OTHERWISE, PROVIDE A 1/4" CAP PLATE AT THE ENDS OF HSS AND PIPE MEMBERS. IF VENT HOLES ARE USED FOR GALVANIZING, SEAL THE HOLES AFTER GALVANIZING.

10.) SUBMIT SHOP DRAWINGS FOR STRUCTURAL STEEL FOR REVIEW PRIOR TO CONSTRUCTION.

STEEL DECK:

1.) STEEL DECK CONSTRUCTION SHALL CONFORM TO THE THE LATEST MANUAL OF CONSTRUCTION AND CODE OF STANDARD PRACTICE FOR STEEL DECK PUBLISHED BY THE STEEL DECK INSTITUTE.

2.) STEEL DECK SHALL BE CONTINUOUS OVER 3 OR MORE SPANS UNLESS NOTED OTHERWISE.

3.) STEEL DECK SHALL BE GALVANIZED, G60.

4.) PERMANENT STEEL FORM DECK FASTENING SHALL CONFORM TO THE SDI RECOMMENDATIONS FOR FASTENING TO SUPPORTING MEMBERS.

5.) COMPOSITE STEEL DECK FASTENING SHALL CONFORM TO THE SDI RECOMMENDATIONS FOR FASTENING TO SUPPORTING MEMBERS.

6.) STEEL ROOF DECK SHALL BE FASTENED TO SUPPORTS AT 12" O.C. MAXIMUM (6" O.C. @ PERIMETER) WITH 5/8" PUDDLE WELDS. SIDE LAPS SHALL BE FASTENED WITH #10 TEK SCREWS AT 18" O.C. OR 3 PER SPAN, WHICHEVER IS GREATER.

7.) ALTERNATE DECK FASTENING MAY BE SUBMITTED FOR REVIEW. CONTRACTOR TO SUBMIT ENGINEERING DATA INDICATING EQUIVALENCE OF THE ALTERNATE FASTENING TO THAT WHICH IS SPECIFIED.

8.) PROVIDE WELD WASHERS FOR FASTENING AS RECOMMENDED BY THE MANUFACTURER.

9.) THE CONTRACTOR SHALL VERIFY THE SIZE AND LOCATION OF ALL DECK OPENINGS REQUIRED FOR PENETRATIONS AND SHALL PROVIDE SUMP PANS OR ADDITIONAL FRAMING AS REQUIRED. HOLES SHALL BE CUT NEATLY WITH A MAXIMUM SIZE OF PIPE OR DUCT DIMENSION PLUS 1". A STEEL FRAMING ASSEMBLY IS REQUIRED AT ALL OPENINGS EXCEEDING 6".

10.) SUBMIT SHOP DRAWINGS FOR STEEL DECK PRODUCT DATA, DECK LAYOUT, AND DECK FASTENERS FOR REVIEW PRIOR TO CONSTRUCTION.

REVISIONS:

MARK	DESCRIPTION	DATE

CLIENT/OWNER:
PLATINUM PROPERTY MANAGEMENT
P.O. BOX 26350
ROCHESTER, NY 14626

PROJECT TITLE:
GATEWAY BUSINESS PARK
PINEWILD DRIVE
GREECE, NY

SCALE: AS NOTED
DATE: 9/3/2015
DRAWN BY: RF
ENGINEER: JRB

JensenBRV ENGINEERING, PLLC
Structural Engineering Consultants
1653 EAST MAIN STREET
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phone: (585) 402-8130
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SHEET NO.:
S001

PROJECT NO.:

COLD-FORMED STEEL:

1.) ALL LIGHT GAGE FRAMING SHALL CONFORM TO THE 2007 NORTH AMERICAN SPECIFICATION (NAS-2007) FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.

2.) MEMBERS TO HAVE MINIMUM YIELD STRESS OF 50ksi AND SHALL HAVE A MINIMUM FLANGE WIDTH OF 1 5/8" AND A RETURN LIP LENGTH OF 1/2". WHERE FLANGE WIDTHS OF 2" ARE SPECIFIED, THE RETURN LIP LENGTH SHALL BE 5/8", WHERE FLANGE WIDTHS OF 3" ARE SPECIFIED, THE RETURN LIP LENGTH SHALL BE 5/8".

3.) SIZES AND GAGE SHOWN ON THE DRAWINGS ARE MINIMUM.

4.) ALL STEEL TO BE GALVANIZED, G60 IN ACCORDANCE WITH ASTM A924.

5.) PROVIDE ALL ACCESSORIES, SUCH AS TRACKS, CLIPS, WEB STIFFENERS, BRIDGING, AND CONNECTORS REQUIRED FOR A COMPLETE INSTALLATION AND AS RECOMMENDED BY THE MANUFACTURER.

6.) MINIMUM TRACK GAGE IS 54 MIL. MINIMUM STUD GAGE IS 54 MIL UNLESS NOTED.

7.) BUILT UP MEMBERS SHALL BE FASTENED TOGETHER TO CREATE COMPOSITE MEMBERS PER AISI 2007.

8.) STRUCTURAL PERFORMANCE: PROVIDE COLD-FORMED STEEL MEMBERS CAPABLE OF WITHSTANDING DESIGN LOADS WITHIN LIMITS AND UNDER CONDITIONS INDICATED.

A.) DESIGN LOADS: AS INDICATED ON THE DRAWINGS AND LOADS REQUIRED BY THE GOVERNING BUILDING CODE FOR THE PROPOSED OCCUPANCY.

B.) DEFLECTION LIMITS: DESIGN FRAMING SYSTEMS TO WITHSTAND DESIGN LOADS WITH DEFLECTIONS NOT GREATER THAN THE FOLLOWING:

i.) EXTERIOR NON- LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/600 OF THE WALL HEIGHT SUPPORTING BRICK.

ii.) EXTERIOR NON- LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/600 OF THE WALL HEIGHT SUPPORTING MATERIAL OTHER THAN BRICK.

iii.) INTERIOR LOAD-BEARING WALL FRAMING: HORIZONTAL DEFLECTION OF 1/360 OF THE WALL HEIGHT UNDER A HORIZONTAL LOAD OF 5psf.

9.) SUBMIT SHOP DRAWINGS AND CALCULATIONS STAMPED BY AN ENGINEER LICENSED IN THE STATE OF NEW YORK FOR LIGHT GAGE FRAMING FOR REVIEW PRIOR TO CONSTRUCTION.

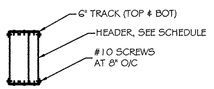
10.) WELD COLD-FORMED STEEL IN ACCORDANCE WITH AWS D1.3 FOR WELDING SHEET STEEL IN STRUCTURES. CONSULT AWS D19.0, WELDING ZINC COATED STEEL, AND ANSI STANDARD Z49.1 FOR INFORMATION REGARDING SAFE WELDING PROCEDURES. WELDS SHALL BE TOUCHED-UP WITH A ZINC-RICH RUST-INHIBITIVE PRIMER.

MISCELLANEOUS HEADER SCHEDULE

LOCATION	ROUGH OPENING	HEADER MEMBERS	JACK STUDS	KING STUDS
INTERIOR NON-LOAD BRG	0 TO 6'-0"	(2) 800S162-54 # 600T200-43	(1) 6" 43 MIL, (1) 3 5/8" 43 MIL	(1) 6" 43 MIL, (1) 3 5/8" 43 MIL
EXTERIOR NON-LOAD BRG	0 TO 6'-0"	(2) 1000S200-54 # 600T200-54	(1) 6" 43 MIL, (1) 3 5/8" 43 MIL	(3) 6" 43 MIL, (2) 3 5/8" 43 MIL

HEADER NOTES:

1.) PROVIDE SCHEDULED HEADERS UNLESS SIZE INDICATED ON PLANS.
2.) LOCATION OF OPENINGS ARE SHOWN ON THE ARCHITECTURAL DRAWINGS.
3.) FOR OPENINGS LARGER THAN 6'-0", CONSULT STRUCTURAL ENGINEER.
4.) HEADERS SHALL HAVE MINIMUM JACK STUDS NOTED EACH END. JACK STUDS SHALL CONTINUE TO FOUNDATION FROM SECOND FLOOR OPENINGS.
5.) REFER TO ARCHITECTURAL DRAWINGS FOR STUD SIZES AT INTERIOR WALLS.
6.) PROVIDE WEB BEARING STIFFENERS AT EACH END, EACH PLY.
7.) HEADERS SHALL BE FABRICATED FROM UNPUNCHED MATERIAL.



SPECIAL INSPECTIONS:

1.) SPECIAL INSPECTIONS SHALL CONFORM TO CHAPTER 17 OF "BUILDING CODE OF NEW YORK STATE", 2010.

2.) SPECIAL INSPECTIONS AND TESTING REQUIREMENTS INDICATED IN THIS SECTION ARE THE OWNER'S RESPONSIBILITY. OWNER WILL ENGAGE QUALIFIED SPECIAL INSPECTORS AND TESTING AGENCIES TO PERFORM THESE SERVICES.

3.) CONTRACTOR RESPONSIBILITY:

A.) PREPARE SAMPLES FOR TESTING BY THE OWNER'S TESTING AGENCY WHERE INDICATED AND AS DIRECTED BY THE OWNER'S SPECIAL INSPECTOR.

B.) SUBMIT MANUFACTURER AND FABRICATOR CERTIFICATIONS AND RELATED DOCUMENTATION SPECIFIED OR REQUESTED BY THE SPECIAL INSPECTOR.

C.) CORRECT DEFICIENT CONSTRUCTION AS REQUIRED BY THE INSPECTOR AND CONTRACT DOCUMENTS.

D.) COSTS FOR RETESTING AND RE-INSPECTING CONSTRUCTION THAT REPLACES OR IS NECESSITATED BY WORK THAT FAILED TO COMPLY WITH THE CONTRACT DOCUMENTS ARE TO BE BORNE BY THE CONTRACTOR.

4.) DEFICIENCIES FOUND BY THE SPECIAL INSPECTOR SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE ARCHITECT.

5.) THE SPECIAL INSPECTOR SHALL PROVIDE REPORTS AS REQUIRED BY CHAPTER 17 OF THE BUILDING CODE OF NEW YORK.

6.) SPECIAL INSPECTIONS SHALL BE AS INDICATED IN THE SCHEDULE BELOW:

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
SOILS:		
1. VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	PERIODIC	GEOTECHNICAL ENGINEERING REPORT AND NYSBC 1704.7
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	PERIODIC	
3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	PERIODIC	
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	CONTINUOUS	
5. PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT THE SITE HAS BEEN PROPERLY PREPARED.	PERIODIC	
CONCRETE:		
1. INSPECTION OF REINFORCING STEEL, SIZE AND PLACEMENT	PERIODIC	ACI 318: 7.1-7.7
2. INSPECT BOLTS TO BE INSTALLED IN CONCRETE, PRIOR TO AND DURING PLACEMENT OF CONCRETE.	PERIODIC	NYSBC 1912.5
3. VERIFYING USE OF REQUIRED DESIGN MIX	PERIODIC	ACI 318: Ch. 4, 5.2-5.4
4. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP, TEMP. AND AIR CONTENT TESTS.	CONTINUOUS	ASTM C 172; ASTM C 31; ACI 318: 5.6, 5.8
5. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	CONTINUOUS	ACI 318: 5.9, 5.10
STEEL CONSTRUCTION:		
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS, HIGH-STRENGTH BOLTING:		APPLICABLE ASTM MATERIAL SPECIFICATIONS: AISC 335 Sec. A3.4; AISC LRFD SECTION A3.3
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	PERIODIC	
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	PERIODIC	
2. INSPECTION OF BEARING-TYPE CONNECTIONS	PERIODIC	AISC LRFD Sec. M2.5
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL:		ASTM A-6 OR ASTM A-568
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	---	
b. MANUFACTURER'S CERTIFIED MILL TEST REPORTS REQUIRED	---	
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:		AISC ASD Sec. A3.6
a. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS	---	
b. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED	---	
5. INSPECTION OF WELDING:		AWS D1.1
a. STRUCTURAL STEEL:		
i. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	CONTINUOUS	
ii. MULTI-PASS FILLET WELDS	CONTINUOUS	
iii. SINGLE-PASS FILLET WELDS > 5/16"	CONTINUOUS	
iv. SINGLE-PASS FILLET WELDS < 5/16"	PERIODIC	
v. FLOOR AND ROOF DECK WELDS	PERIODIC	AWS D1.3
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS:	PERIODIC	NYSBC 1704.3.2
a. DETAILS SUCH AS BRACING AND STIFFENING		
b. MEMBER LOCATIONS		
c. APPLICATION OF JOINT DETAILS AT EACH CONNECTION		
7. INSPECTION OF FABRICATORS: VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES. EXCEPTION: FABRICATORS THAT ARE APPROVED IN ACCORDANCE WITH 1704.2.2	---	NYSBC 1704.2

SPECIAL INSPECTION	FREQUENCY	REFERENCED STANDARD
MASONRY CONSTRUCTION:		
1. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:		ACI 530/ASCE 6/TMS 602-ART. 2.6A, ART. 3.3B, ART. 3.4, ART. 3.6A
a. PROPORTIONS OF SITE-PREPARED MORTAR.	PERIODIC	
b. CONSTRUCTION OF MORTAR JOINTS.	PERIODIC	
c. LOCATION OF REINF. AND CONNECTORS.	PERIODIC	
2. THE INSPECTION PROGRAM SHALL VERIFY:		
a. SIZE AND LOCATIONS OF STRUCTURAL ELEMENTS.	PERIODIC	ART. 3.3G
b. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	PERIODIC	ACI 530/ASCE 5/TMS 402-SEC. 1.2.2(e), 2.1.4, 3.1.6, 1.12, 2.1.10.6.2, 3.2.3.4(b)
c. SPECIFIED SIZE, GRADE AND TYPE OF REINF.	PERIODIC	
d. WELDING OF REINFORCING BARS.	CONTINUOUS	
e. PROTECTION OF MASONRY DURING COLD OR HOT WEATHER.	PERIODIC	NYSBC 2104.3 AND 2104.4
3. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:		
a. GROUT SPACE IS CLEAN.	PERIODIC	ACI 530/ASCE 6/TMS 602-ART. 3.2D, ART. 3.4, SEC. 1.12, ART. 2.6B, ART. 3.5B
b. PLACEMENT OF REINF. AND CONNECTORS.	PERIODIC	
c. PROPORTIONS OF SITE PREPARED GROUT.	PERIODIC	
d. CONSTRUCTION OF MORTAR JOINTS.	PERIODIC	
4. GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENTS:	CONTINUOUS	ACI 530/ASCE 6/TMS 602-ART. 3.5
5. PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	CONTINUOUS	ACI 530/ASCE 6/TMS 602-ART. 1.4, AND NYSBC SEC. 2105.2.2 AND 2105.3
6. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	PERIODIC	ACI 530/ASCE 6/TMS 602-ART. 1.5.
COLD-FORMED STEEL: (SEE STRUCTURAL STEEL FOR GENERAL REQUIREMENTS)		
FRAMING MEMBERS: SIZE, SPACING, STITCH CONNECTORS, ORIENTATION, AND ALIGNMENT	PERIODIC	AISI S100-2004
SELF-TAPPING SCREWS FOR BEARING AND SHEAR WALLS: SIZE, QUANTITY, SPACING, EDGE DISTANCE, AND LOCATION	PERIODIC	AISI S100-2004
SHOP AND FIELD WELDED CONNECTIONS (BURN THROUGH WILL BE CAUSE FOR REJECTION)	PERIODIC	AWS D1.3
POST-INSTALLED ANCHORS:		
1. DURING PLACEMENT OF ANCHORS AS SPECIFIED ON THE CONSTRUCTION DOCUMENTS IN MASONRY AND CONCRETE:		
a. SIZE AND EMBEDMENT OF ANCHORS	CONTINUOUS	MANUFACTURERS INSTALLATION INSTRUCTIONS
b. ANCHORS INSTALLED PER MANUFACTURERS RECOMMENDATIONS.	CONTINUOUS	

REVISIONS:

MARK	DESCRIPTION	DATE
1		
2		
3		
4		
5		

CLIENT/OWNER:
PLATINUM PROPERTY MANAGEMENT
P.O. BOX 26350
ROCHESTER, NY 14626

PROJECT TITLE:
GATEWAY BUISINESS PARK
PINEWILD DRIVE
GREECE, NY

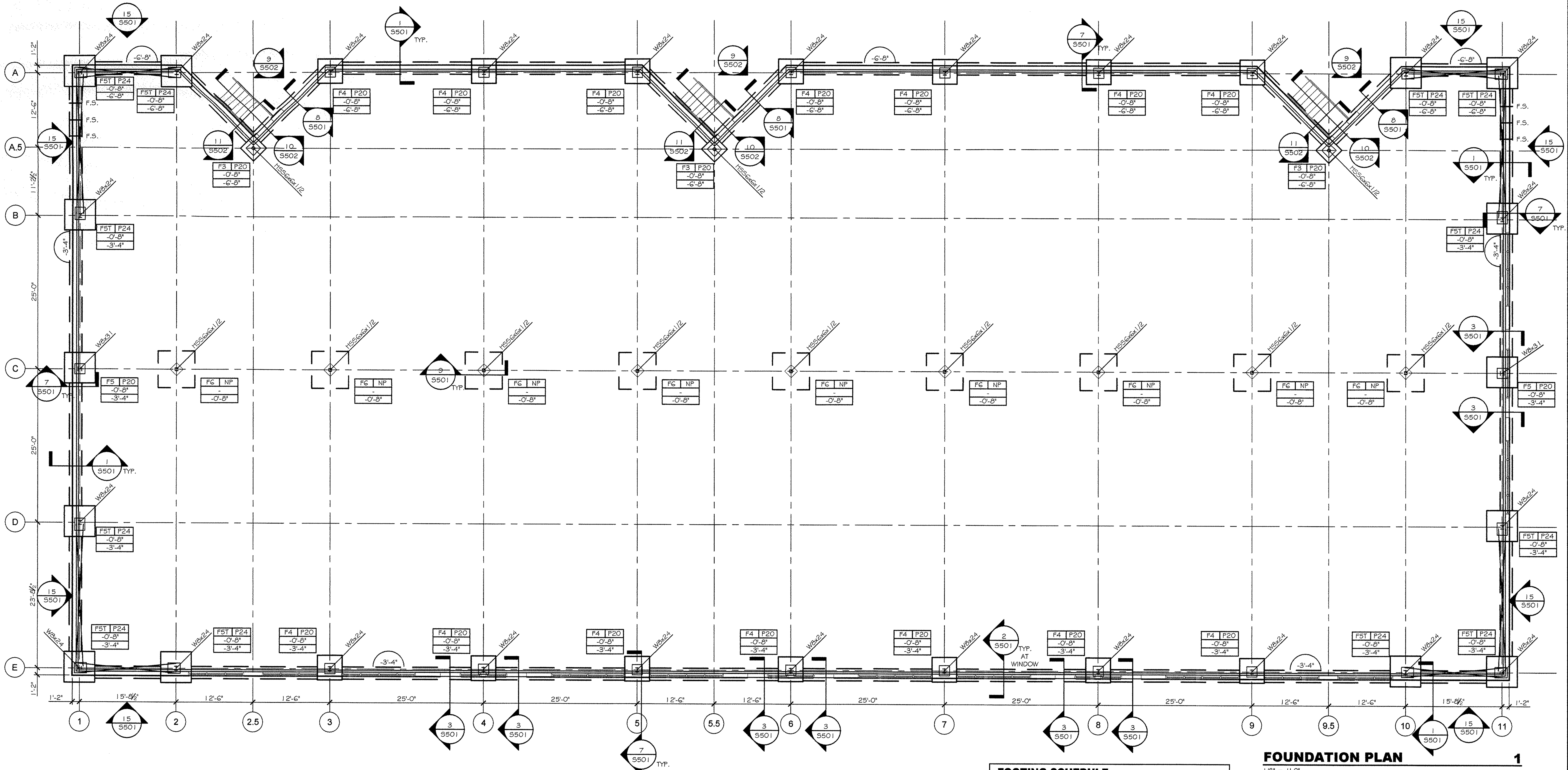
SCALE: AS NOTED
DATE: 9/3/2015
DRAWN BY: RF
ENGINEER: JRB

JensenBRV ENGINEERING, PLLC
Structural Engineering Consultants
1653 EAST MAIN STREET
ROCHESTER, NEW YORK 14609
phone: (585) 482-8130
fax: (585) 482-0440
web site: www.jensenbrv.com

STATE OF NEW YORK * JOHN R. BAUMLER * LICENSED * 075853 * PROFESSIONAL ENGINEER

SHEET NO.:
S002

PROJECT NO.:



FOOTING SCHEDULE			
MARK	DIMENSIONS	DEPTH	REINFORCING
F4	4'-0"x4'-0"	1'-0"	(5)-#5 x 3'-6" EA. WAY BOT.
F5	5'-0"x5'-0"	1'-0"	(6)-#5 x 4'-6" EA. WAY BOT.
F5T	5'-0"x5'-0"	1'-0"	(6)-#5 x 4'-6" EA. WAY TOP& BOT.
F6	6'-0"x6'-0"	1'-4"	(7)-#6 x 5'-6" EA. WAY BOT.

PIER SCHEDULE		
MARK	DIMENSIONS	REINFORCING
P20	1'-8"x1'-8"	(6)-#6's VERTICAL, w/ #3 TIES @ 12"O.C.
P24	1'-8"x2'-0"	(8)-#6's VERTICAL, w/ #3 TIES @ 12"O.C.

FOUNDATION PLAN 1

1/8" = 1'-0"

1. DATUM IS FINISHED FLOOR = +0'-0".
2. SLAB ON GRADE IS 5" THICK REINFORCED WITH 6x6-W2.9xW2.9 WWF TO BE INSTALLED AFTER COORDINATION WITH TENANT REQUIREMENTS.
3. CONTROL JOINTS TO BE ON COLUMN LINES AND A MAXIMUM OF 15'-0" o.c.

LEGEND

FOOTING MARK (SEE FOOTING SCHEDULE)
PIER MARK (SEE PIER SCHEDULE)
F3 | P24
-0'-8"
-4'-0"
TOP OF PIER ELEVATION (REFERENCE FROM +0'-0")
TOP OF FOOTING ELEVATION (REFERENCE FROM +0'-0")
HSS8x8
COLUMN SIZE

X'-X" = TOP OF FOOTING ELEVATION
F.S. = FOOTING STEP - (SEE DETAIL 10/S501)
C.J. = CONTROL/CONSTRUCTION JOINTS (SEE DETAILS 11, 12 AND 14 ON S501)
N.P. = NO PIER

REVISIONS:

MARK	DESCRIPTION	DATE
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△		

CLIENT/OWNER:
PLATINUM PROPERTY MANAGEMENT
P.O. BOX 26350
ROCHESTER, NY 14626

PROJECT TITLE:
GATEWAY BUSINESS PARK
PINEWILD DRIVE
GREECE, NY

SCALE: AS NOTED
DATE: 9/3/2015
DRAWN BY: RF
ENGINEER: JRB

Jensen BRV ENGINEERING, PLLC
Structural Engineering Consultants
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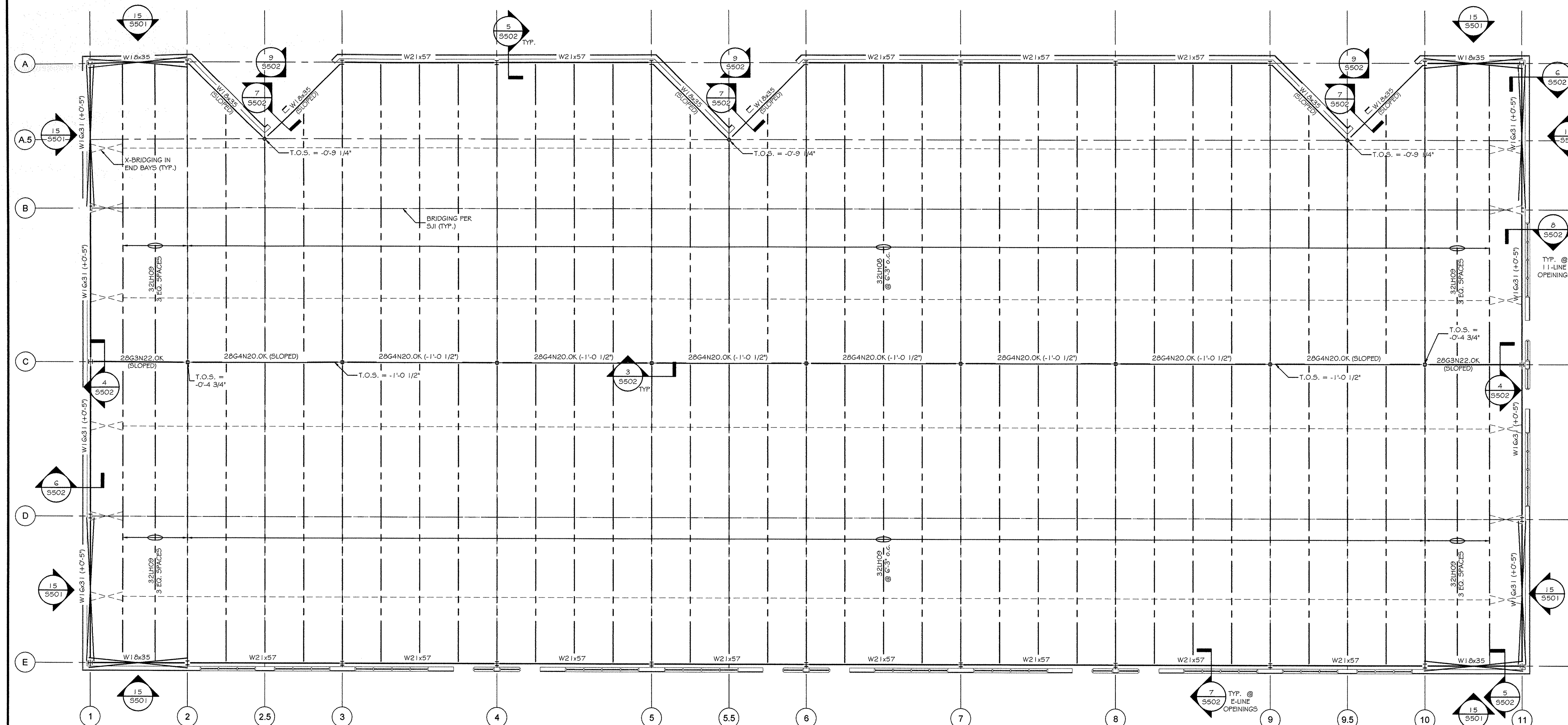
STATE OF NEW YORK · JOHN R. BAUMLER · LICENSED PROFESSIONAL ENGINEER

SHEET NO.:
S101

PROJECT NO.:



ROOF FRAMING PLAN 1

1/8" = 1'-0"

1. TOP OF STEEL (JOIST BEARING) ELEVATION IS REFERENCED FROM FINISHED GROUND FLOOR ELEVATION = 17'-0" = +0'-0".

2. ROOF DECK IS 1½"-20 GA. CONTINUOUS OVER THREE OR MORE SPANS.

3. JOIST AND JOIST GIRDER DESIGN METHOD IS "ASD".

4. SEE S503 FOR MISCELLANEOUS COLD FORMED METAL FRAMING DETAILS.

DRAWING ALTERATION

THE FOLLOWING IS AN EXCERPT FROM THE NEW YORK EDUCATION LAW, ARTICLE 145, SECTION 7209, SPECIAL PROVISIONS, AND APPLIES TO THIS DRAWING.

"IT IS A VIOLATION OF THIS LAW FOR ANY PERSON, UNLESS HE OR SHE IS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE SEAL OF AN ENGINEER IS ALTERED, THE ALTERING ENGINEER SHALL AFFIX TO THE ITEM HIS OR HER SEAL AND THE NOTATION "ALTERED BY" FOLLOWED BY HIS OR HER SIGNATURE AND THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION".

REVISIONS:

MARK	DESCRIPTION	DATE
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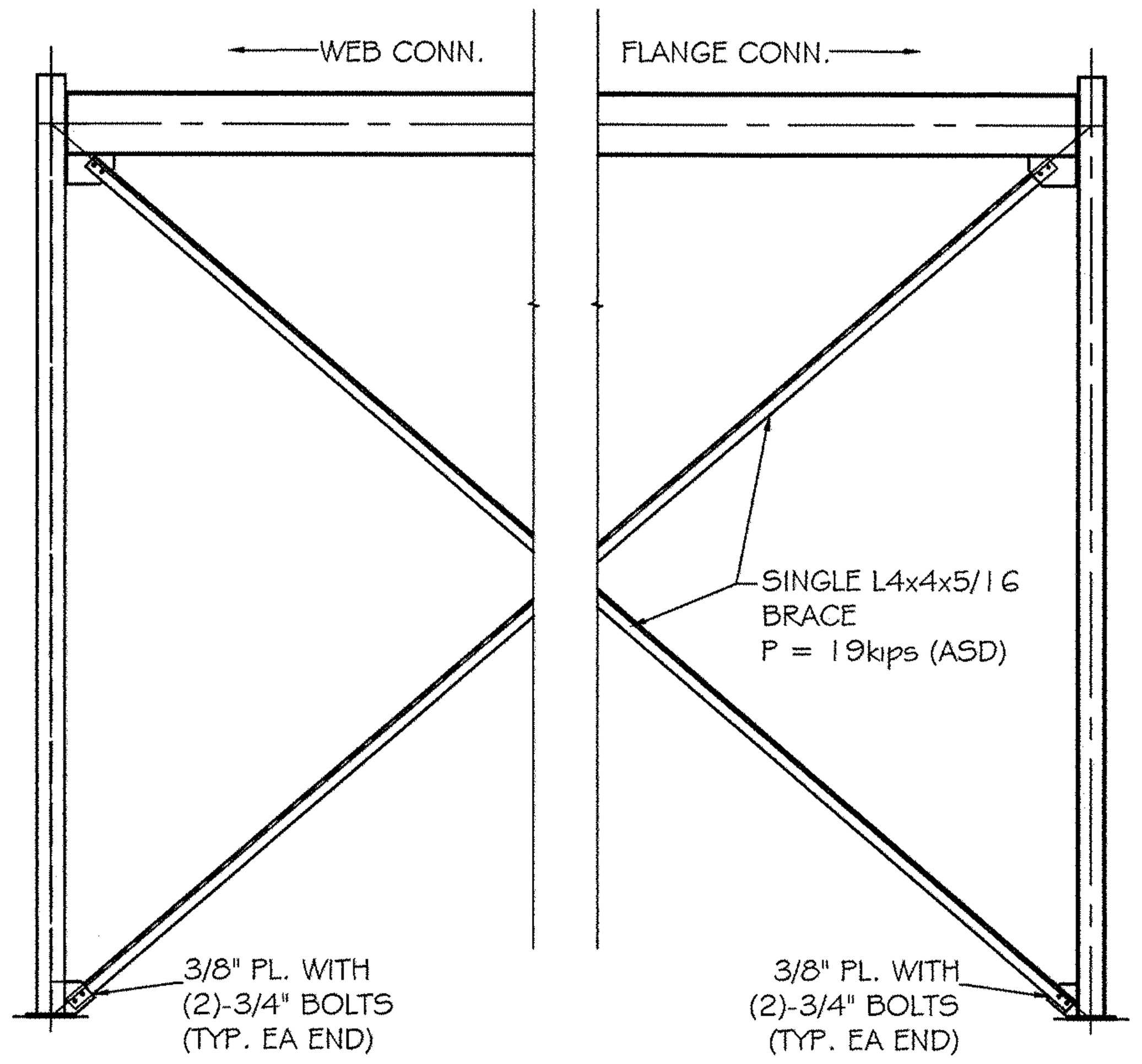
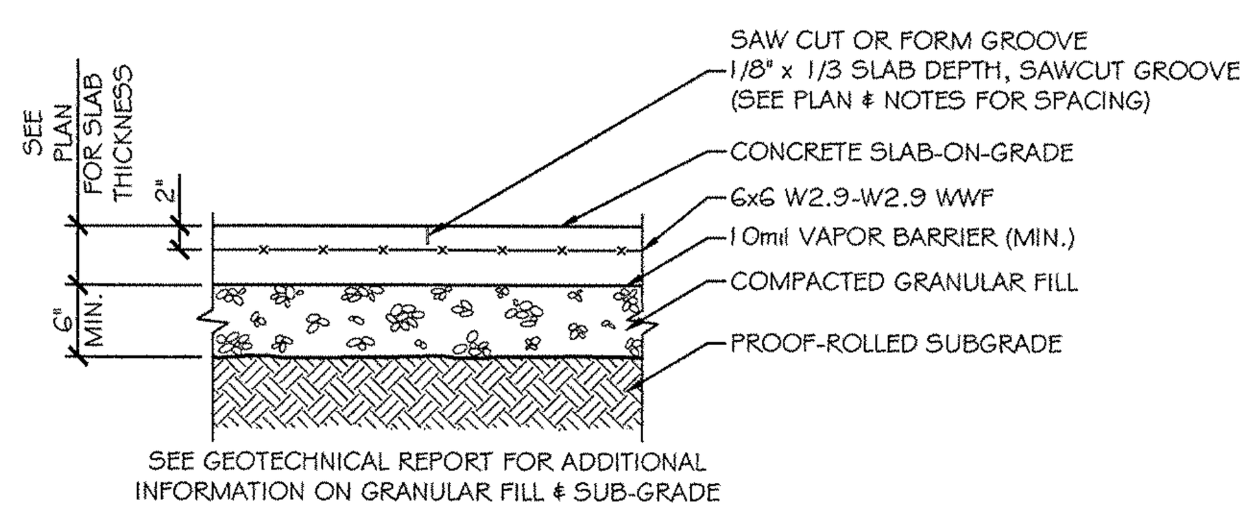
STATE OF NEW YORK * JOHN R. BAUMLER * LICENSED PROFESSIONAL ENGINEER 079855

SHEET NO.:
S102

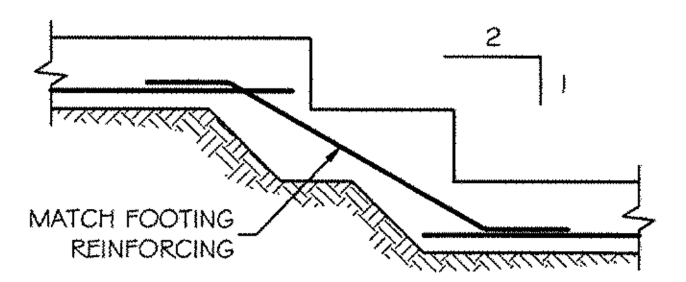
PROJECT NO.:



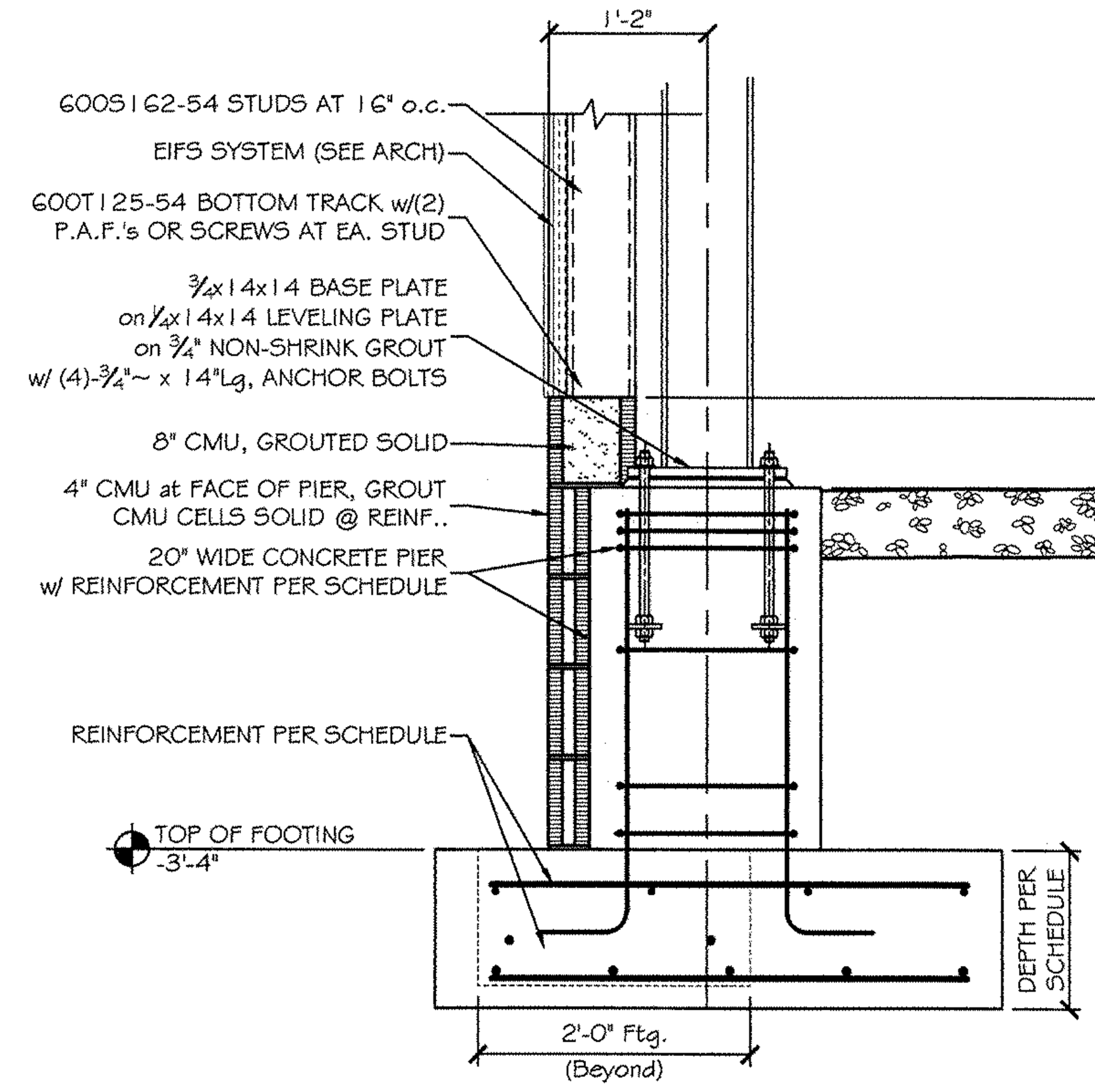
ELEVATION 15
1/4" = 1'-0"



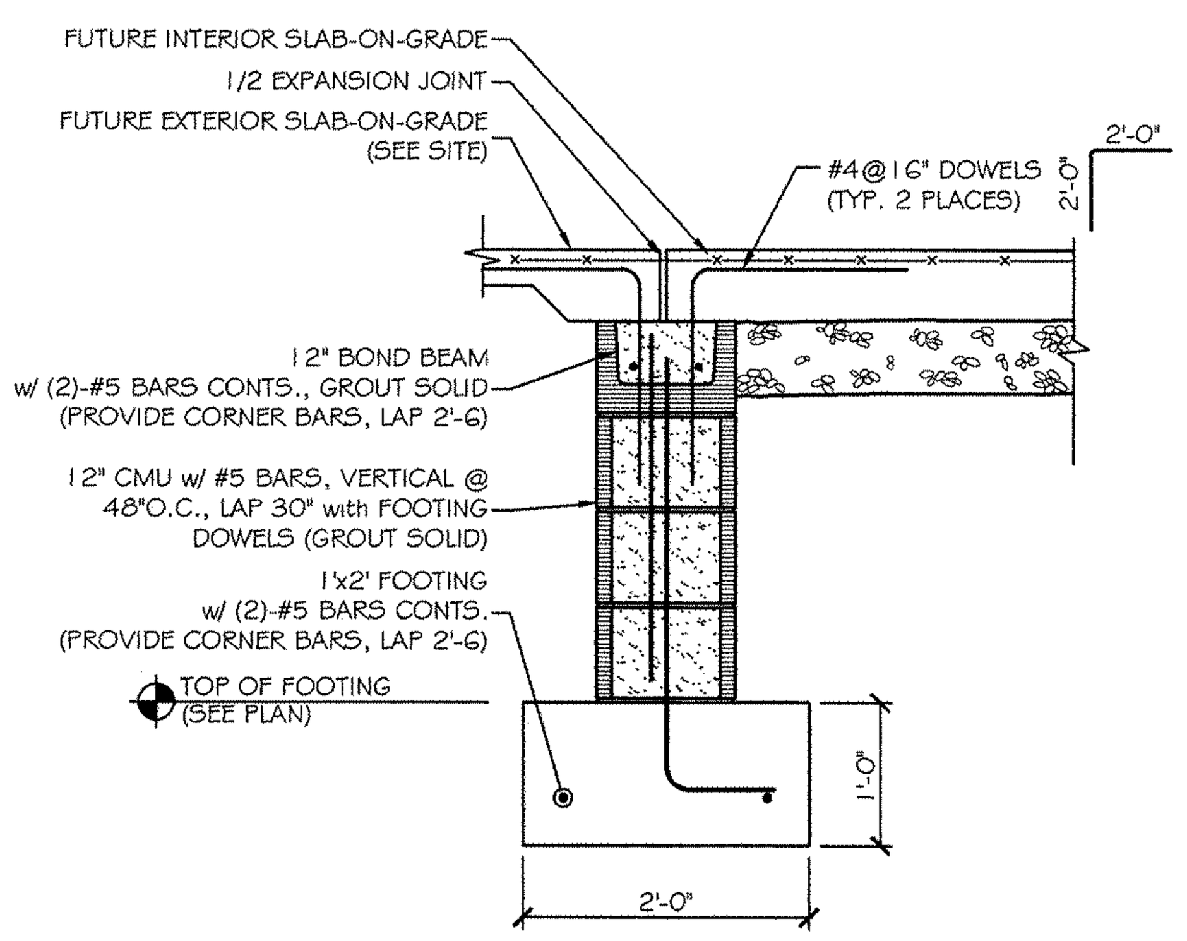
FUTURE SLAB-ON-GRADE DETAIL 11
3/4" = 1'-0"



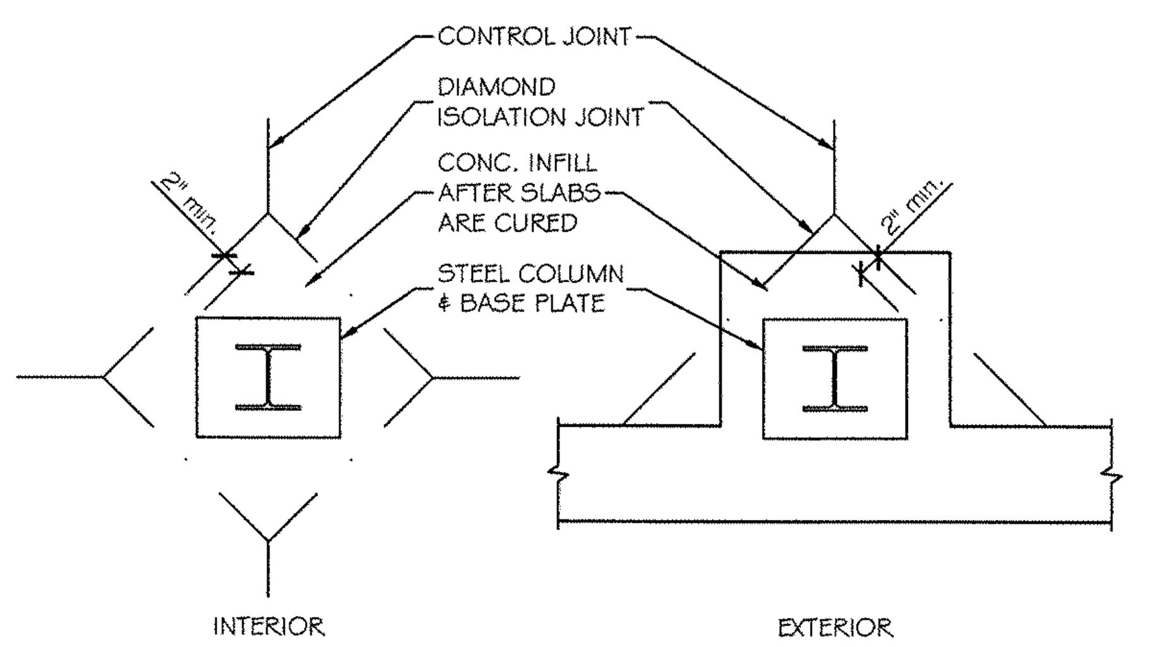
STEP FOOTING DETAIL 10
3/4" = 1'-0"



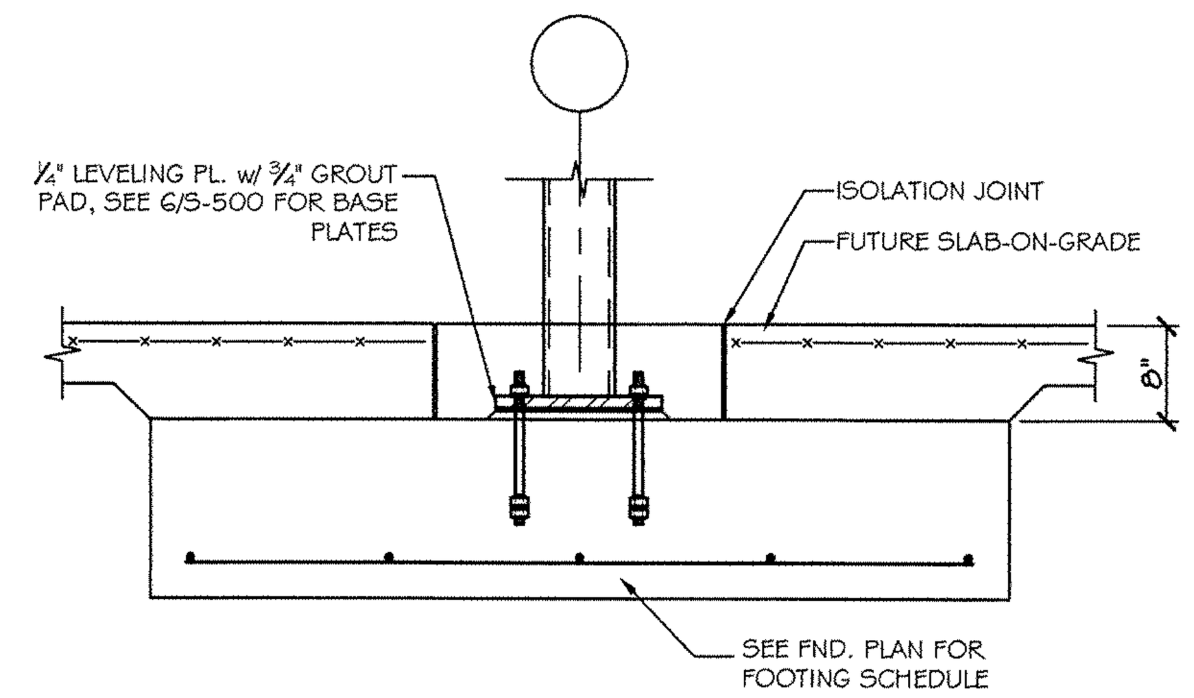
TYPICAL EXTERIOR COLUMN PIER 7
SCALE: 3/4" = 1'-0"



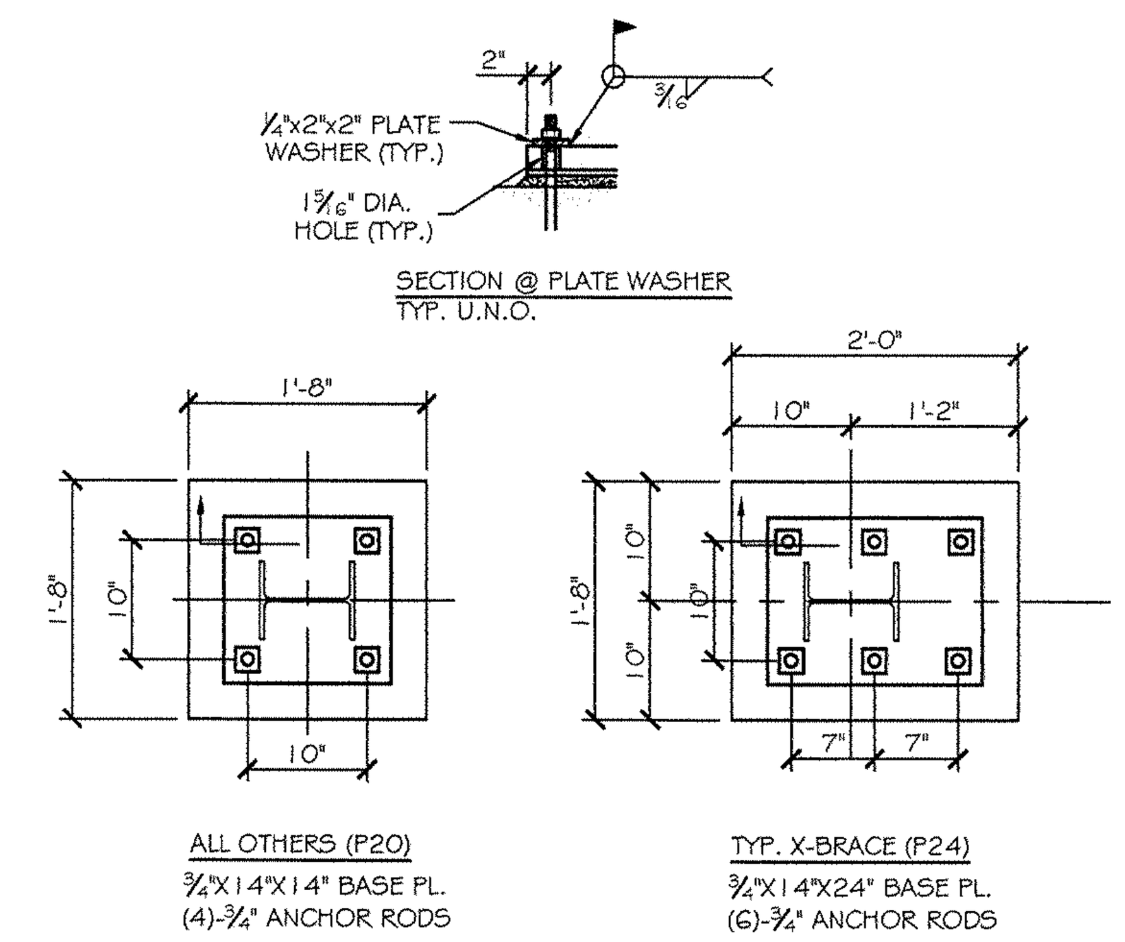
TYPICAL SECTION AT ENTRY DOORS 3
3/4" = 1'-0"



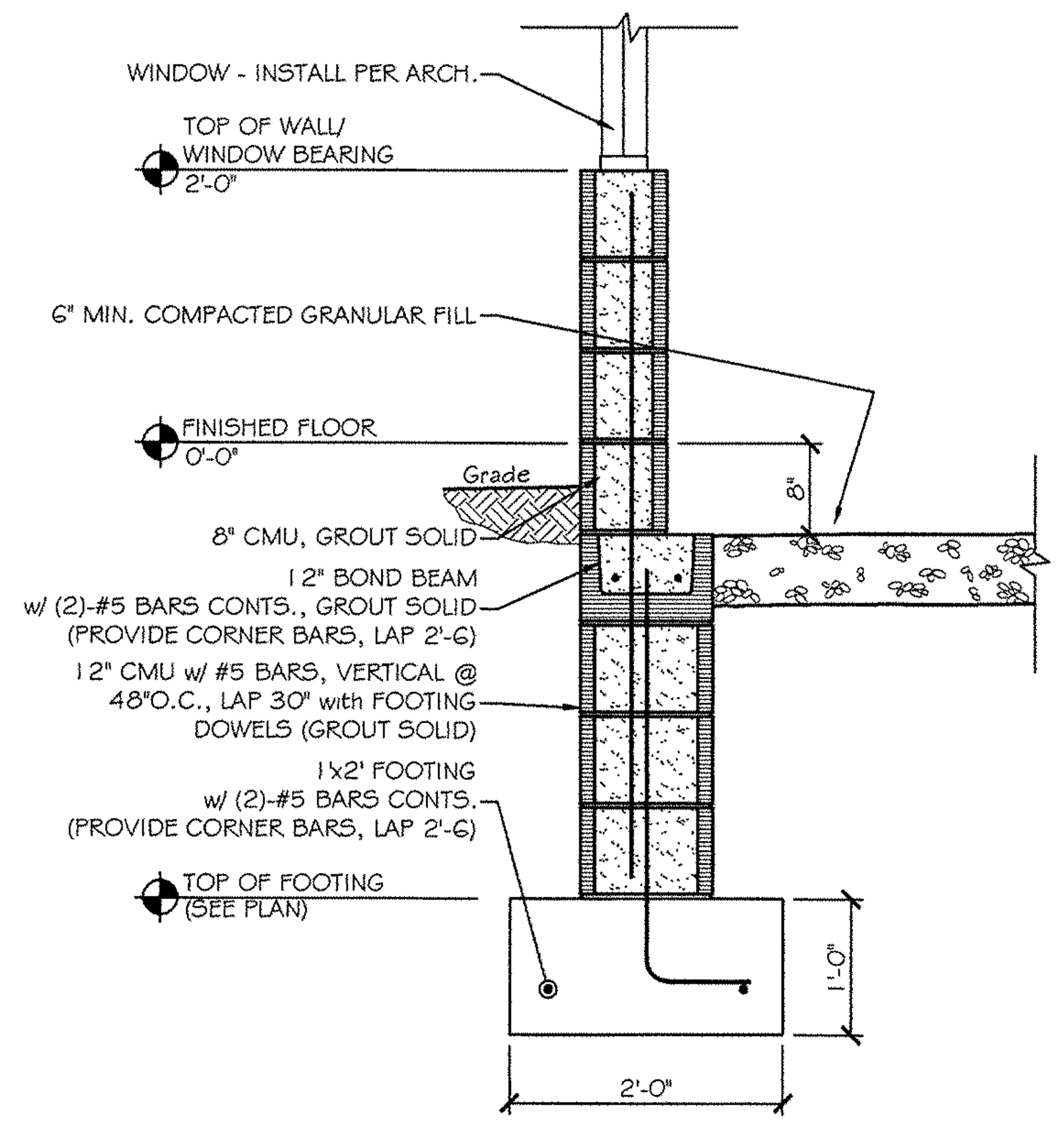
CONTROL JOINT DETAIL 14
3/4" = 1'-0"



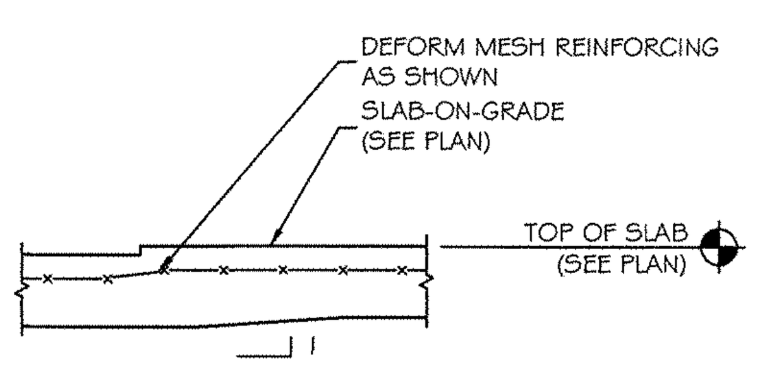
TYP. INTERIOR FOOTING 9
3/4" = 1'-0"



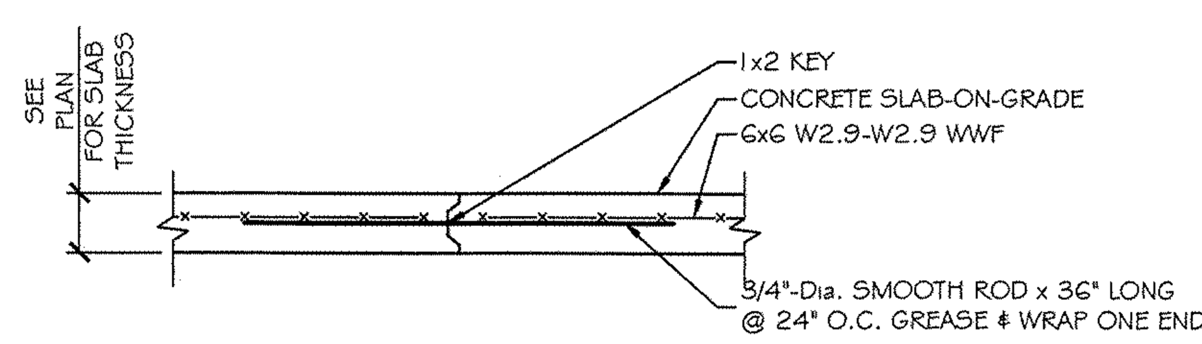
BASE PLATE & PIER DETAILS 6
3/4" = 1'-0"



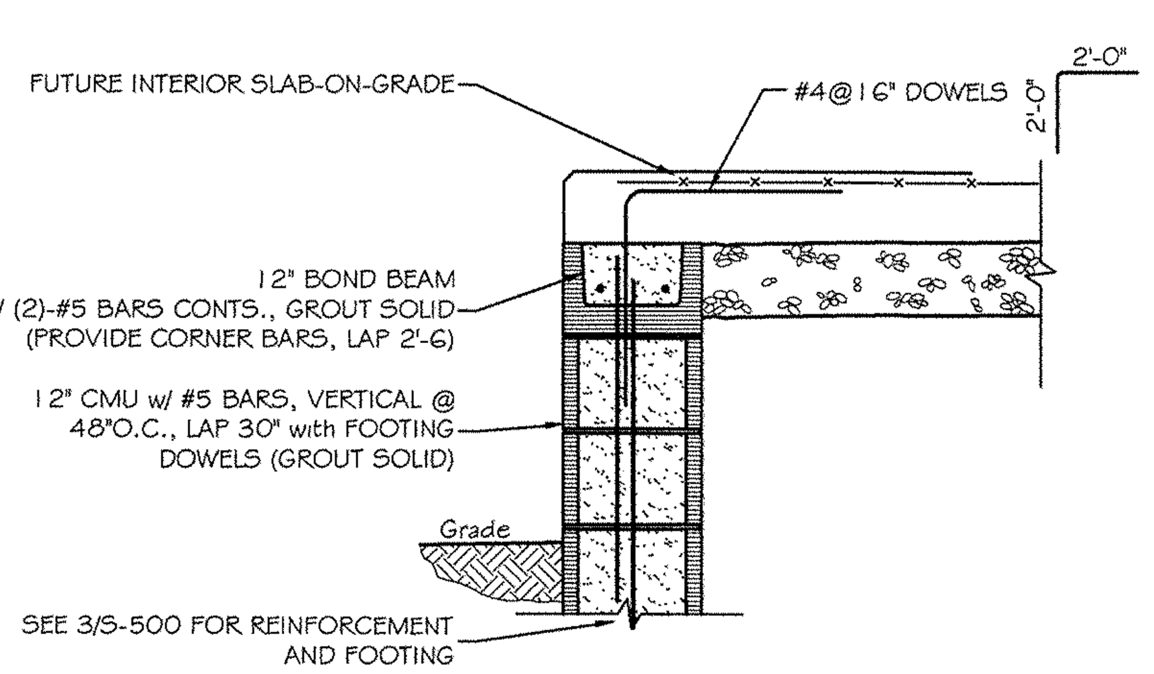
FOUNDATION WALL AT WINDOWS 2
SCALE: 3/4" = 1'-0"



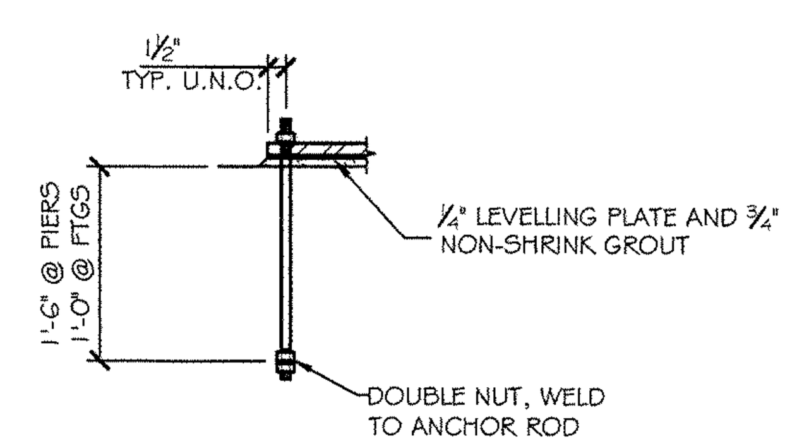
RECESSED SLAB DETAIL 13
3/4" = 1'-0"



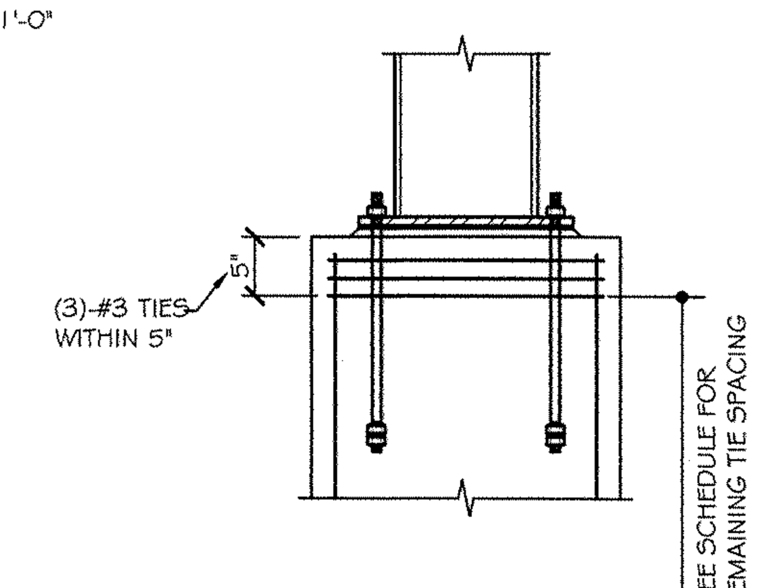
SLAB CONSTRUCTION JOINT DETAIL 12
3/4" = 1'-0"



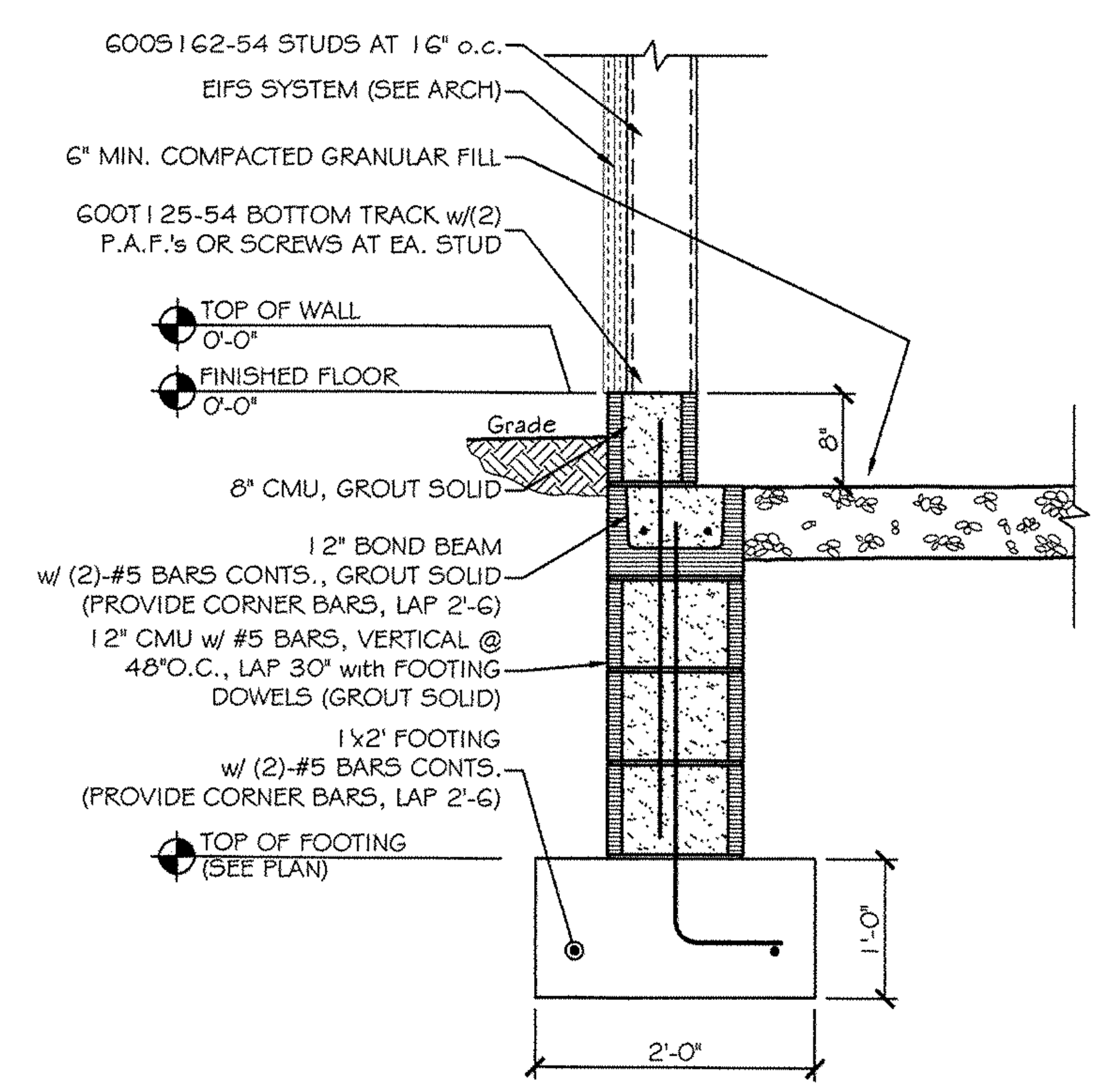
SECTION AT ENTRY (ABOVE GRADE) 8
3/4" = 1'-0"



TYP. ANCHOR ROD DETAILS 5
3/4" = 1'-0"



TYP. PIER DETAIL 4
3/4" = 1'-0"



TYP. FOUNDATION WALL SECTION 1
SCALE: 3/4" = 1'-0"

REVISIONS:

MARK	DESCRIPTION	DATE
△		
△		
△		
△		
△		

CLIENT/OWNER:
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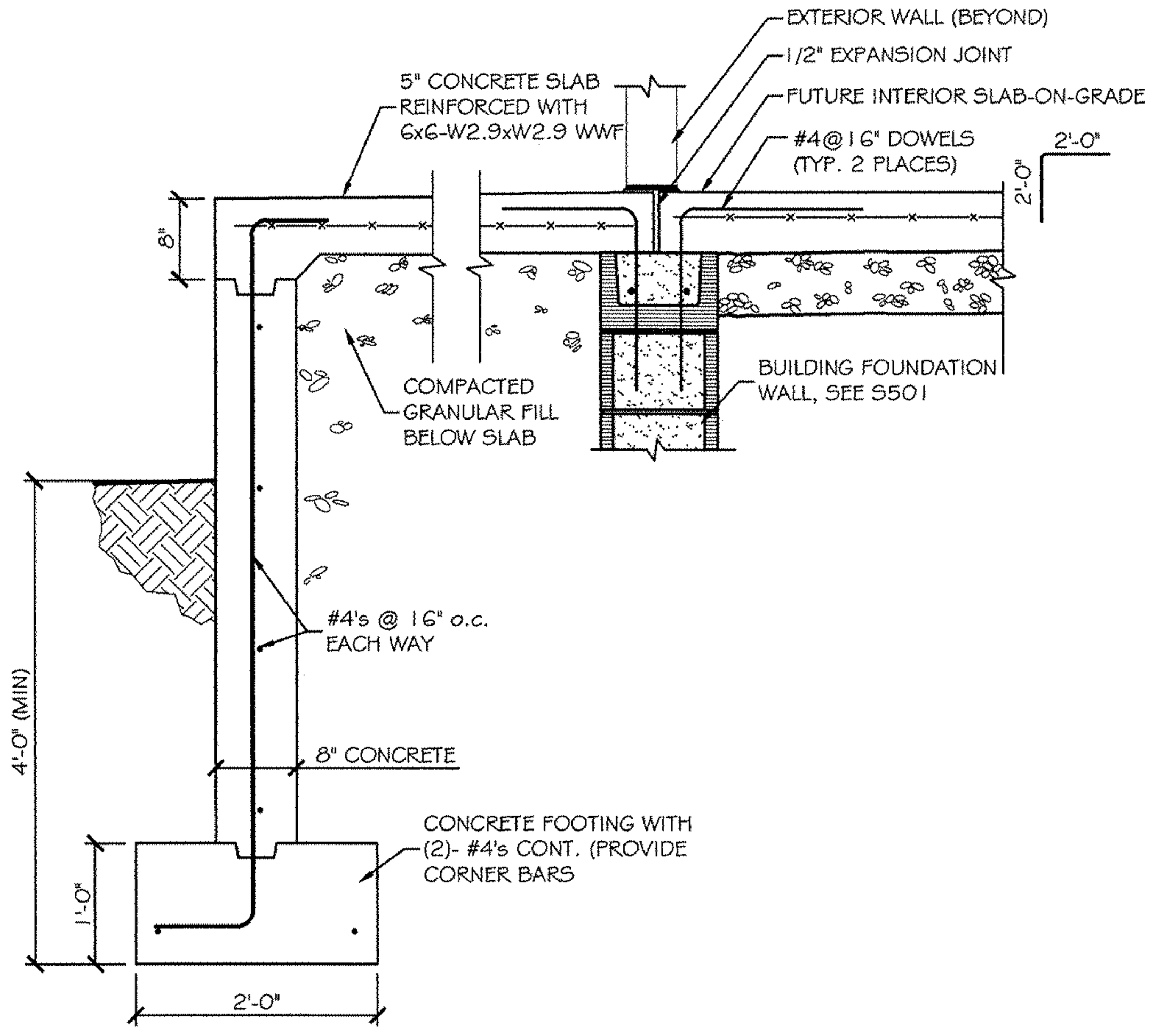
STATE OF NEW YORK
JOHN R. BAUMLER
PROFESSIONAL ENGINEER

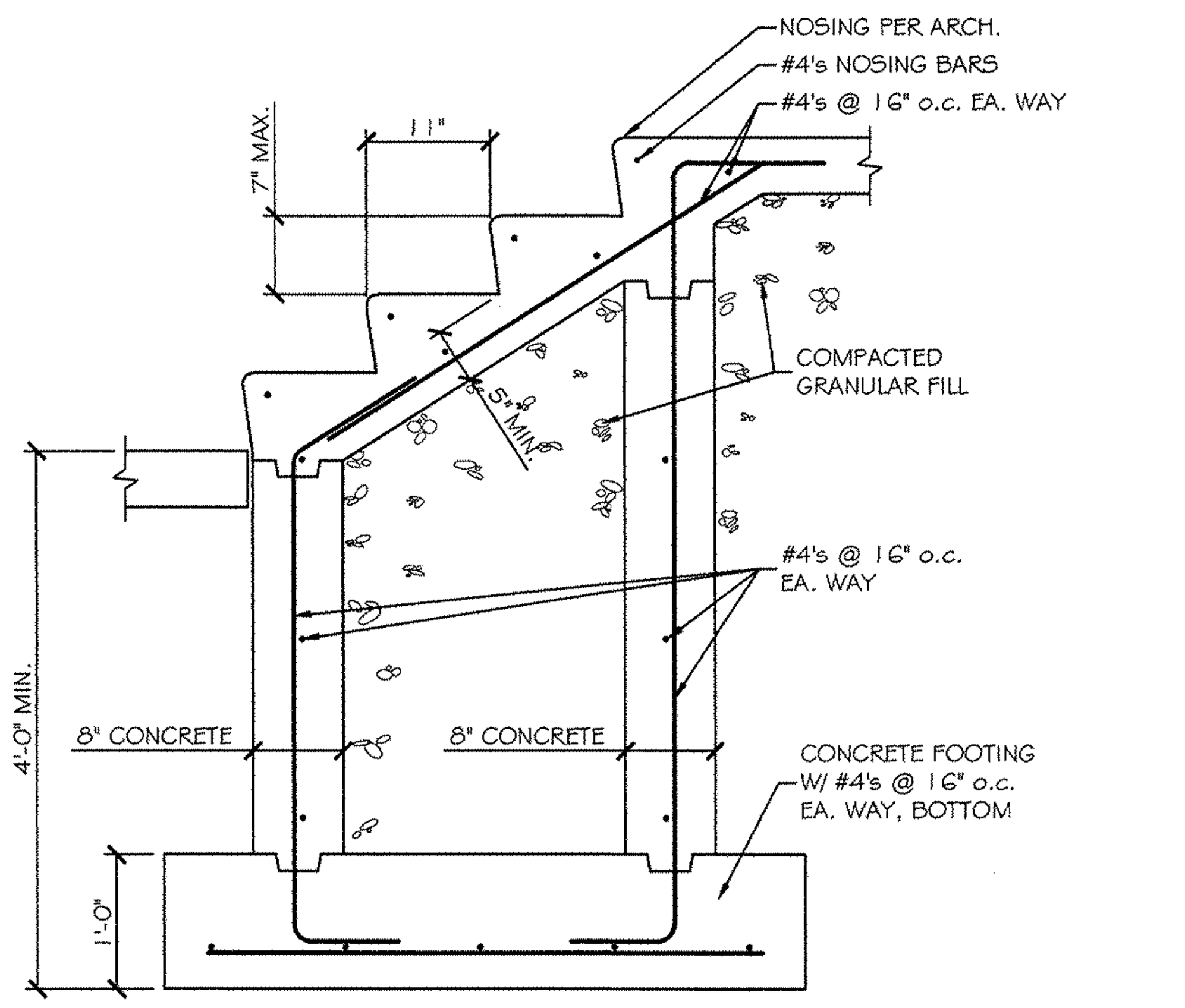
SHEET NO.:
S501

PROJECT NO.:



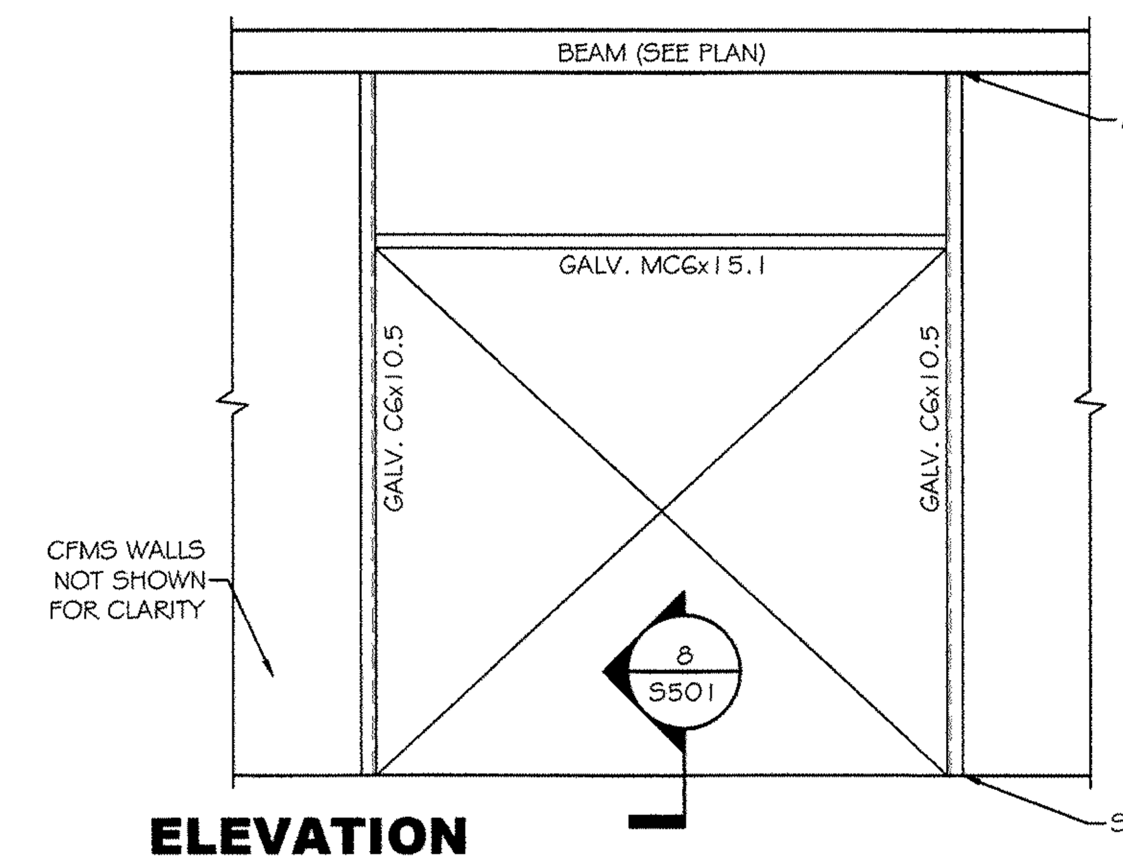
SECTION AT EXTERIOR STAIRS 11

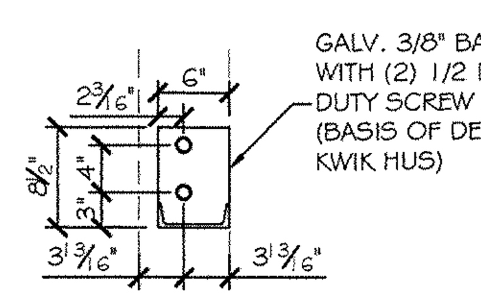
SCALE: 3/4" = 1'-0"



SECTION AT EXTERIOR STAIRS 12

SCALE: 3/4" = 1'-0"

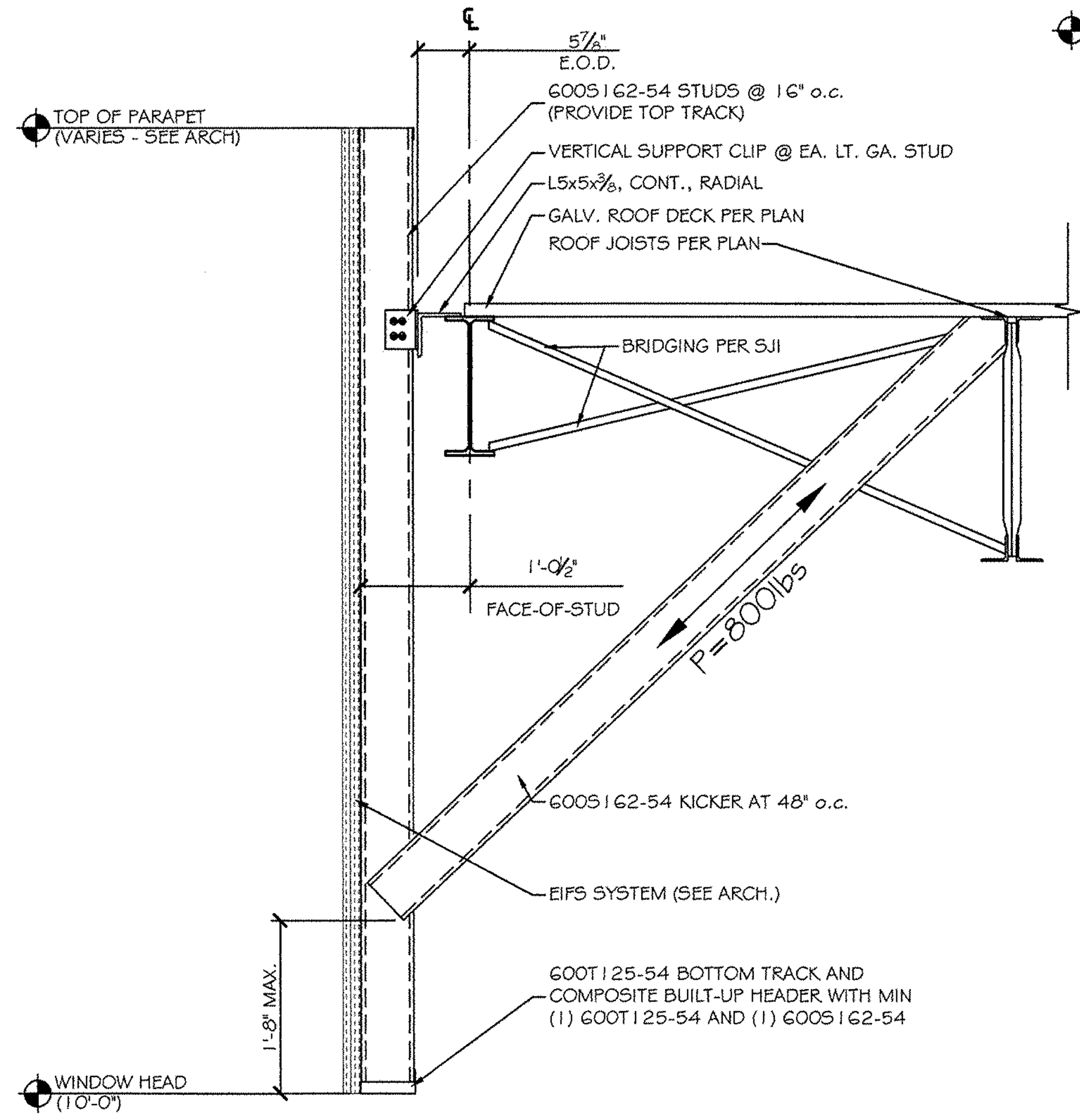




DET. 1 - C6 BASE PLATE

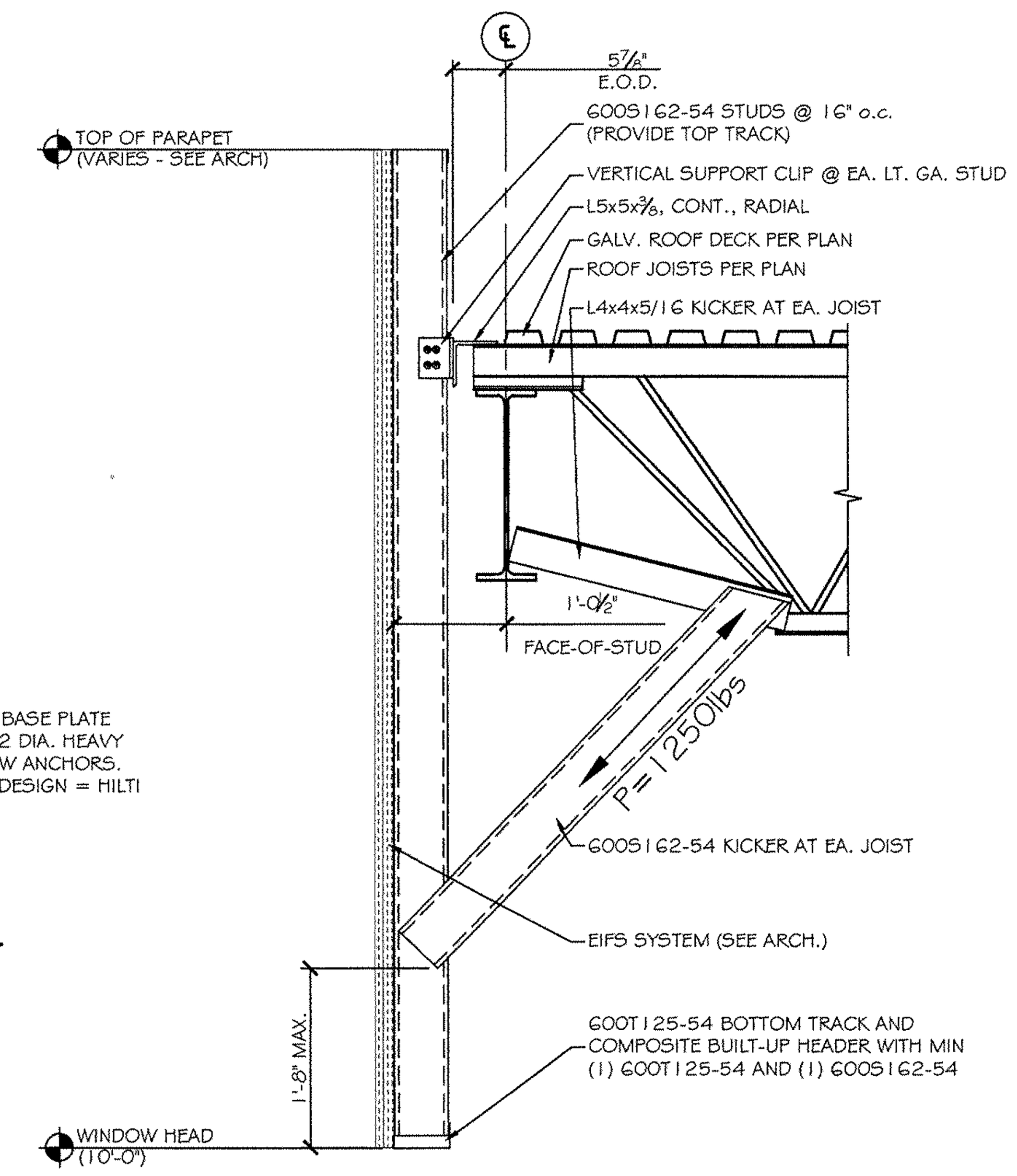
ELEVATION at OVERHEAD DOORS 9

SCALE: Not to Scale



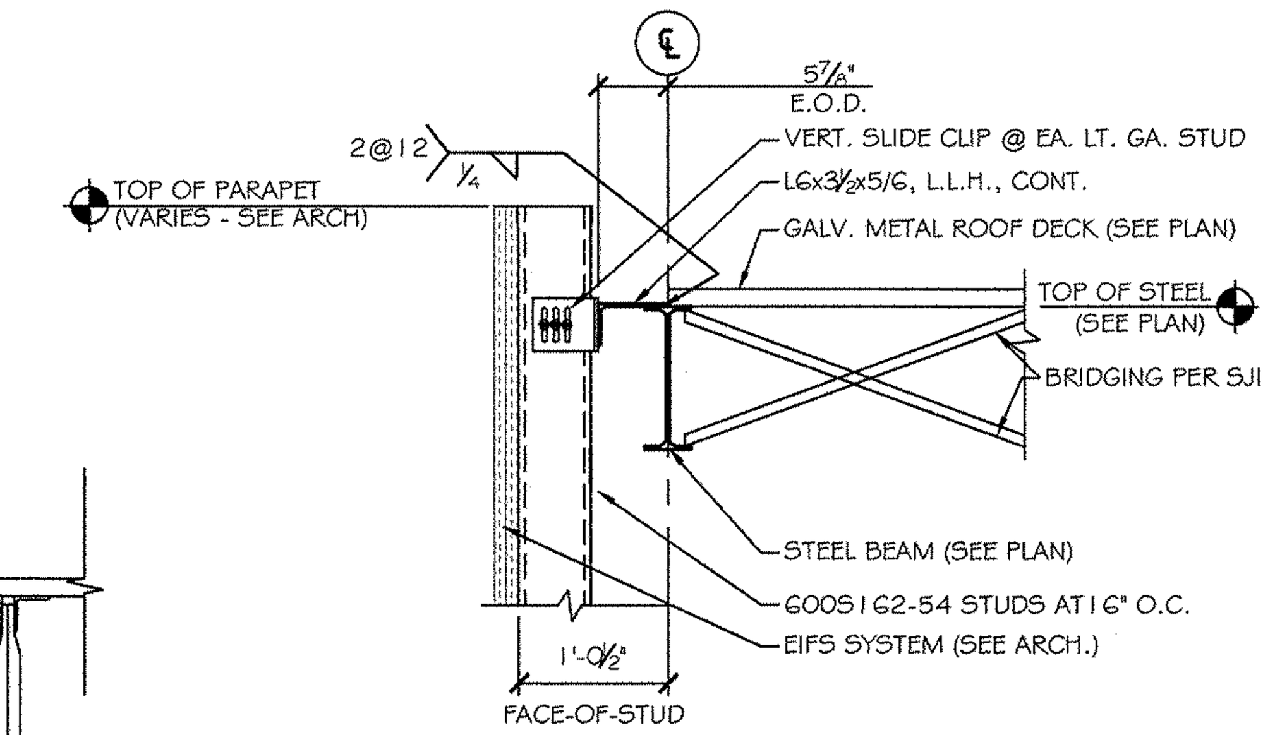
SECTION AT OPENINGS 8

SCALE: 3/4" = 1'-0"



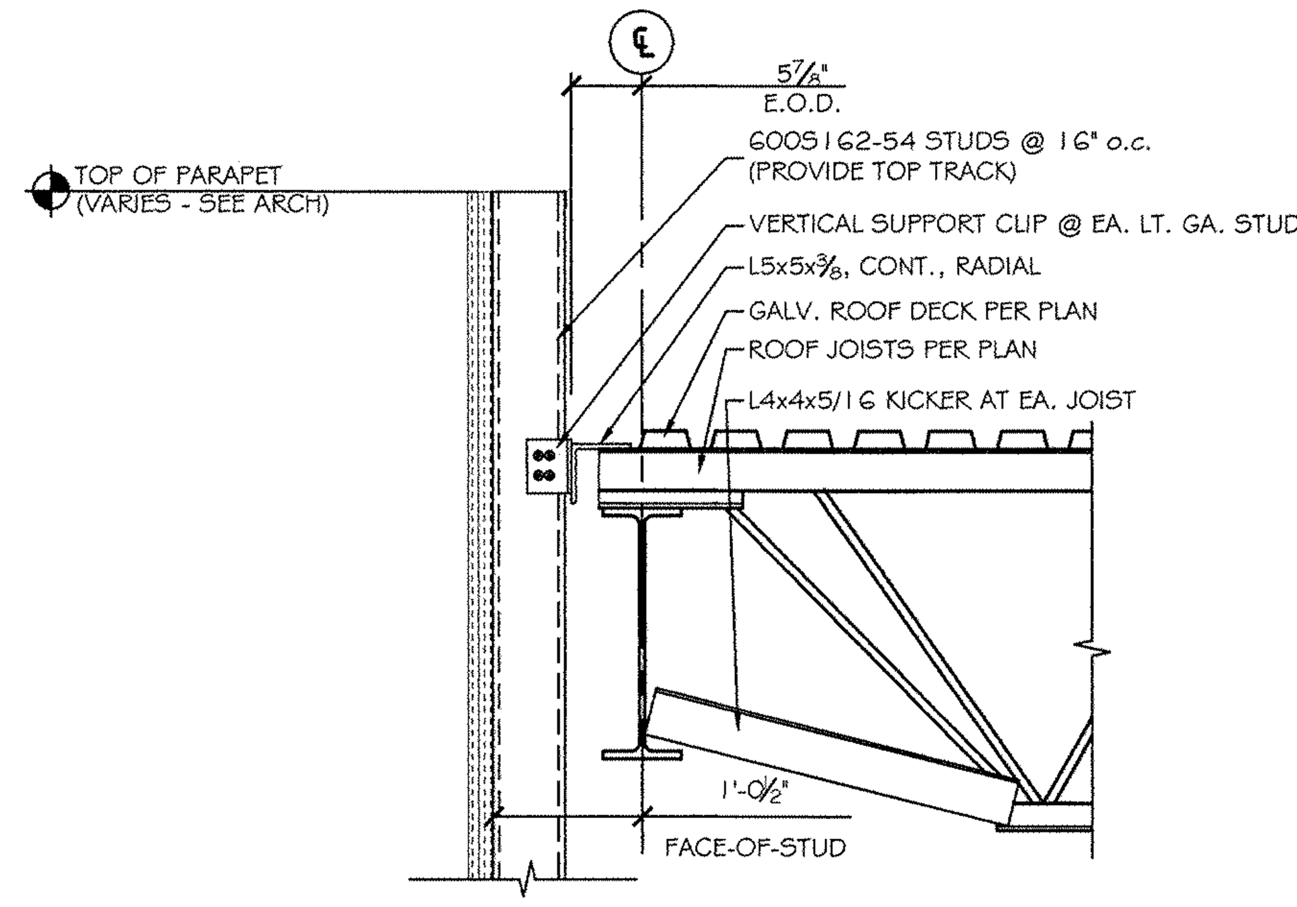
SECTION AT OPENINGS 7

SCALE: 3/4" = 1'-0"



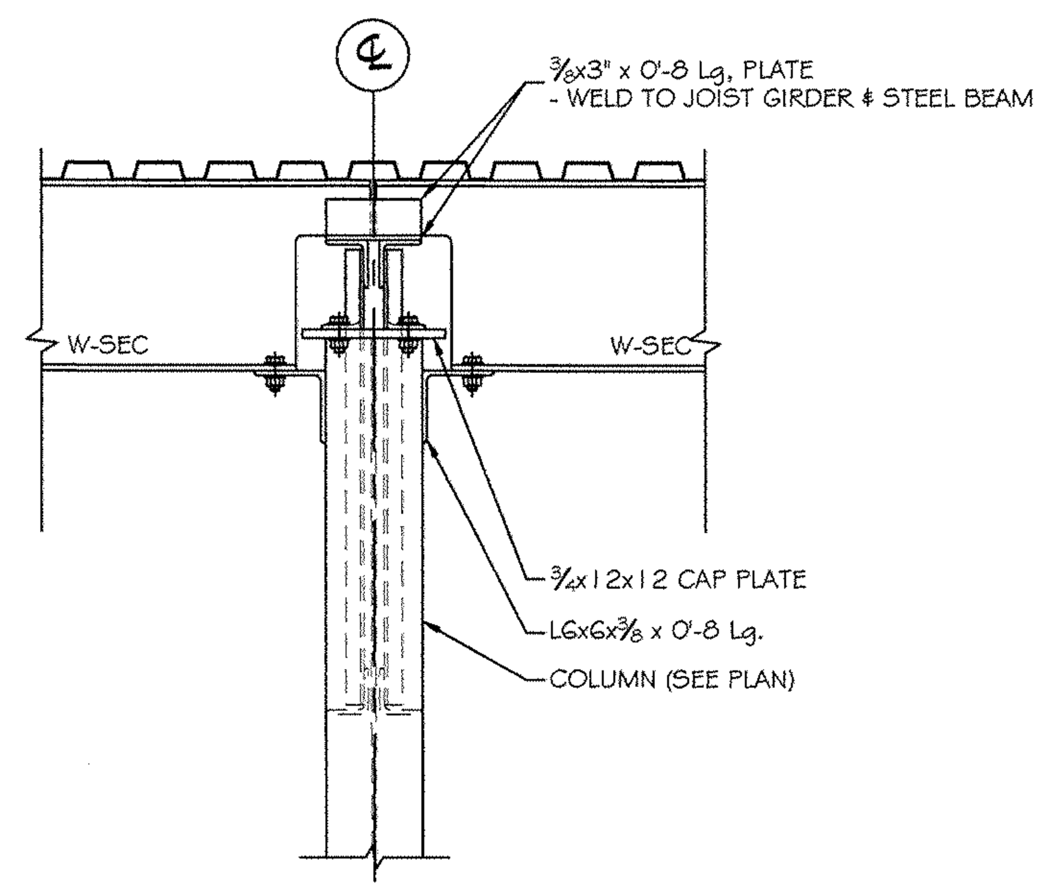
SECTION 6

3/4" = 1'-0"



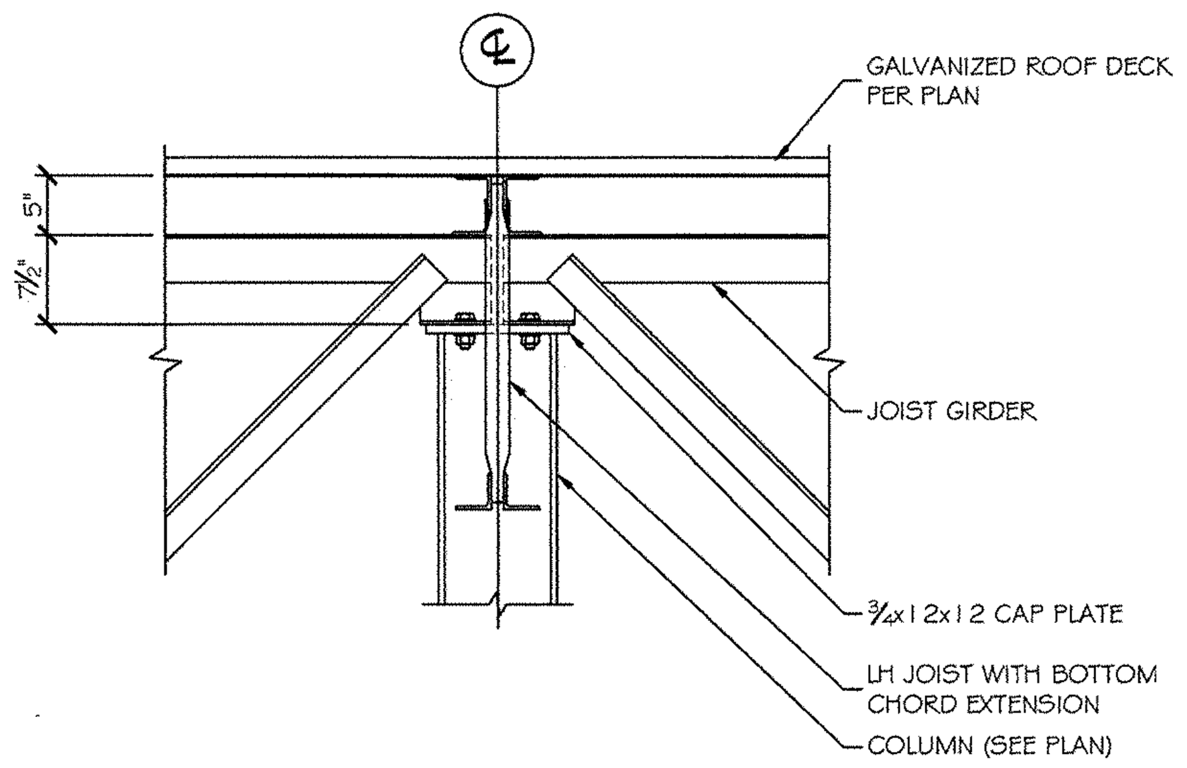
SECTION 5

SCALE: 3/4" = 1'-0"



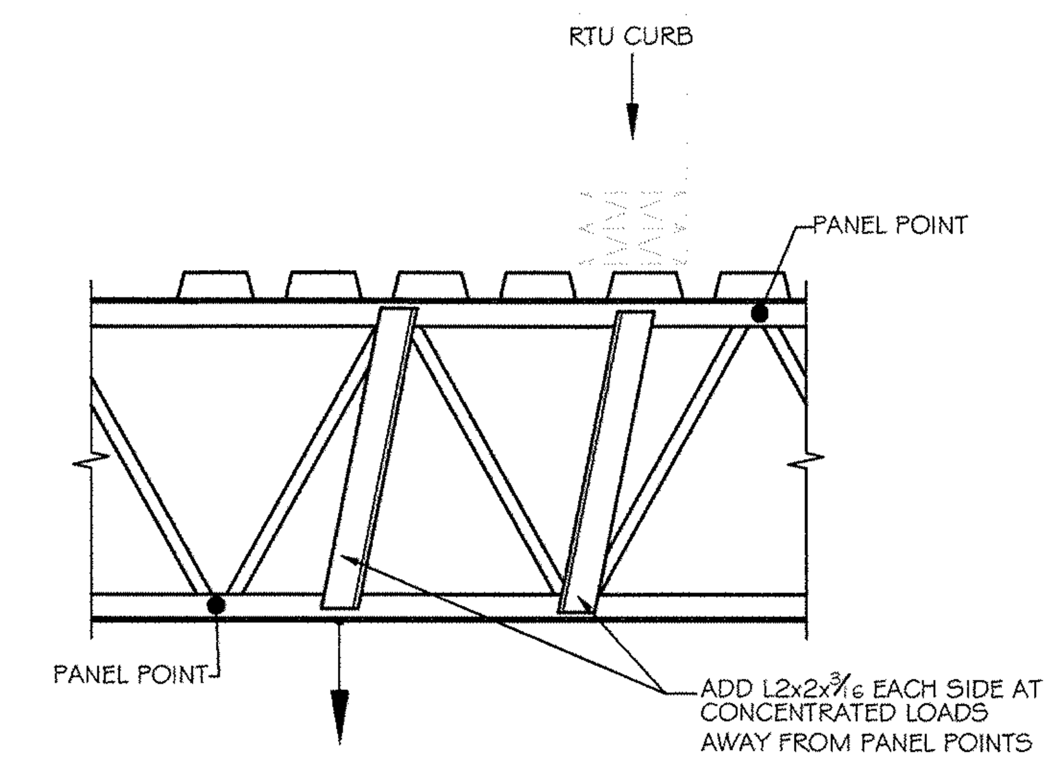
SECTION 4

SCALE: 3/4" = 1'-0"



SECTION 3

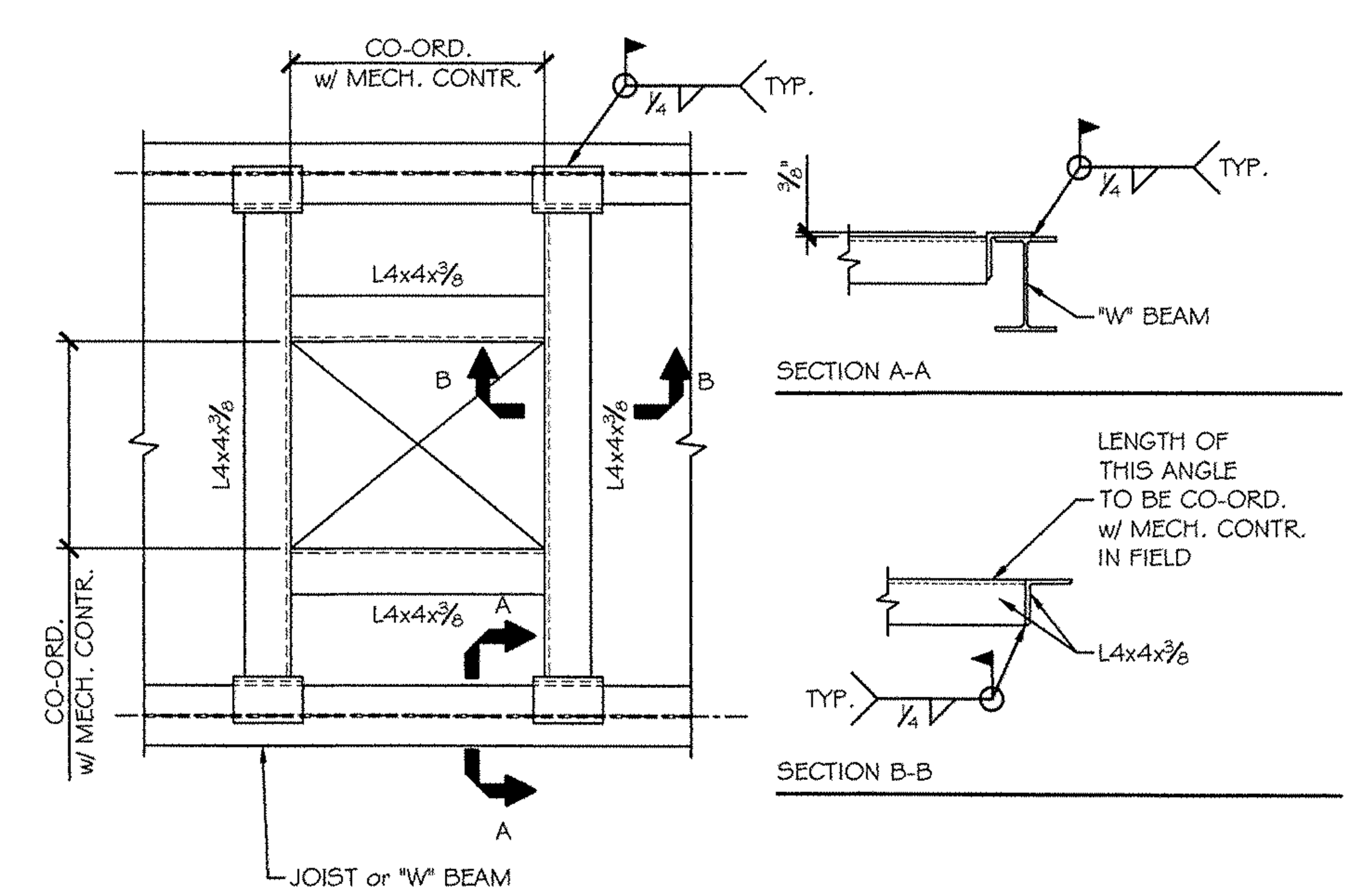
SCALE: 3/4" = 1'-0"



NOTE: PROVIDE WHERE CONCENTRATE LOADS ARE LOCATED MORE THAN 4" FROM A PANEL POINT.

TYP. JOIST REINFORCING DETAIL 2

3/4" = 1'-0"



TYPICAL ROOF FRAME PENETRATION 1

SCALE: 3/4" = 1'-0"

DRAWING ALTERATION

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MARK	DESCRIPTION	DATE
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△		
△		

CLIENT/OWNER:
PLATINUM PROPERTY MANAGEMENT
P.O. BOX 26350
ROCHESTER, NY 14626

PROJECT TITLE:
GATEWAY BUISINESS PARK
PINEWILD DRIVE
GREECE, NY

SCALE: AS NOTED
DATE: 9/3/2015
DRAWN BY: RF
ENGINEER: JRB

Jensen BRV ENGINEERING, PLLC
Structural Engineering Consultants
1653 EAST MAIN STREET
ROCHESTER, NEW YORK, 14609
phone: (585) 402-8130
fax: (585) 482-0440
web site: www.jensenbrv.com

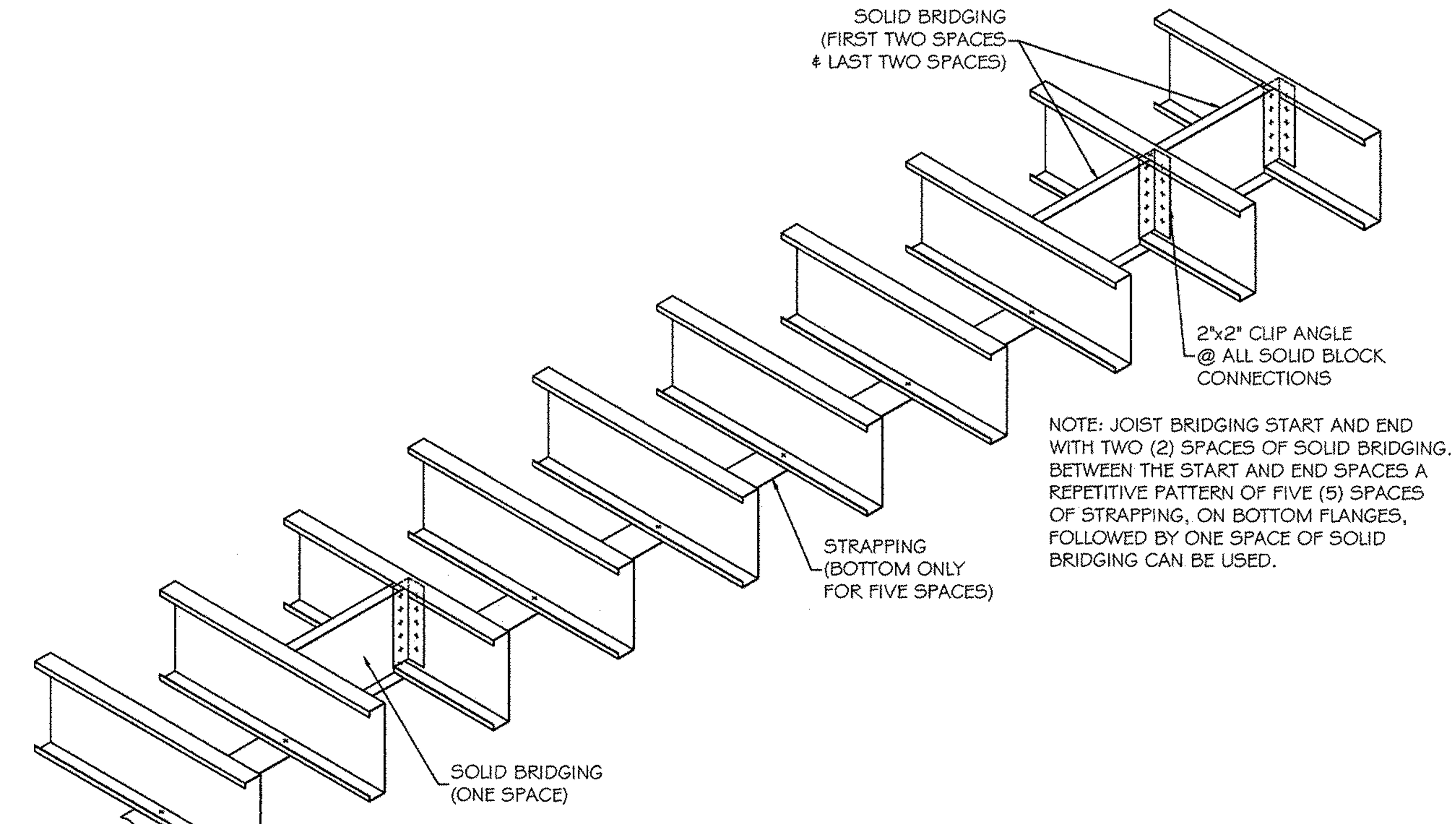
STATE OF NEW YORK · JOHN R. BAUMLER · PROFESSIONAL ENGINEER

SHEET NO.:
S502

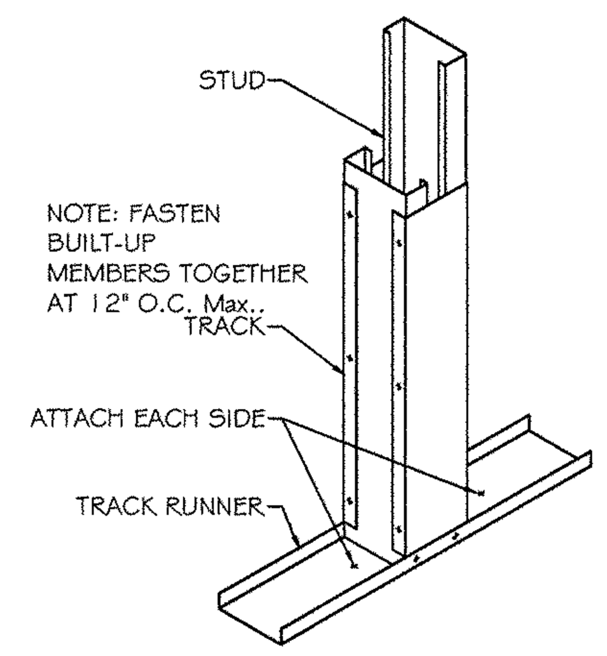
PROJECT NO.:

FASTENER/CONNECTOR SCHEDULE			
CONNECTOR	SUBSTRATE	DESCRIPTION	PRODUCT
SCREWS	METAL TRACK	#10-16 x 5/8" PAN HEAD	BUILDEX "TEKS" COMPASS "DARTS" GRABBER SELF-DRILLING
	STUD-TO-STUD	#10-16 x 5/8" HEX HEAD	BUILDEX "TEKS" COMPASS "DARTS" GRABBER SELF-DRILLING
	METAL-TO-STRUCTURAL STEEL	#12-24 x 1¼" HEX HEAD,#5 TIP	BUILDEX "TEKS" HILTI KWIK-PRO
	WOOD FRAMING or PLYWOOD	#14-20 x 2¾" PHILLIPS FLAT HEAD, #4 WINGS	BUILDEX "TEKS" HILTI KWIK-PRO
P.D.F.'s (Powder Driven Fasteners)	CONCRETE or GROUTED C.M.U.	0.143"Ø (Min. Shank) 1¼" Embedment	POWERS .300 HEAD DRIVE PAN HILTI 0.145"ø X-DNI
	STRUCTURAL STEEL	0.143"Ø (Min. Shank) 5/8" (Min. Length)	POWERS .300 HEAD DRIVE PAN HILTI X-ENI

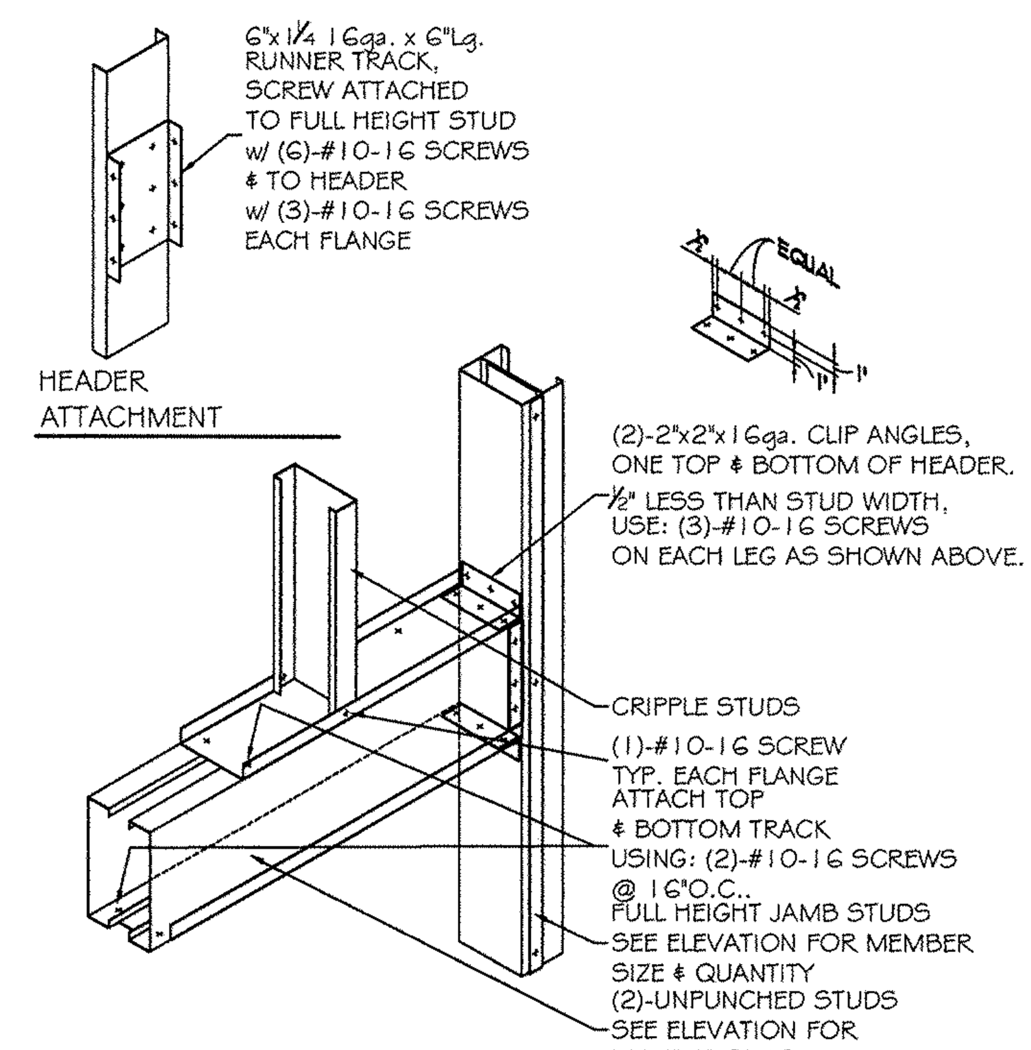
** PRODUCTS BY OTHER MANUFACTURERS MAY BE ACCEPTABLE, PLEASE CONTACT OUR OFFICE OR SUBMIT PRODUCT DATA.



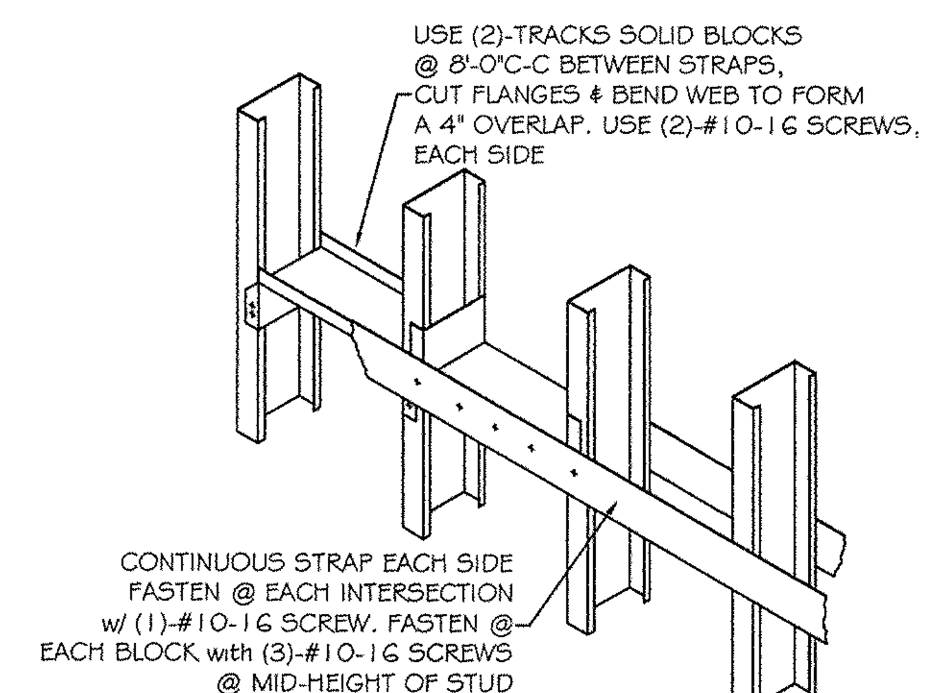
BRIDGING DETAILS -
BLOCK & STRAP FOR JOISTS -
ONE ROW AT MID-SPAN OF FLOOR JOISTS **4**
SCALE: 1" = 1'-0"



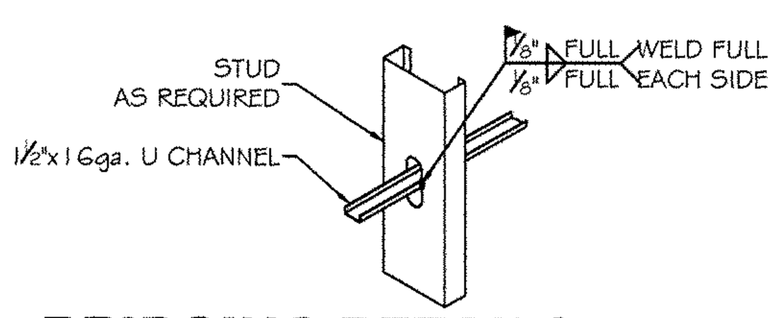
BUILT-UP POST **7**
SCALE: 1" = 1'-0"



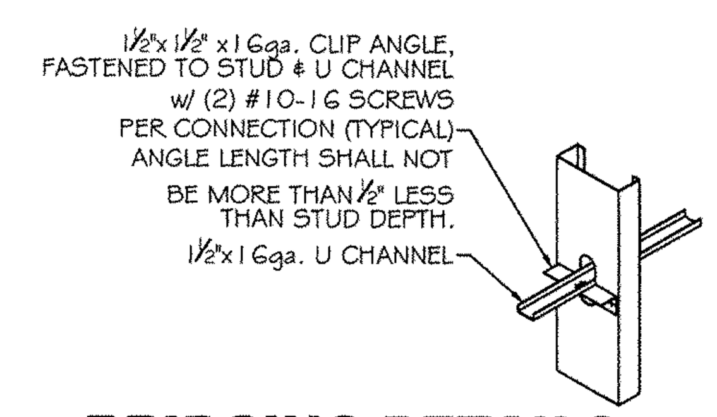
BOXED HEADER CONNECTION -
HEADER-TWO MEMBER BOXED **6**
SCALE: 1" = 1'-0"



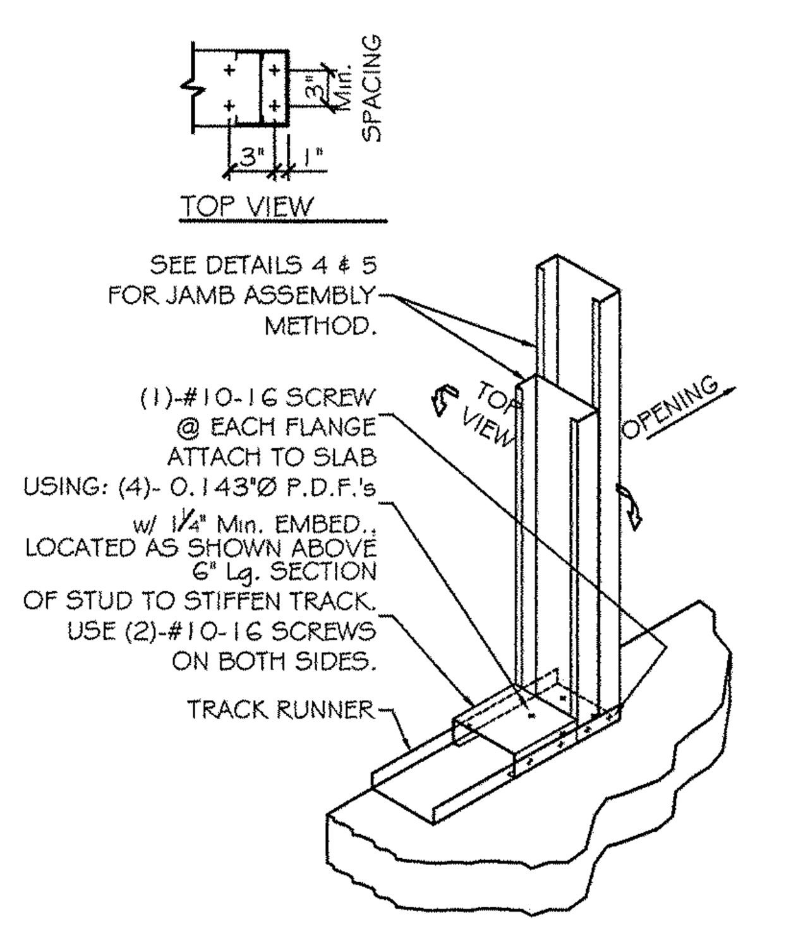
BRIDGING DETAILS -
BLOCK & STRAP FOR WALL STUDS **3**
SCALE: 1" = 1'-0"



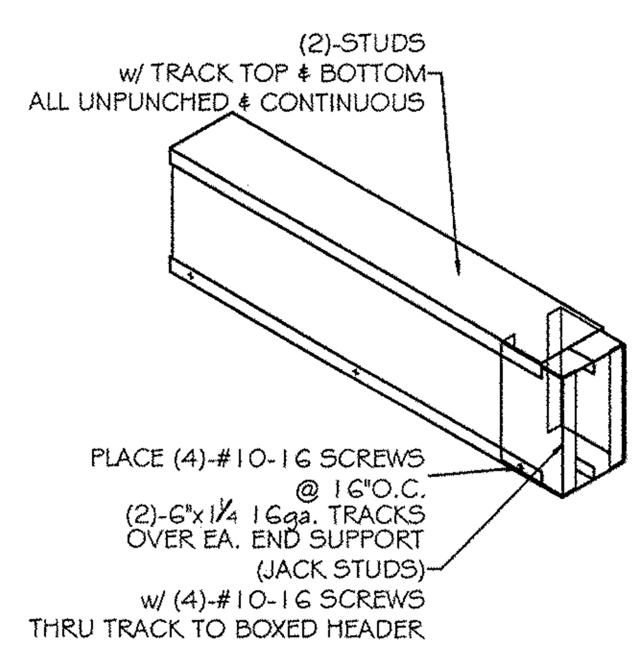
BRIDGING DETAILS -
WELDED U CHANNEL **2**
SCALE: 1" = 1'-0"



BRIDGING DETAILS -
U CHANNEL with CLIP ANGLE **1**
SCALE: 1" = 1'-0"



DOOR JAMB ANCHORAGE -
(2)-JAMB ANCHORAGE TO
STRUCTURE w/ STUD STIFFENER **8**
SCALE: 1" = 1'-0"



WEB STIFFENERS -
BOXED HEADER **5**
SCALE: 1" = 1'-0"

REVISIONS:

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SHEET NO.:
S503

PROJECT NO.: